City of Stamford Zoning Board c/o Ralph Blessing, Land Use Bureau Chief
888 Washington Boulevard
Stamford, CT 06901
Re: 819-831 East Main Street \& 15, and 27-29 Lafayette Street Zone Map Change, GDP, Special Permit, and Final Site and Architectural Plan Application

Dear Mr. Blessing and Board Members,
As discussed, on behalf of 819 East Main Street LLC, 831-833 East Main Street LLC, and New Star Lafayette LLC (collectively "the Applicants"), enclosed please find an application and supportive materials for a Zone Change, Special Permit, and Final Site and Architectural Plans to facilitate the construction of an 130-unit residential development with ground floor retail and associated site improvements. Application details and design elements are described further in the attached Project Narrative and reflected in the enclosed plans.

In support of the applications, enclosed please find:

1. A check in the amount of $\$ 5,235.64$ for:

- Zone Change: $\$ 1,060$;
- Special Permit: \$3,175.64
- Public Hearing Fee: $\$ 1,000$

2. Planning Board request Letter;
3. Zone Change Application;
4. GDP Application;
5. Special Permit Application;
6. Site \& Architectural Plan (FSP) application;
7. Project Narrative;
8. Parking Management Plan;
9. Drawing List;
10. Aerial Exhibit;
11. MX-D Exhibit;
12. General Property Description;
13. Zone Change Map;
14. Zone Change Area Description;
15. Owner List;
16. Zoning Data Charts;
17. Civil Engineering Plans;
18. Architectural Plans and Elevations;
19. Landscaping Plan prepared;
20. Lighting Plan;
21. Sustainability Score Card;
22. Drainage Narrative;
23. Traffic Report
24. Letters of Authorization.

Please feel free to contact us with any questions or comments. We look forward to continuing to work with you and the Planning \& Zoning Boards on this exciting redevelopment.

> Sincerely,


Enclosures
CC: V. Mathur, Associate Planner
Redevelopment Team

City of Stamford Planning Board c/o Ralph Blessing, Land Use Bureau Chief
888 Washington Boulevard
Stamford, CT 06901
Re: 819-831 East Main Street \& 15, and 27-29 Lafayette Street Zone Map Change, GDP, Special Permit, and Final Site and Architectural Plan Application

Dear Mr. Blessing,
As you may be aware, we have submitted a Zone Map Change, GDP, Special Permit, and Final Site and Architectural Plan Application applications. Please let this letter serve as our formal request for members of the consultant team to speak, should the Planning Board have any questions for the applicant at the forthcoming referral meeting. Please let us know if you have any questions or would like additional information.


Enclosures
CC: V. Mathur, Associate Planner

## APPLICATION FOR CHANGE IN THE ZONING MAP OF STAMFORD, CONNECTICUT

Complete, notorize, and forward thirteen (13) hard copies and (1) electronic copy in PDF format to Clerk of the Zoning Board with a $\$ 1,000.00$ Public Hearing Fee and the required application filing fee (see Fee Schedule below), payable to the City of Stamford.

NOTE: Cost of required Public Hearing advertisements are payable by the Applicant and performance of mailing of required property owners is the sole responsibility of the applicant. LAND RECORDS RECORDING FEE: $\$ 60.00$ for First page - $\$ 5.00$ for each additional page)

Fee Schedule

| Map Change (Affected Area of 1 Acre or Less) | $\$ 1,060.00$ |
| :--- | :--- |
| Map Change (Affected Area of greater than 1 Acre) | $\$ 1,060.00+$ <br> $\$ 2,000$ per acre <br> or portion <br> thereof in <br> excess of 1 <br> acre |

APPLICANT NAME (S): 819 East Main Street LLC, 831-833 East Main Street LLC, and New Star Lafayette LLC (collectively "the Applicants") APPLICANT ADDRESS: c/o Redniss \& Mead - 22 First Street, Stamford, CT 06905

APPLICANT PHONE \#:__clo203-327-0500
IS APPLICANT AN OWNER OF PROPERTY IN THE CITY OF STAMFORD? ___ Yes
PRESENT ZONING DISTRICT: C-I PROPOSED ZONING DISTRICT: ___ MX-D

LOCATION OF PROPOSED CHANGE: (Give boundaries of each parcel in proposed change and indicate dimensions from nearest intersecting street. Also include Assessor's Card number and Town Clerk's Block number, and square footage of land. Attach twelve (12) copies of map showing area proposed for change.)

| Please see attached Zone Change Description |
| :--- |
| LIST NAME AND ADDRESS OF THE OWNERS OF ALL LAND INCLUDED WITHIN THE PROPOSED CHANGE: <br> NAME \& ADDRESS <br> ARE THERE DEATION |

$\qquad$

DOES ANY PORTION OF THE PREMISES AFFECTED BY THIS APPLICATION LIE WITHIN 500 FEET OF THE BORDER LINE WITH GREENWICH, DARIEN OR NEW CANAAN? $\qquad$ No (If yes, notification must be sent to Town Clerk of neighboring community by registered mail within 7 days of receipt of application - PA 87-307).


NOTE: The application cannot be scheduled for public hearing until 35 days have elapsed from the date of referral to the Stamford Planning Board. If applicant wishes to withdraw the application, this must be done in writing, and be received by the Zoning Board at least three (3) working days prior to public hearing in order to provide sufficient time to publicize the withdrawal. Applications withdrawn less than three (3) days prior to a schedule hearing date will not be rescheduled within 90 days.
STATE OF CONNECTICUT
COUNTY OF FAIRFIELD

COUNTY OF FAIRFIELD
Personally appeared Ray mend $R$. Mazzeo_, signer of the foregoing application, who made oath to the truth of the contents thereof, before me.

DAVID PINTO
Notary Public, State of Connecticut
而
"My Commission Expires Mar 31, 2026
FOROFFICEUSEONLY
APPL. \#: Received in the office of the Zoning Board: Date: $\qquad$

By: $\qquad$

## APPLICATION FOR APPROVAL OF SITE \& ARCHITECTURAL PLANS AND / OR REQUESTED USES

Complete, notorize, and forward thirteen (13) copies and one (1) electronic copy in PDF format to Clerk of the Zoning Board with a $\$ 1,000.00$ Public Hearing Fee and the required application filling fee (see Fee Schedule below), payable to the City of Stamford.

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(GENERAL DEVELOPMENT PLAN)
Fee Schedule

| General Development Plan - Sites 20,000 sq. ft. or less parcel area. | $\$ 460.00$ |
| :--- | :--- |
| General Development Plan - Sites more than 20,000 sq. ft. or parcel area. | $\$ 460+\$ 20$ per <br> 1,000 sq. ft. in <br> excess of 20,000 <br> sq. ft. |

APPLICANT NAME (S): 819 East Main Street LLC, 831-833 East Main Street LLC, and New Star Lafayette LLC (collectively "the Applicants").
APPLICANT ADDRESS: c/o Redniss \& Mead - 22 First Street, Stamford, CT 06905
APPLICANT PHONE \#:c/o 203-327-0500
IS APPLICANT AN OWNER OF PROPERTY IN THE CITY OF STAMFORD? Yes
LOCATION OF PROPERTY IN STAMFORD OWNED BY APPLICANT (S): Please see attached Owner List
ADDRESS OF SUBJECT PROPERTY: _ Please see attached Owner List
PRESENT ZONING DISTRICT: MX-D \& C-I
TITLE OF SITE PLANS \& ARCHITECTURAL PLANS: Please see attached Drawing List
REQUESTED USE: $\quad$ Please see attached Project Narrative

LOCATION: (Give boundaries of land affected, distance from nearest intersecting streets, lot depths and Town Clerk's Block Number)
$\qquad$
Please see attached Property Description

NAME AND ADDRESS OF OWNERS OF ALL PROPERTY INVOLVED IN REQUEST:
NAME \& ADDRESS
Please see attached Owner List

DOES ANY PORTION OF THE PREMISES AFFECTED BY THIS APPLICATION LIE WITHIN 500 FEET OF THE BORDER LINE WITH GREENWICH, DARIEN OR NEW CANAAN? No (If yes, notification must be sent to Town Clerk of neighboring community by registered mail within 7 days of receipt of application -PA 87-307).

DOES THE PROJECT RESULT IN THE CREATION OF 10 OR MORE UNITS OR 10,000 SF OR MORE IN FLOOR AREA OR DISTURBANCE OF 20,000 SF OR MORE IN LAND AREA, THROUGH NEW DEVELOPMENT, RECONSTRUCTION, ENLARGEMENT OR SUBSTANTIAL ALTERATIONS? $\qquad$ (If yes, then complete the Stamford Sustainability
Scorecard per Section 15.F).


NOTE: Application cannot be scheduled for Public Hearing until 35 days have elapsed from the date of referral to the Stamford Planning Board. If applicant wishes to withdraw application, please notify the Zoning Board at least three (3) days prior to Public Hearing so that the Board may have sufficient time to publicize the withdrawal.
COUNTY OF FAIRFIELD
SS STAMFORD $\qquad$ February 8
2022

Personally appeared Raymond R. Mazzeo $\qquad$ signer of the foregoing application, who made oath to the truth of the contents thereof, before me.


APPL. \#: $\qquad$ Received in the office of the Zoning Board: Date: $\qquad$

By: $\qquad$

## APPLICATION FOR SPECIAL PERMIT

Complete, notorize, and forward thirteen (13) hard copies and (1) electronic copy in PDF format to Clerk of the Zoning Board with a $\$ 1,000.00$ Public Hearing Fee and the required application filling fee (see Fee Schedule below), payable to the City of Stamford.

NOTE: Cost of required advertisements are payable by the Applicant and performance of required mailing to surrounding property owners is the sole responsibility of the applicant. LAND RECORDS RECORDING FEE: $\$ 60.00$ for First page $\$ 5.00$ for each additional page)

Fee Schedule

| Special Permit 20,000 sq. ft. or less | $\$ 460.00$ |
| :--- | :--- |
|  | $\$ 460.00+\$ 30$ per <br> 1,00 sq. ft. or <br> Special Permit more than 20,000 sq. ft. |
| excess of oreof in 20,000 sq. <br> ft. |  |

APPLICANT NAME (S): 819 East Main Street LLC, 831-833 East Main Street LLC, and New Star Lafayette LLC (collectively "the Applicants") APPLICANT ADDRESS: c/o Redniss \& Mead - 22 First Street, Stamford, CT 06905
APPLICANT PHONE \#:_c/o 203-327-0500
IS APPLICANT AN OWNER OF PROPERTY IN THE CITY OF STAMFORD? Yes
LOCATION OF PROPERTY IN STAMFORD OWNED BY APPLICANT (S): _ Please see attached Owner List
ADDRESS OF SUBJECT PROPERTY: $\frac{\text { Please see attached Owner List }}{}$
PRESENT ZONING DISTRICT: $\quad$ MX-D \& C-I
TITLE OF SITE PLANS \& ARCHITECTURAL PLANS: $\quad$ Please see attached Drawing List

REQUESTED SPECIAL PERMIT: (Attach written statement describing request)
$\qquad$

LOCATION: (Give boundaries of land affected, distance from nearest intersecting streets, lot depths and Town Clerk's Block Number)

## Please see attached Property Description

NAME AND ADDRESS OF OWNERS OF ALL PROPERTY INVOLVED IN REQUEST:
NAME \& ADDRESS
LOCATION

Please see attached Owner List

DOES ANY PORTION OF THE PREMISES AFFECTED BY THIS APPLICATION LIE WITHIN 500 FEET OF THE BORDER LINE WITH GREENWICH, DARIEN OR NEW CANAAN? No $\qquad$ (If yes, notification must be sent to Town Clerk of neighboring community by registered mail within 7 days of receipt of application - PA 87-307).

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DATED AT STAMFORD, CONNECTICUT, THIS $\qquad$ DAY OF
 SIGNED:


NOTE: Application cannot be scheduled for Public Hearing until 35 days have elapsed from the date of referral to the Stamford Planning Board. If applicant wishes to withdraw application, please notify the Zoning Board at least three (3) days prior to Public Hearing so that the Board may have sufficient time to publicize the withdrawal.


APPL. \#: $\qquad$ Received in the office of the Zoning Board: Date: $\qquad$

By: $\qquad$

## APPLICATION FOR APPROVAL OF SITE \& ARCHITECTURAL PLANS AND / OR REQUESTED USES

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Fee Schedule -WITHOUT GDP

| Site Plans 20,000 sq. ft. or less of building area application fee -without GDP | $\$ 460.00$ |
| :--- | :--- |
|  | $\$ 460.00+\$ 30$ per <br> Site Plans more than 20,000 sq. ft. of building area-application Fee -without GDP <br> portion thereof in <br> excess of 20,000 <br> sq. ft. |

Fee Schedule -WITH GDP

| Site Plans 20,000 sq. ft. or less of building area application fee -with GDP. | $\$ 260.00$ |
| :--- | :--- |
| Site Plans more than 20,000 sq. ft. of building area-application Fee -with GDP. | $\$ 260.00+\$ 10$ per <br> 1,000 sq. ft . or <br> portion thereof in <br> excess of 20,000 <br> sq. ft. |

APPLICANT NAME (S): 819 East Main Street LLC, 831-833 East Main Street LLC, and New Star Lafayette LLC (collectively "the Applicants")
APPLICANT ADDRESS: c/o Redniss \& Mead - 22 First Street, Stamford, CT 06905
APPLICANT PHONE \#:_c/o 203-327-0500
IS APPLICANT AN OWNER OF PROPERTY IN THE CITY OF STAMFORD? Yes
LOCATION OF PROPERTY IN STAMFORD OWNED BY APPLICANT (S): _ Please see attached Owner List
ADDRESS OF SUBJECT PROPERTY: __Please see attached Owner List
PRESENT ZONING DISTRICT: $\quad$ MX-D \&C-I
TITLE OF SITE PLANS \& ARCHITECTURAL PLANS: Please see attached Drawing List _ P_

TITLE OF SITE PLANS \& ARCHITECTURAL PLANS: Please see attached Drawing List

REQUESTED USE: ___ Please see attached Project Narrative

LOCATION: (Give boundaries of land affected, distance from nearest intersecting streets, lot depths and Town Clerk's Block Number)

Please see attached Property Description

NAME AND ADDRESS OF OWNERS OF ALL PROPERTY INVOLVED IN REQUEST:
NAME \& ADDRESS
LOCATION

Please see attached Owner List

[^0]DATED AT STAMFORD, CONNECTICUT, THIS $\qquad$ $8^{\text {th }}$ Day of february 2022

SIGNED


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STATE OF CONNECTICUT

$\square$ 8 2022
COUNTY OF FAIRFIELD
Personally appeared
Raymond R Nazzeo $\qquad$ signer of the foregoing application, who made oath to the truth of the contents thereof, before me.


APPLy. \#: $\qquad$ Received in the office of the Zoning Board: Date: $\qquad$
$\qquad$

# Project Narrative <br> 819-831 East Main Street and 15-31 Lafayette Street Special Permit and Site \& Architectural Plan Applications 

 February 8, 2022
## 1. Introduction/Overview

819 East Main Street, LLC ("the Applicant") is a related entity of Wellbuilt and the owner and contract purchaser of several contiguous parcels along East Main Street and Lafayette Street. The combined parcels are approximately 1.15 acres and includes the following properties
(collectively "the Site"):
a) 819-821 E. Main Street - commercial/retail building
b) $825 \& 827$ E. Main Street - multi-family dwellings with nonconforming street front parking
c) 831 E. Main Street - vacant lot
d) 27 \& 29 Lafayette Street - multi-family dwellings with nonconforming street front parking
e) 15 Lafayette Street - multi-family dwellings with nonconforming street front parking

The Applicant is proposing a comprehensive redevelopment of the Site that will revitalize the property and neighborhood by removing dangerous nonconforming parking and access on US-1 and Lafayette Street, activating vacant and blighted parcels and transforming underutilized land into a new and exciting residential community. The proposed plan will create 130 new apartments including 18 onsite Below Market Rate units, approximately 3,000 sf of ground level retail, and $650 \pm$ linear feet of new sidewalks with associated onsite parking, landscaping and usable open space.

In order to facilitate the potential redevelopment, the Applicant is proposing an expansion and modification of prior approvals to incorporate 15 Lafayette Street into the overall site and MXD Zone. Applications include a Zoning Map Change, Special Permit, General Development Plan and Site \& Architectural Plans and/or Requested Uses, as further described herein.

## 2. Approval History

This is the third iteration of this proposed redevelopment. Each time the project has improved and expanded to include surrounding underutilized parcels in need of redevelopment, with the current proposal comprising more of the full block from E. Main Street to N. State Street in a comprehensive building design. Prior approvals include:
A. In 2018 the Zoning Board approved applications (\#218-35 \& 218-36) for a redevelopment of the properties including 819-827 East Main Street and 27-31 Lafayette Street (the "Original Property"). These approvals included a zone change to MX-D as well as General Development Plan and Special Permit approvals to facilitate a mixed-use building with 63 one- and two-bedroom apartments, including 7 BMR units, above approximately 2,150 square feet of ground floor retail and residential amenities (the "Original Approval"). Special Permit approvals related to the proposed parking ratio,
proximity of parking to residential units and a fee-in-lieu payment related to the fractional Below Market Rate requirement.
B. The Applicant then completed the purchase of the Original Property and contracted to purchase the adjacent vacant outparcel (831 East Main Street). In 2021 The Zoning Board approved a modified and expanded project incorporating the vacant lot (Apps \#220-45, 220-46 and 221-19). This set of approvals included another zone change to MX-D (for the vacant lot) as well as General Development Plan, Special Permit and Final Site \& Architectural Plan and/or Requested Uses approvals to create 85 one- and twobedroom apartments, including 10 BMR units, with ground floor retail and residential amenities (the "Recent Approval"). Special Permit approvals related to equivalencies of proposed BMR units, location of parking spaces in proximity to the building and reduced parking requirements consisting of 1.0 spaces onsite and another 0.25 spaces to be provided offsite within 500 ' of the property.
C. To complete the block, the Applicant has now secured a contract for the remaining piece at the corner of Lafayette Street and N. State Street to be incorporated into the overall redevelopment as further described herein. The property was subject to Special Exception approval (90-007) relating to historic preservation and associated bonuses. The building has since fallen into disrepair with structural deterioration, and will be difficult to maintain going forward due to significant and recurring flooding issues in the immediate area.

## 3. Surrounding Area

The surrounding area consists primarily of Master Plan Category 9 (Urban Mixed Use) with portions of Category 11 (Downtown) and 13 (Industrial - General) to the south. Adjacent zoning districts include R-H and C-N to the north, C-I to the east, M-L to the south, and R-MF to the west.

Properties along this stretch of CT Route 1 (East Main Street) are used for a variety of purposes including medium-to-high density multifamily, retail, commercial and industrial uses. The site is less than a mile from the Stamford Transportation Center and the Stamford Town Center. It is well served by local bus routes and sidewalk connections to the surrounding community. The eastern entrance to the Urban Transitway, which serves as a connection between the East Side neighborhood and the Stamford Transportation Center, is located just two blocks from the Site.

In recent years, there has been a concerted effort to remove some of the less "neighborhood friendly" uses and replace them with housing and updated commercial storefronts. Glenview House and Eastside Commons, both located on the opposite side of East Main Street from the Site, and the shopping center at the northwest corner of the intersection of Lafayette Street and East Main Street are examples of this effort. However, other properties in the immediate area which require significant capital investment, including the subject site, remain.

## 4. Project Area/Development Site

The Site is approximately 1.15 acres with fontange on East Main Street, Lafayette Street, and North State Street. It is improved with five separate buildings: three multi-family buildings, one
single-family home, and one multi-tenant commercial building. The residential buildings are noted on the tax card as being constructed between 1875 and 1900, though little, if any, of the original character of the buildings appears to remain. The buildings are in need of significant aesthetic, safety and functional improvements and appear out of place among the ongoing redevelopment of this stretch of Stamford's East Side. Much of the Site's access and parking is nonconforming with oversized curb-cuts, perpendicular spaces backing directly out into both Lafayette Street and East Main Street, creating unsafe conditions for pedestrians and motorists, and nonconforming parking and trash enclosure directly abutting the N. State Street sidewalk.

The Site has been identified by the City and neighborhood as a target for redevelopment. Several supportive letters from neighbors are being submitted under separate cover, including a detailed and highly supportive letter from the East Side Partnership that puts the existing Site, and welcomed redevelopment thereof, in an appropriate context.

## 5. Proposed Development

The proposed development project consists of up to 130 apartments and approximately 3,000 square feet of ground floor retail/flex amenity space along the East Main Street frontage. This modified proposal increases the amount of housing by more than $50 \%$ over the Recent Approval, thereby helping to address the serious housing shortage facing Stamford today.

## A. Unit Mix

The current unit mix includes 44 studio, 55 one-bedroom and 31 two-bedroom apartments. This is a significant change from the Prior Approval which included 42 2-BR units comprising 50\% of the development and no studios. The Applicant believes the more diversified mix better addresses the market demand and should reduce parking demand as well. The exact unit size and mix may change slightly depending on market conditions and other factors which may arise as the plans develop.

## B. Below Market Rate Housing

The MX-D infill zone includes a base $10 \%$ Below Market Rate (BMR) unit requirement which equates to 13 of the 130 proposed apartments. Because 17 units of existing "Market Rate Affordable Housing" will be removed from the Site to enable its redevelopment, an additional BMR unit, affordable at $65 \%$ of Area Median Income, is required for every 2 units removed. This equates to an additional 8.5 BMR units ( $@ 65 \%$ AMI), or 5.1 BMR units ( $@ 50 \%$ AMI) based on the equivalencies in Section 7.4. Based on this calculation, the total BMR obligation for the project is 18 units ( $@ 50 \%$ AMI). The Applicants intend to satisfy this requirement by providing 5 studio, 8 one-bedroom and 5 two-bedroom units onsite. A special permit request pursuant to Sections 7.4.C.1.g and 7.4C.1.k is included to facilitate this aspect of the proposal.

## C. Site/Building Composition \& Features

The building has been designed with its main lobby and resident entrance at the corner of East Main and Lafayette Street and 2 potential retail storefronts completing the East Main Street frontage. Four additional levels of apartments continue above with a landscaped rooftop providing approximately $8,700 \mathrm{sf}$ of open space for use by residents. An additional $1,500 \mathrm{sf}$ of usable open space is located on top of the sub-grade garage along the N. State Street frontage.

While no designated child play area is specifically required in this design district, approximately 2,000 square feet has been set aside on the roof for this purpose and an alternative interior space is also contemplated on the ground floor.

The design depicts a clearly defined "base, middle, and top" with TerraNeo finish Dryvit at the base, brick patterned façade on the middle floors and Azek trim and decorative banding and paneling at the top floor. The Vertical window line and tower element help to accentuate the prominent building corner at the intersection of E. Main and Lafayette. The roofline then drops one story as the building continues down Lafayette Street. A clean, sophisticated color palette has been employed which is complementary to the attractive renovations to the retail center across East Main Street. White and dark grey brick colors alternate to break up the building length, along with 1 ' deep façade articulations. A partial $5^{\text {th }}$ floor extends along the interior of the building approximately $30^{\prime}$ from the Lafayette building face. The short N. State Street frontage mimics the design of Lafayette Street, with a concrete base for the exposed portion of the basement garage level.

The sole vehicular access will be at the south end of the site on North State Street. Parking is provided beneath and behind the building at grade. Parking is provided in accordance with Sections 9.C.4.i, 9.C.5.b.5 and 12.D, with a total of 148 striped spaces and two shared vehicle spaces (accounting for an additional 8 spaces), or 156 total onsite parking spaces (a ratio of 1.20 parking spaces per unit, and 0.98 spaces per bedroom). This is an improvement from the prior approval which provided onsite a ratio of 1.0 spaces per unit and 0.67 spaces per bedroom. The applicant is requesting, by Special Permit pursuant to Section 12.D.1.d, to provide the remaining 17 required spaces on an as-needed basis. Such spaces may be provided onsite through the use of valet and/or vehicle stackers, or, similar to the prior approval, at an offsite location within 500 ' of the property. While there is no parking requirement for retail in the MX-D zone, depending on the ultimate retail tenant and residential demand, some spaces may be shared for retail use outside of the peak residential demand times. Should this become desirable, the Applicants would return to the Zoning Board for administrative approval of a Shared Parking Agreement, in conformance with Section 12.I of the Stamford Zoning Regulations, to the extent necessary.

The site frontages will be lined by a continuous sidewalk with overall widths varying from 10, on the Lafayette and N. State Street sides to 15 ' along East Main Street. This represents a major improvement over the wide driveways and unmitigated head-in parking spaces that make up the current frontages. One streetside loading space can be created along Lafayette Street, and metered street parking will be maintained along the rest of the frontage. At least one of the street spaces is intended to be designated for short term drop-off and pick-up only, subject to approval by the Transportation Advisory Committee. This will encourage and accommodate the use of parking demand management strategies like Uber, Lyft and/or other ride share opportunities.

## D. Construction Timing

Site work would likely begin in fall of 2022 with an 18-month construction schedule to be completed by the Spring of 2024.
E. Conformity with Stamford Zoning Regulations, Master Plan and East Main Street
Corridor Neighborhood Plan

## Master Plan

Category 9 (Urban Mixed-Use) of the Master Plan contemplates "a full array of uses including high-density residential uses as the primary use in this category, supported by a dynamic mix of neighborhood retail and service uses, office, and recreational uses serviced by mass transportation and quality streetscapes that enhance connections between the Downtown and outlying neighborhoods."

## Zoning Regulations

As previously noted, the majority of the Site was rezoned to MX-D in 2018 and 2021. The current proposal seeks to extend that designation to the southern lot. The MX-D infill zone, which "promotes the creation of new residential dwelling units in under-utilized areas," is the perfect tool to implement the Master Plan's goals for this neighborhood. Moreover, the proposed development meets all of the requirements of the MX-D Infill Zone. Please refer to the Zoning Data Chart for additional zoning information.

## East Main Street Corridor Neighborhood Plan

The proposed development and related streetscape improvements serve to implement several of the Plan's goals, including:

- Eliminate or reconfigure non-standard front yard parking lots;
- Enhance facades and business signage;
- Reduce widths and quantity of curb cuts;
- Remove concrete curbs, replace with granite;
- Add ornamental pedestrian scale lighting;
- Provide uniform and continuous concrete sidewalk with decorative/amenity band;
- Separate sidewalk from curb with lawn strip or decorative pavement and street trees;


## 6. Action Items

To facilitate the development, the Applicants have the following applications.
A. Zone Change (from CI to MX-D) related to 15 Lafayette Street;
B. General Development Plan and Final Site \& Architectural Plans and Requested Uses, including the following specific requests/findings of the Zoning Board;
i. Pursuant to Sections 9.C. 3 and 9.C.6.a, in order to maximize flexibility and potential success of the retail space, Applicants request approval for all retail and restaurant type uses allowed in the zone. ${ }^{1}$
ii. Pursuant to Section 9.C.4.h, Applicant requests approval of the proposed relationship of yard requirements and separation of Structures on the site.
iii. Pursuant to Section 9.C.5.b.3, Applicant requests approval of the proposed location and design of required Usable Open Space.

[^1]iv. Pursuant to Section 12.D.1.e provision of two (2) shared vehicles at a ratio of four (4) parking spaces each constituting less than $10 \%$ of required parking.
C. Special Permits pursuant to the following sections of the Zoning Regulations and specific requests:
i. Section 7.4.C.1.g, in conjunction with 7.4.C.1.k, Applicants request Special Permit approval to provide 18 BMR units at $50 \%$ of AMI.
ii. Section 7.Q, Applicant requests the child play area to be located on the rooftop.
iii. Pursuant to Section 12.D.1.d and 19.F, Applicant requests that seventeen (17) spaces, constituting less than $10 \%$ of the parking requirement, be provided on an asneeded basis.

## 7. Conclusions

The proposed development embodies nearly all of the applicable policy goals of both the Urban Mixed-Use Master Plan Category, MX-D Zone and East Main Street Corridor Neighborhood Plan by "providing a mix of uses complimentary and supportive of the Downtown" with "high-density residential uses as the primary use" with "quality streetscapes that enhance connections between the Downtown and outlying neighborhoods of the City." It will revitalize an important stretch of East Main Street and alleviate unsafe parking and curb cuts along multiple street frontages.

## 8. Statement of Findings

I. The above referenced specific Special Permit requests are integral to the development project as a whole. Thus, for purposes of demonstrating compliance with the standards and conditions below, the entire development proposal is considered. The Applicants submit that all applicable criteria contained in Stamford Zoning Regulations Article V, Section 19.C. 2 are met for the following specific reasons:
a. Special Permits shall be granted by the reviewing board only upon a finding that the proposed use or structure or the proposed extension or alteration of an existing use or structure is in accord with the public convenience and welfare after taking into account, where appropriate:

1) the location and nature of the proposed site including its size and configuration, the proposed size, scale and arrangement of structures, drives and parking areas and the proximity of existing dwellings and other structures.

The proposed development is appropriately located within a mixed residential and commercial neighborhood and the Urban Mixed-Use Master Plan Category. The proposed building is compatible in scale and style with the surrounding area, particularly the more recently constructed Glenview House and Eastside Commons developments. The proposed setbacks and arrangement of buildings are appropriate for infill development and serve to activate pedestrian street frontages while maintaining ideal sidewalk width, adequate parking and open space. All parking is appropriately accommodated onsite below and
behind the proposed building. A single access drive on N. State Street greatly improves the existing unsafe condition of multiple curb cuts with vehicles backing into rights-of-way.
2) the nature and intensity of the proposed use in relation to its site and the surrounding area. Operations in connection with special permit uses shall not be injurious to the neighborhood, shall be in harmony with the general purpose and intent of these Regulations and shall not be more objectionable to nearby properties by reason of noise, fumes, vibration, artificial lighting or other potential disturbances to the health, safety or peaceful enjoyment of property than the public necessity demands.

Category 9 (Urban Mixed-Use) of the Master Plan contemplates "a full array of uses including high-density residential uses as the primary use in this category, supported by a dynamic mix of neighborhood retail and service uses, office, and recreational uses serviced by mass transportation and quality streetscapes that enhance connections between the Downtown and outlying neighborhoods." The proposed development fits within this category and fulfills the policy goals of the neighborhood. The proposed structures are similar in scale and design to the surrounding multifamily and commercial buildings and will significantly improve upon existing conditions. Improvements to parking and streetscapes also serve as an enhancement of the property and surrounding neighborhood with added health and safety benefits. Thus, the Applicants submit that the proposed development is appropriate for the neighborhood, will increase property values and will not be objectionable to nearby properties.
3) the resulting traffic patterns, the adequacy of existing streets to accommodate the traffic associated with the proposed use, the adequacy of proposed off-street parking and loading, and the extent to which proposed driveways may cause a safety hazard, or traffic nuisance.

Traffic can be safely and adequately accommodated on the surrounding streets and the residential use will not adversely impact any peak traffic demand. The elimination of existing curb cuts and formalization of existing street parking on both Lafayette Street and E. Main Street will serve to further improve safety of both motorists and pedestrians. Parking is safely and adequately provided onsite.
4) the nature of the surrounding area and the extent to which the proposed use or feature might impair its present and future development.

The surrounding area includes a variety of residential, commercial, industrial and retail uses. The proposed residential use is compatible with these uses and will serve as a further catalyst for others to invest in their properties. It will also place people on the streets thereby increasing the patronage of nearby retail and service establishments and encourage further redevelopment.
5) the Master Plan of the City of Stamford and all statements of the purpose and intent of these regulations.

The site lies within Master Plan Category 9 (Urban Mixed-Use) and meets the goals of the Master Plan, as previously stated. Other goals of the Master Plan that are advanced by this proposal include:

- 6C.2: Promote development of a variety of housing types.
- 6C.5: Encourage increased density along transit corridors and within Downtown through land-use regulations and developer incentives.
- ES1.2: Promote new, higher-density mixed-use development along the Stamford Urban Transitway
- ES2.1: Promote context-sensitive residential and mixed-use development that relates well in scale and design to the surrounding residential areas.
- ES2.3: Promote efforts to formalize East Main Street as a key gateway into Stamford...and overall streetscape improvements such as landscaping, building façade enhancements; and aesthetically attractive streetlights.
- ES3: Promote new retail opportunities and services for the neighborhood.
- ES4: Improve mobility and circulation.
II. Pursuant to Section 19.C.2.a of the Zoning Regulations, the Zoning Board must find that the proposed use or structure or the proposed extension or alteration of an existing use or structure is in accord with the public convenience and welfare.

The Applicants are proposing to raze the existing residential and commercial improvements on the property and construct a new mixed-use building with associated parking and streetscape improvements in its place. The existing improvements are in disrepair and out of scale and character with the changing neighborhood. The replacement of these improvements with the proposed building will increase the tax base and significantly improve the appearance of one of the most visible thoroughfares in Stamford. The active ground floor frontage and 130 new residential units, both affordable and market rate, will enliven this long-underutilized site and breathe new life into the East Side neighborhood. This housing is sorely needed to help address the significant housing crisis facing our community today. For all of these reasons, the Applicants submit that the proposal, and the associated Special Permit requests which are inextricably intertwined, are in accord with the public convenience and welfare.
III. Pursuant to Section 9.C. 2 of the Stamford Zoning Regulations, additional land may be designated and incorporated as an integral part of the MX-D Development at the discretion of the Zoning Board, provided that the additional land is contiguous ... and that the incorporation and Development of said property is consistent with the standards and objectives of the MX-D District.

The Applicants confirm that 15 Lafayette Street is contiguous to the previously approved development site and, as detailed in the preceding narrative, the proposed development is consistent with the standards and objectives of the MXD District.
IV. Pursuant to Sections 9.C.5.b. 2 of the Stamford Zoning Regulations, there shall be no net increase in commercial uses.

According to the Stamford Tax Assessor Records, the existing commercial building on the Site is $4,209 \mathrm{sf}$. The proposed commercial square footage is approximately $2,950 \mathrm{sf}$. Thus, there will be no net increase in commercial uses.
V. Pursuant to Sections 9.C.5.b. 3 of the Stamford Zoning Regulations, a minimum of 75 sf of Usable Open Space per dwelling unit is required for sites with at least $50 \%$ of the street frontage either vacant or used for parking.

Today, over $50 \%$ of the street frontage is either vacant or used for parking. Thus, the proposal qualifies for the 75 sf Usable Open Space standard.
VI. Pursuant to Sections 9.C.4.c and 9.C.5.b.5 of the Stamford Zoning Regulations, the Zoning Board must find that a minimum of $2 / 3$ of the parking structure is integrated within the building and/or screened/hidden from sensitive views.

The proposed plan effectively uses the multiple street frontages, sloping grade, and at-grade landscaping to provide adequate parking without impacting sensitive pedestrian views. The parking is tucked behind the building frontage and beneath the building. Any surface parking not located directly beneath the building will be significantly screened from public view by the building and landscaping and does not exceed the $1 / 3$ maximum.
VII. Pursuant to Sections 9.C.4.h of the Stamford Zoning Regulations, the Zoning Board must find that the proposal provides for adequate light, open space, screening, landscape, safety and privacy for existing and proposed dwelling units.

The proposal has been reviewed by the adjacent neighbors to ensure there are no adverse impacts on their respective properties. All parties are supportive of the project and believe it will be beneficial for the entire neighborhood. The proposed setbacks and arrangement of buildings provide adequate light, and the planned landscape and streetscape improvements will improve open space, safety and privacy conditions for adjacent properties.

# PARKING \& TRANSPORTATION DEMAND MANAGEMENT PLAN PROPOSED RESIDENTIAL / MIXED-USE DEVELOPMENT 

East Main Street, Lafayette Street \& N. State Street Stamford, CT

February 8, 2022

This plan has been prepared in support of a proposed residential / mixed-use development encompassing approximately 1.15 acres of land in Master Plan Category 9 (Urban - Mixed Use) and the MX-D Zone with frontage on E. Main Street, Lafayette Street and N. State Street.

## PROJECT OVERVIEW

The Applicant proposes to redevelop the existing site which currently contains a mix of retail and multi-family residential buildings with associated surface parking. The proposed development will consist of 130 rental apartments in a 5 -story building with up to 2,950 square feet of new retail space. Apartments will be comprised of 44 studio, 55 one-bedroom and 31 two-bedroom units. The new building will include 82 striped spaces in a lower-level garage and 68 surface spaces for a total of 150 striped spaces. A net addition of 8 street parking spaces ( 15 total) will also be created along the site frontage through the closing of several existing curb cuts. The proposed development will include modern tenant amenities, including a lounge and communal rooftop terrace, and onsite bicycle storage space.

## EXISTING DATA

Section 9.C.4.i of the Stamford Zoning Regulations states that no parking is required for any retail uses (provided such uses are less than $10 \%$ of the total proposed floor area) and requires 1.25 spaces for residential units of 1 bedroom or less and 1.5 spaces for 2-bedroom units. With the proposed unit mix, the parking standard requires 171 spaces for residents of the site. No additional off-street parking is required. By Special Permit pursuant to Section 12.D.1.d of the regulations, up to $10 \%$ of the required parking ( 17 spaces) may be provided on an "as needed" basis. This leaves 154 required spaces.

The site is located approximately one mile from both the Stamford Transportation Center and the Glenbrook Train Station with multiple bus lines running across the E. Main Street frontage. The site has a Walk Score of 87 "Very Walkable" and a Transit Score of 62 "Good Transit". Given the site's urban location and proximity to multiple transit nodes and many retail, restaurant and service establishments, these parking standards are appropriate.

The current plan includes a total of 148 self-parking spaces available to residents with an additional 2 spaces allocated to shared vehicles. Collectively, this parking arrangement is expected to easily meet/exceed residential demand.

## PARKING OPERATIONS

Parking operations will be actively supervised by the onsite property management employees. A total of 148 full time self-parking spaces are proposed onsite. Two shared vehicles will also be provided, for a total of 156 effective parking spaces.

Vehicle access will be provided from the sole curb cut on N. State Street which will lead to both the rear surface spaces and below grade garage spaces.
To comply with code, 13 handicapped-accessible spaces are proposed. The regulations also require 19 Class A and 13 Class B bicycle spaces. A total of 72 bicycles can be stored on vertical wall racks within the secure garage. Up to 14 spaces, to be designated as charging stations for electric vehicles, are proposed, exceeding the minimum requirement of 12 .
The site will operate generally as a self-park facility. Property managers will monitor and document parking usage at regular intervals during and after the initial "lease-up" period to ensure the regular tenant demand is being met.

Patrons of the retail space may have shared use of the surface parking with specific hours of use to be determined once tenants have been secured. Should this become desirable, the Applicant would return to the Zoning Board for administrative approval in conformance with Section 12.I of the Stamford Zoning Regulations, to the extent necessary. All spaces will be signed and enforced accordingly.

## ADDITIONAL PARKING CAPACITY

While the Applicant is confident the parking demand will be met with the provided spaces, should it be determined during the reporting period that the additional 17 "as needed" spaces are in demand, one or more of the following strategies to increase parking capacity could be employed:

1. Vehicle Stackers could be implemented within portions of the surface lot.
2. Offsite parking could be attained by leasing spaces at one of several nearby retail/commercial establishments within 500' of the site.

## DEMAND MANAGEMENT STRATEGIES

## Shared Vehicles

The developer is proposing to provide two (2) "shared vehicles" (i.e. ZipCar). Shared vehicles provide residents with a useful amenity and viable alternative to private vehicle ownership.

## Unbundled Parking

The rent structure of the residential units is proposed to be "unbundled" with regard to parking. Apartments will not automatically include a "free" parking space (which has been demonstrated to encourage vehicle ownership and usage). The separate charge for spaces provides an incentive for residents to consider an alternative to individual car ownership and
usually results in lower demand. It also allows those residents without cars to enjoy a lower cost of living (rather than pay for parking they do not need).

## Public Transportation

The proposed development is located within a mile of both the Stamford Transportation Center (which provides access to both Amtrak and Metro North Railroad services, as well as Greyhound and Peter Pan buses) and the Glenbrook Train Station.

Connecticut Transit has multiple stops in and around the downtown area, including the 341 (Stamford-Norwalk), 342 (East Main and Stamford Transportation Center), and 344 (STC, Glenbrook Station, Noroton Heights Station) which stop directly across from the site on East Main Street. Additional lines to other parts of Stamford make stops at nearby Grove Street.

Additionally, Uber, Lyft, Metro Pool and NuRide provide corporate and personal ride sharing programs that are currently utilized by residents in the downtown area, as well as the suburbs.

## Bicycle Storage

To encourage ridership as an alternate means of travel, and as an amenity to building residents, the applicant is providing well above the minimum number of bicycle parking spaces - up to 72 spaces where a total of 32 (Class A and B combined) are required. Additional bike racks may be installed on the ground level of the site if needed.

## LOADING

An on-street loading space can be accommodated along the Lafayette Street frontage. Additionally, one or more of the onsite surface parking spaces can also be used for loading purposes outside of peak residential demand times. All spaces will be signed and enforced accordingly.

819-833 E. Main Street and 15, 27 \& 29 Lafayette Street
Special Permit and Site \& Architectural Plans

Zoning Data Chart - MXD Infill

| Standard | Permitted <br> MX-D Infill | Prior <br> Approval | Proposed <br> GDP/FSP | Notes |
| :--- | :---: | :---: | :---: | :--- |
| Min Lot Area | 20,000 | 34,562 | 50,237 | Complies.§9.C.5.a.2 <br> Proposed area includes acquisition of 15 Lafayette Street <br> Max Building Stories$\quad \mathrm{n} / \mathrm{a}$ |

## Notes

Per plans provided by Do H. Chung \& Partners and by DiMarzo \& Bereczky

## Zoning Data (cont.)

819-833 E. Main Street and 15, 27 \& 29 Lafayette Street Special Permit and Site \& Architectural Plans

## Parking Calculation

| Residential (by unit) |  | Req. per Unit | (total req) | Provided | Notes |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Studio (market) | 44 | 1.25 | 55.0 | 156 | Complies. §9.C.4.i and §9.C.5.b.5 <br> -Special Permit per §12.D.1.d to permit up to $10 \%$ of required parking to be provided on an "as needed" basis. <br> - 148 self-park spaces +2 shared vehicles $(8$ spaces $)=156$ parking spaces <br> - Additional spaces (off-site, stackers, etc.) to be provided as outlined in Parking Management Plan. <br> $\cdot$ Prior approval provided 1.0 spaces onsite with 0.25 additional offsite requirement |
| 1-BR (market) | 55 | 1.25 | 68.8 |  |  |
| 2-BR (market) | 31 | 1.50 | 46.5 |  |  |
| TOTAL | 130 | - | 170.3 |  |  |
| Amount to be provided "as needed" |  |  | $\underline{-17.0}$ |  |  |
| Minimum Onsite Requirement |  |  | 153.3 |  |  |

## Below Market Rate

Required units:

$$
\begin{array}{rlc}
130 \text { (total proposed dwelling units) } \times 10 \%=13.0(50 \% \text { AMI }) & \text { [§7.4 - Table 7.4.1] } \\
17 \text { (existing market rate aff) } \div 2=8.5(65 \% \text { AMI) } \times 0.6 \text { (equivalency })=5.1(50 \% \text { AMI }) & \text { [§7.4.c.1.n.2] } \\
13.0+5.1=18.1(50 \% \text { AMI }) & 14 \%
\end{array}
$$

| Total Units |  | AffordabilityLevel(AMI) | Conversion <br> Rate <br> (per §7.4) <br> 0.33 | Required BMR |  |  | Proposed BMR |  | Notes |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Number of Units |  | Equivalency | Number | Equivale |  |
| Studio | 44 |  |  | 10\% | 4.4 | 1.47 | 5 | 1.67 | Complies. <br> Special Permit Request per §7.4.C. 1 subsections (g) and (k) |
| Studio |  |  | 65\% | 0.20 | $\mathrm{n} / \mathrm{a}$ | 2.9 | 0.58 |  |  | 0.00 |
| 1BR | 55 | 50\% | 0.50 | 10\% | 5.5 | 2.75 | 8 | 4.00 |  |
|  |  | 65\% | 0.30 | $\mathrm{n} / \mathrm{a}$ | 3.6 | 1.08 |  | 0.00 |  |
| 2BR | 31 | 50\% | 1.00 | 10\% | 3.1 | 3.10 | 5 | 5.00 |  |
|  |  | 65\% | 0.60 | $\mathrm{n} / \mathrm{a}$ | 2.0 | 1.22 | 0 | 0.00 |  |
| TOTAL | 130 |  |  |  | 21.5 | 10.19 | 18 | 10.67 |  |

## Drawing List

819-831 East Main Street \& 15, and 27-29 Lafayette Street Zone Map Change, GDP, Special Permit, and Final Site and Architectural Plan Application February 8, 2022

## Sheet \#

## Civil

| PTS | Property \& Topographic Survey | DiMarzo \& Bereczky | $12 / 14 / 2021$ |
| :--- | :--- | :--- | :--- |
| ZLS | Zoning Location Survey | DiMarzo \& Bereczky | $02 / 03 / 2022$ |
| C-1 | Site Plan | DiMarzo \& Bereczky | $02 / 03 / 2022$ |
| C-2 | Utility \& Grading Plan | DiMarzo \& Bereczky | $02 / 03 / 2022$ |
| C-3 | Traffic Signage \& Pavement Plan | DiMarzo \& Bereczky | $02 / 03 / 2022$ |
| C-4 | Erosion \& Sediment Control Plan | DiMarzo \& Bereczky | $02 / 03 / 2022$ |
| C-5 | Notes \& Details | DiMarzo \& Bereczky | $02 / 03 / 2022$ |
| C-6 | Details-1 | DiMarzo \& Bereczky | $02 / 03 / 2022$ |
| C-7 | Details-2 | DiMarzo \& Bereczky | $02 / 03 / 2022$ |
| C-8 | Details-3 | DiMarzo \& Bereczky | $02 / 03 / 2022$ |
| C-9 | Low Impact Development Plan | DiMarzo \& Bereczky | $02 / 03 / 2022$ |

## Architectural

CS-1
A-001
A-101
A-102
A-103
A-104
A-105
A-106
A-107
A-201
A-202
A-203
A-301
A-302

## Landscape

LP. 1 Landscape Plan

## Lighting

SL-1
Lighting Plan

Cover Sheet
Overall Site \& Grading Plan Open Space Areas
Basement Plan
First Floor Plan
Second Floor Plan
Third Floor Plan
Fourth Floor Plan
Fifth Floor Plan
Roof Plan
Typ. Bldg. Elevations
Typ. Bldg. Elevations
Typ. Bldg. Elevation
Typ. Sections
Typ. Sections

Environmental Land
Solutions, LLC

Illuminate
02/03/2022
Prepared by
Date

12/14/2021
02/03/2022
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02/04/2022
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## Owner List

819-831 East Main Street \& 15, and 27-29 Lafayette Street
Zone Map Change, GDP, Special Permit, and Final Site and Architectural Plan Application
February 8, 2022

| Property Address: | 27, 29 Lafayette Street and 821, 825, 827 East Main Street |
| :--- | :--- |
| Owner Name: | 819 East Main Street, LLC |
| Owner Address: | 2 Armonk Street <br> Greenwich, CT 06930 |
|  |  |
| Property Address: | 831 East Main Street |
| Owner Name: | 831-833 East Main Street, LLC <br> 1156 Newfield Avenue |
| Owner Address: | Stamford, CT 06905 |
|  |  |
| Property Address: | 15 Lafayette Street |
| Owner Name: | New Star Lafayette LLC |
| Owner Address: | 19 High Ridge Road \#8120 |
|  | Stamford, CT 06905-9993 |



## MX-D DESIGNATION CRITERIA EXHIBIT 819 EAST MAIN STREET STAMFORD, CT



Pursuant to Section 9.C.5.a.

1. Min. $25 \%$ of Site area used for commercial purposes or vacant.
1.1. Provided 33\%土
2. Min Lot Areas of 20,000 square feet.
2.1. Provided 50,237 square feet (per Survey provided by DiMarzo \& Bereczky dated 12/14/2021)
3. Min. frontage 50'.
3.1. Provided 624士 LF.
4. Min. $50 \%$ of site frontage either vacant of used for parking.
4.1. Provided 351' (56\%).

REDNISS
\&MEAD

## General Property Description <br> 15, 27 \& 29 Lafayette Street; 821, 825, 827 \& 831 East Main Street

January 21, 2022
Block \#: 104
Area: $\quad 50,237 \pm \mathrm{SqFt}$

All those parcels of land commonly known as 15 Lafayette Street (001-1420), 27 Lafayette Street (001-7662), 29 Lafayette Street (001-7663), 821 East Main Street (001-7666), 825 East Main Street (001-7664), 827 East Main Street (002-5499) and 831 East Main Street (000-4639); located in the City of Stamford, and generally described as follows:

Beginning at the intersection of the southerly side of East Main Street and the easterly side of Lafayette Street, said land is bounded by the following:

Northerly: $\quad 150 \pm$ by the southerly side of East Main Street;

Easterly: $\quad 284^{\prime} \pm$ by the land n/f of 837-845 East Main ST Assoc (835 East Main Street);

Southerly: $\quad 187^{\prime} \pm$ by the northerly side of North State Street;

Westerly: $297^{\prime} \pm$ by the easterly side of Lafayette Street to the point of beginning;


# Zone Change Description <br> 819 East Main Street 

January 21, 2022
Block \#: 104
Area: $\quad 21,980 \pm \mathrm{SqFt}$ (includes $6,854 \pm \mathrm{SqFt}$ of portion of Lafayette St and North State Street right-of-ways along site frontage, each in part).

DESCRIPTION OF AREA OF ZONE CHANGE FORM C-1 (Intermediate Commercial District) TO M-XD (MIXED USE DEVELOPMENT DISTRICT):

Parcel of land commonly known as 15 Lafayette Street (001-1420); located in the City of Stamford, and generally described as follows:

Beginning at a point at the intersection of the centerline of Lafayette Street and centerline of North State Street, said land is bounded by the following:

Westerly: $\quad 119^{\prime} \pm$ by the centerline of Lafayette Street;

Northerly: $\quad 199^{\prime} \pm$ by the land n/f of 819 East Main Street LLC (27 Lafayette Street), and a portion of Lafayette Street, each in part;

Easterly: $\quad 79^{\prime} \pm$ by the land n/f of 837-845 East Main ST Assoc (835 East Main Street), and a portion of North State Street, each in part;

Southerly: $\quad 218^{\prime} \pm$ by the centerline of North State Street, to the point of beginning

City of Stamford Planning \& Zoning Boards c/o Ralph Blessing, Land Use Bureau Chief 888 Washington Boulevard
Stamford, CT 06901

Re: 831 E. Main Street - Stamford, CT
Dear Mr. Blessing:
This letter serves to authorize the firms of Redniss \& Mead Inc. (with offices at 22 First in Stamford, CT), to act as our agents in connection with the preparing, filing, and processing of any and all applications required for Planning and Zoning approvals relating to the above referenced properties.

Thank you for your acknowledgement of said authority.


City of Stamford Planning \& Zoning Boards c/o Ralph Blessing, Land Use Bureau Chief 888 Washington Boulevard
Stamford, CT 06901

## Re: 15 Lafavette Street-Stamford, CT

Dear Mr. Blessing:
This letter serves to authorize the firms of Redniss \& Mead Inc. (with offices at 22 First in Stamford, CT), to act as our agents in connection with the preparing, filing, and processing of any and all applications required for Planning and Zoning approvals relating to the above referenced properties.

Thank you for your acknowledgement of said authority.

Sincerely


Anuj L Gupta
Member, Manager
New Star Lafayette LLC

City of Stamford Planning \& Zoning Boards c/o Ralph Blessing, Land Use Bureau Chief
888 Washington Boulevard
Stamford, CT 06901

Re: 821, 825, 827 \& 831 E. Main Street and 15, 27-29 Lafayette Street - Stamford, CT
Dear Mr. Blessing:
This letter serves to authorize the firms of Redniss \& Mead Inc. (with offices at 22 First in Stamford, CT), to act as our agents in connection with the preparing, filing, and processing of any and all applications required for Planning and Zoning approvals relating to the above referenced properties.

Thank you for your acknowledgement of said authority.




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$\frac{\text { (NORTH STATE ST) }}{\text { N.T.S. }}$

| NOTES \& DETAIL S DEPICTING <br> 821, 825, 827 \& 831 EAST MAIN STREET 15, 27 \& 29 LAFAYETTE STREET STAMFORD, CT PREPARED FOR <br> 819 EAST MAIN STREET, LLC |  |
| :---: | :---: |
| DATE: $2 / 03 / 2022$ <br> JOB NO. 173 | SCALE: AS NOTED |
|  |  |
|  | C-5 |





PLAN


LATERAL CONNECTION TO $\frac{\text { SANITARY SEWER }}{\text { NTS. }}$


DETAILS - 2

## 821, 825, 827 \& 831 EAST MAIN STREET

 15, 27 \& 29 LAFAYETTE STREET 819 EAST MREPARED FOR STREET, LLC

| DATE: $2 / 03 / 2022$ <br> JOB NO. 173 | SCALE: AS NOTED |
| :---: | :---: |
|  |  |
|  | C-7 |



SIDEWALK WITH GRASS PLOT TYPICAL SIDEWALK DETAILS
ScALE $1 / 2^{\prime \prime}=r^{1}-0^{\prime \prime}$


NOTES

1. ALP Cubang To


DETAILS - 3

| DETAILS - 3 DEPICTING <br> 821, 825, 827 \& 831 EAST MAIN STREET 15, 27 \& 29 LAFAYETTE STREET <br> STAMFORD, CT PREPARED FOR <br> 819 EAST MAIN STREET, LLC |  |
| :---: | :---: |
| DATE: $2 / 03 / 2022$ <br> JOB NO. 173 | SCALE: AS NOTED |
|  | $\begin{aligned} & \hline \text { DIMARZO \& } \\ & \text { BERECLKY } \end{aligned}$ |
|  | C-8 |



## THE LAFAYETTE

PROJECT OWNER
819 E. MAIN ST. STAMFORD, CT 06902
WELLBUILT COMPANY
2 ARMONK STREET
GREENWICH, CT 0683 (866)846-4874

PROJECT TEAM
ARCHITECT
DO H. CHUNG and PARTNERS ARCHITECTS PLANNER ${ }^{105}$ EDEDFRRD ST, STAMFORD, CONNECTICUT 0699 . 203.357.0089

## WEST ELEVATION (FROM LAFAYETTE ST.)



KEY PLAN =-















Office Use only

| Received Date |  |
| :--- | :--- |
| Application \# |  |

1. Address of Development

| Address1 | 819 E Main St |
| :--- | :--- |
| Address2 |  |


| 2. Applicant Info |  |
| :--- | :--- |
| Name |  |
| Company |  |
| Address1 |  |
| Address2 |  |
| Email |  |
| Phone |  |


| 3. Owner info |  |
| :--- | :--- |
| Name |  |
| Company | Wellbuilt Co |
| Address1 |  |
| Address2 | Greenwich CT |
| Email |  |
| Phone |  |

4. Mark one

| 4. Mark one |  |
| :---: | :--- |
| $X$ | First Submission |
|  | Second Sumbission |

## Office

 Use Only
## POINT CALCULATIONS

| Max <br> Potential | Eligible | Claimed | Notes <br> (Indicate the plan/document where relevant information is <br> located) |
| :---: | :---: | :---: | :---: |

## 5. Building Health

| BH1 - Indoor Air Quality <br> 2nd Submission ONLY | 1 | 0 | 0 |  |
| :--- | :---: | :---: | :---: | :--- |
| BH2 - Low Emitting Materials <br> 2nd Submission ONLY | 1 | 0 | 0 |  |
| BH3 - Moisture Control | 1 | 1 | 1 | Will provide HVAC plans showing humidity mitigation <br> measures |
| BH4 - Daylighting <br> LARGE PROJECTS ONLY | 1 | 1 | 0 |  |
| BH5 - Window Shading <br> 2nd Submission only | 1 | 0 | 0 |  |
| BH6 - Operable Windows | 1 | 1 | 1 | Windows will be operative |
| BH7- Active Design | 1 | 1 | 0 |  |
| BH8- Fitness Equipment <br> LARGE PROJECTS ONLY | 1 | 1 | 0 |  |

## 6. Energy Usage

| Office Use Only |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| POINT CALCULATIONS | Max Potential | Eligible | Claimed | Notes <br> (Indicate the plan/document where relevant information is located) |
| EU1 - Energy Usage 2nd Submission ONLY | 7 | 0 | 0 |  |
| EU2 - Submetering | 2 | 2 | 2 | Will have individual meters for each residential unit and each commercial tenant |
| EU3 - Cool Roofs | 2 | 2 | 2 | Flat roof will have TPO membrane which should meet SRI requirements |
| EU4 - Exterior Lighting | 1 | 1 | 1 | Will provide lighting plan. All landscape lighting to be downlighting |
| EU5 - Interior Lighting | 1 | 1 | 1 | Will have timed lights to account for usage patterns |
| EU6 - Solar Ready Design <br> (NOT APPLICABLE FOR ADAPTIVE REUSE PROJECTS) | 2 | 0 | 0 |  |
| EU7 - Renewable Energy Production | 5 | 0 | 0 |  |
| EU8 - Combined Heat and Power | 3 | 3 | 3 | Energy Star compliant mechanical items to be used |

## 7. Land Use

| LU1 - Brownfield Redevelopment <br> 2nd Submission ONLY | 3 | 0 | 0 |  |
| :--- | :---: | :---: | :---: | :--- |
| LU2 - Redevelopment | 1 | 1 | 1 | Redevelopment of underutilized plots which contained <br> mostly construction fill and over-paved lots |
| LU3 - Adaptive Reuse | 2 | 0 | 0 |  |
| LU4 - Historic Preservation | 2 | 0 | 0 |  |
| LU5 - Mixed Use | 4 | 4 | 2 | Primary entrances within $1 / 4$ mile of 3 neighborhood services |
| LU6 - Transit-supportive Density | 2 | 2 | 1 |  |

## 8. Landscaping and Open Space

| LA1 - Green Roof | 2 | 2 | 0 |  |
| :--- | :---: | :---: | :---: | :--- |
| LA2 - Tree Preservation | 1 | 1 | 0 |  |
| LA3 - Tree Canopy | 1 | 1 | 0 |  |
| LA4 - Additional Landscaping | 1 | 1 | 0 |  |
| LA5 - Native Plants | 1 | 1 | 1 | Will provide landscaping plan with native plants |
| LA6 - Xeriscaping | 1 | 1 | 1 | Will provide landscaping plan with water-efficient plants |
| LA7 - Organic Land Care <br> 2nd Submission ONLY | 1 | 0 | 0 |  |


| Office Use Only |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| POINT CALCULATIONS | Max Potential | Eligible | Claimed | Notes <br> (Indicate the plan/document where relevant information is located) |
| LA8 - New Publicly Accessible Amenity Space LARGE PROJECTS ONLY: ONLY TO LOTS 1 ACRE AND LARGER IN MP 5, 9, 11 | 2 | 0 | 0 |  |

## 9. Mobility

| MO1 - Reduce Vehicle Travel <br> LARGE PROJECTS ONLY (non-residential uses only) | 2 | 0 | 0 |  |
| :--- | :---: | :---: | :---: | :--- |
| MO2 - Transit Proximity | 3 | 1 | 1 | Primary entrance within 800 feet of existing bus stop |
| MO3 - Shared Parking | 3 | 0 | 0 |  |
| MO4 - Incentivize Transit Use <br> LARGE PROJECTS ONLY (non-residential uses only) | 2 | 0 | 0 |  |
| MO5 - Car Share <br> LARGE PROJECTS ONLY | 2 | 2 | 1 | Will submit plan for car share program |
| MO6 - Shuttles or Support for Transit <br> LARGE PROJECTS ONLY | 2 | 2 | 0 |  |
| M07 - Bicycle Facilities | 1 | 1 | 0 |  |
| MO8 - Parking Availability | 1 | 1 | 1 | Will provide plans showing that parking provided is less than <br> $105 \% ~ o f ~ m i n ~ r e q u i r e d ~$ |
| M09 - Electric Vehicles | 1 | 1 | 0 |  |
| M010 - Unbundled Parking | 1 | 1 | 0 |  |
| MO11 - Road Infrastructure Contributions <br> LARGE PROJECTS ONLY 2nd Submission ONLY | 1 | 0 | 0 |  |
| M012 - Walkscore | 1 | 1 | 0 |  |

## 10. Resiliency

| RE1 - Development outside the floodplain | 3 | 3 | 3 | Outside of 500 year flood plain |
| :--- | :---: | :---: | :---: | :--- |
| RE2 - Flood Resiliency | 2 | 0 | 0 |  |
| RE3 - Building Resiliency | 2 | 2 | 1 | Backup generator will be provided for generator. |
| RE4 - Sea Level Rise | 2 | 2 | 0 | Cannot find map mentioned in reporting instructions |
| RE5 - Emergency preparation and continuation of <br> operations plan <br> LARGE PROJECTS ONLY 2nd Submission ONLY | 1 | 0 | 0 |  |

11. Urban Design

| Office Use Only |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| POINT CALCULATIONS | Max Potential | Eligible | Claimed | Notes <br> (Indicate the plan/document where relevant information is located) |
| UD1 -Block size | 1 | 1 | 1 |  |
| UD2 - Minimal Visual Impact of Parking | 2 | 2 | 1 | All surface parking blocked from view |
| UD3 - Building Orientation | 1 | 1 | 1 |  |
| UD4 - Building façade | 1 | 1 | 1 |  |
| UD5 -Building Materials | 1 | 1 | 0 |  |
| UD6 -Proximity of Building to Street | 1 | 1 | 1 |  |
| UD7 - Building Certification 2nd Submission ONLY | 3 | 0 | 0 |  |

## 12. Waste Management

| WM1 -Construction and demolition debris <br> 2nd Submission ONLY | 2 | 0 | 0 |  |
| :--- | :---: | :---: | :---: | :---: |
| WM2 - Recycling <br> 2nd Submission ONLY | 1 | 0 | 0 |  |
| WM3 - Organic Waste <br> 2nd Submission ONLY | 2 | 0 | 0 |  |
| WM4 - Reusable Materials <br> (non-residential buildings only) | 1 | 0 | 0 |  |

## 13. Water Use

| WU1 - Indoor Water Management | 3 | 3 | 0 |  |
| :--- | :---: | :---: | :---: | :--- |
| WU2 - Outdoor Water Management | 1 | 1 | 0 |  |
| WU3 - Stormwater Management | 2 | 2 | 0 |  |
| WU4 - Stormwater Retention | 1 | 1 | 1 | Plan to increase stormwater detention |
| TOTAL | 108 | 59 | $\mathbf{3 0}$ |  |

## Traffic Access and Impact Study

## Mixed-Use Development 819 East Main Street Stamford, Connecticut



## Prepared for: <br> Wellbuilt

January 14, 2022

Mr. Scott Lumby<br>Wellbuilt Company<br>2 Armonk Street<br>Greenwich, Connecticut 06830<br>Dear Mr. Lumby:

As requested, we are pleased to submit this Traffic Study for submission to the City of Stamford, Connecticut Department of Transportation (CTDOT) and the Office of the State Traffic Administration (OSTA). The proposal is to redevelop several properties located at the intersection of East Main Street, Lafayette Street and North State Street. The development comprises 130 -units of multi-family housing and 2,950 square feet of commercial space. Previously, a portion of this site was approved for a 85 -unit residential development in September 2021, with 2,900 square feet of commercial space. For this proposal, the access to a parking garage and at-grade parking will be from North State Street, with pedestrian access mainly from East Main Street.

The full development will generate 55 and 70 vehicle trip ends during the typical weekday morning and weekday afternoon peak hours, respectively. The previously approved development of this property would have generated 38 and 52 vehicle trip ends during the same two peak hours. Therefore, for comparison purposes only, the net increase in site traffic generation will be 17 and 18 vehicle trip ends during the weekday morning and weekday afternoon peak hours, respectively. To be conservative, this traffic analysis does not take any credit for existing traffic generated by the several different land uses located on the Subject Property.

The analysis includes the signalized intersections of East Main Street at Lafayette Street and East Main Street at North State Street and the unsignalized intersections of Lafayette Street at North State Street/South Main Street, as well as the proposed site access drive to North State Street. Results of the analyses indicate that under a build condition all locations will continue to operate at very acceptable Levels of Service during the two peak hours. At the east Main Street/Lafayette Street signalized intersection it will maintain an overall Level of Service "C." At the signalized intersection of East Main Street/North State Street intersection it will maintain a Level of Service "A." For the unsignalized intersections of Lafayette Street intersections of North State Street and South Street and the site access drive all will operate at Level of Service "A," with little, if any delays.

Based on the results of the analyses, off-site road improvements or modifications to traffic control are not necessary to accommodate the additional traffic to be generated by this development. Note that this traffic

Mr. Scott Lumby
Page 2
January 14, 2022
analysis is based on the full redevelopment of the site with 130 residential units and 2,950 square feet of commercial space.
Respectfully submitted,
sorichical


Michael A. Galante
Director of Traffic
Hardesty \& Hanover, LLC

## Enclosure

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## SUMMARY

This Traffic Access and Impact Study was prepared to provide the City of Stamford, the Connecticut Department of Transportation (CTDOT) and the Office of the State Traffic Administration (OSTA) with a detailed analysis to determine potential traffic impacts from the proposed mixed-use development located at 819 East Main Street. The proposal is to construct 130-units of multifamily housing (mid-rise) and 2,950 square feet of commercial space. The site has a previous approval for 85 -units of multifamily housing (midrise) and 2,900 square feet of commercial space from September 2021. Site access is proposed via right turn in/right turn out to North State Street.

This Study addresses traffic conditions for the 2021 existing, 2024 no-build and 2024 build conditions during the weekday morning and weekday afternoon peak hours. Traffic counts were conducted at the Study Area intersections by Hardesty \& Hanover, LLC in December 2021. Based on discussions CTDOT Planning Division, no COVID adjustments are needed.

The 2024 future projected traffic volumes, without the proposed development, employed a 0.6 percent annual growth rate, as per discussions with CTDOT Planning Division. Based on discussions with CTDOT Planning Division and the City of Stamford, no other nearby developments were identified.

Based on trip rates from "Trip Generation," $11^{\text {th }}$ Edition, published by ITE, 2021, it is estimated that the proposed 130-units will generate a total of 48 and 51 vehicle trip ends during the weekday morning and weekday afternoon peak hours, respectively. The proposed 2,950 square feet of commercial space is estimated to generate a total of 7 and 19 vehicle trip ends during the weekday morning and weekday afternoon peak hours, respectively. Therefore, the total proposed site will generate a total of 55 and 70 vehicle trip ends during the weekday morning and weekday afternoon peak hours, respectively. To be conservative no pass-by credit was applied to the commercial space.

This site received an approval for 85 -units of multifamily housing and 2,900 square feet of commercial space in September 2021. For comparison purposes, that approved development would generate a total of 38 and 52 vehicle trip ends during the weekday morning and weekday afternoon peak hours, respectively. The proposed site will generate a total of 17 and 18 additional vehicle trip ends during the weekday
morning and weekday afternoon peak hours, respectively, when compared to the approved development. However, this traffic analysis is based on the full proposal.

A review of current traffic patterns at the Study Area intersections and in the vicinity of the project influence area was conducted to determine trip distribution for the proposed development. For arriving site traffic, it was found that 40 percent will arrive from the east on U.S. Route 1,35 percent will arrive from the west on U.S. Route 1, 15 percent will arrive from the north on Lafayette Street and 10 percent will arrive from the south on South State Street. For departing site traffic, it was found that 40 percent will depart to the east on U.S. Route 1 and 60 percent will depart to the west on U.S. Route 1.

SYNCHRO 10 capacity analyses were conducted for 2021 existing, 2024 no-build and 2024 build conditions to identify incremental impacts and needs that the proposed development will generate. Results of the analyses indicate that the signalized intersection of U.S. Route 1 at Lafayette Street will continue to operate at the same overall Level of Service during both peak hours with a minimal increase in vehicle delay. The westbound and northbound lane groups and approaches will have an acceptable change in Level of Service from "A" to " $B$ " and " $B$ " to " $C$," respectively, during the weekday morning peak hour. The eastbound lane group and approach will have an acceptable change in Level of Service from "A" to " B " during the weekday afternoon peak hour. The southbound left turn lane group and approach will maintain a Level of Service "E" during both peak hours.

The signalized intersection of U.S. Route 1 and North State Street will continue to operate at the same overall Level of Service during both peak hours with a minimal increase in vehicle delay. All lane groups and approaches will maintain the same Levels of Service during both peak hours. At the all-way STOP-controlled intersection of North State Street and Lafayette Street/South State Street, all critical lanes and the intersection overall will maintain the same Level of Service during both peak hours. The proposed site driveway will operate at Level of Service "A" during both peak hours.

## INTRODUCTION

This report has been prepared to address the potential impacts related to the proposed mixed-use development. An analysis was completed for area roadways and key nearby intersections for the typical weekday morning and weekday afternoon peak hours for existing, no-build and build conditions. An assessment of the results of these analyses indicate impacts and any need for mitigation. In this report there is a discussion of area roadways, accident history, site access considerations, current and future traffic volumes, site traffic generation and assignment, capacity analysis procedures and the results of these analyses. Based on the results of the analysis any mitigation necessary is described.

## Project Understanding

The proposal is to construct 130 -units of multifamily housing (mid-rise) and 2,950 square feet of commercial space. The site has a previous approval for 85 -units of multifamily housing (mid-rise) and 2,900 square feet of commercial space. Site access is proposed via right turn in/right turn out to North State Street. It is assumed that the proposal will be built and fully occupied by the end of 2024.

## EXISTING CONDITIONS

In this section of the report there is a description of the existing traffic volumes obtained on area roadways near the site for the weekday morning and weekday afternoon peak hours. It also includes a description of area roads, current traffic control and accident history.

## Roadways

As noted above, the development is located at 819 East Main Street and adjacent properties.

1. East Main Street - This is generally an east-west, two to four-lane, State-maintained roadway, also designated U.S. Route 1. It begins as a continuation of Tresser Boulevard, also designated U.S. Route 1, to the west at the signalized intersection with Elm Street. It continues east past the site to the Darien Town Line, where it continues as Post Road, also designated U.S. Route 1. In the Study Area this roadway provides a double yellow centerline, curbing and sidewalks along both sides of the road. Currently there is two-hour meter parking from 9 A.M. to 8 P.M. with NO PARKING 4 P.M. to 7 P.M. TOW AWAY ZONE along the south side of the road and NO PARKING 7 A.M. to 9 A.M. TOW AWAY ZONE along the north side of the road between Lafayette Street and Quintard Terrace. The section between Lafayette Street and Quintard Terrace operates as one-lane eastbound during the weekday morning peak hour and one-lane westbound during the weekday afternoon peak hour. NO PARKING ANYTIME is posted everywhere else along East Main Street in the Study Area. The roadway width is 40 feet and the land use is residential and commercial.
2. Lafayette Street - This is a north-south, City-maintained roadway. It begins to the north at the signalized intersection of Daskam Place/Crystal Street as a continuation of Daskam Place. It continues south as a one-way southbound roadway to the signalized intersection with East Main Street, also designated U.S. Route 1. To the north of East Main Street this roadway provides curbing and sidewalks along both sides of the road. There is NO PARKING along the easterly side of the road, as well as along the westerly side of the road along the shopping center frontage. The roadway width is generally 23 feet with a mix of residential and commercial uses.

South of East Main Street, this roadway provides two lanes, with one lane in each directions to the intersection of North State Street. To the south of East Main Street this roadway provides a double
yellow centerline, curbing and sidewalks along both sides of the road. There is NO PARKING along the westerly side of the road for the entire length. The roadway width is generally 27 feet with a mix of residential and commercial uses.
3. North State Street - This is an east-west, City-maintained roadway. From East Main Street to South State Street this roadway is a two-lane, two-way roadway with a double yellow centerline, curbing along both sides of the road and a sidewalk along the northerly side of the road. For a short section between South State Street and Lafayette Street, this roadway is one-way westbound with curbing along both sides of the road, a sidewalk along the northerly side of the road and NO PARKING ANYTIME. West of Lafayette Street, this roadway is a two-lane, two-way roadway with a double yellow centerline, curbing along both sides of the road and a sidewalk along the northerly side of the road. The roadway terminates with access to a large office building. Land use is generally commercial.

Figure 1 provides a summary of current street system characteristics. Photographs of the Study Area intersections are provided in the Appendix of this report.

## Traffic Volumes

To develop baseline traffic volumes for the Study Area intersections and roadways, turning movement counts were conducted at the following intersections:

- U.S. Route 1 at Lafayette Street;
- U.S. Route 1 at North State Street;
- North State Street at Lafayette Street/South State Street, and,
- At the South State Street Split.

The turning movement counts were conducted during the following times:

- Wednesday, December 15, 2021-7:00 to 9:00 A.M.; and,
- Tuesday, December 14, 2021-4:00 to 6:00 P.M.


Based on the results of the traffic counting program the following peak hours were identified at the Study Area intersections:

- Weekday morning - 7:15 to 8:15 A.M.; and,
- Weekday afternoon $-4: 00$ to 5:00 P.M.

Figures 2 and 3 graphically illustrates the 2021 existing traffic volumes for the weekday morning and weekday afternoon peak hours, respectively. Raw and summarized turning movement count data collected by Hardesty \& Hanover, LLC for all Study peak periods can be found in the Appendix of this report.

Based on the results of the traffic counting program the traffic volumes were identified for area roadways and includes U.S. Route 1, west of Lafayette Street, which had a two-way volume of 1,325 and 1,411 vehicles during the weekday morning and weekday afternoon peak hours, respectively. U.S. Route 1 , east of Lafayette Street, had a two-way volume of 1,499 and 1,546 vehicles during the same two peak hours noted above, respectively. Lafayette Street, north of U.S. Route 1, had a one-way volume of 276 and 205 vehicles during the two peak hours noted above, respectively. Lafayette Street, south of U.S. Route 1, had a two-way volume of 44 and 94 vehicles during the two peak hours noted above, respectively.
U.S. Route 1, west of North State Street, had a two-way volume of 1,519 and 1,597 vehicles during the weekday morning and weekday afternoon peak hours, respectively. U.S. Route 1, east of North State Street, had a two-way volume of 1,573 and 1,735 vehicles during the same two peak hours noted above, respectively. North State Street, south of U.S. Route 1, had a two-way volume of 76 and 148 vehicles during the two peak hours noted above, respectively.

North State Street, west of Lafayette Street, had a two-way volume of 36 and 101 vehicles during the weekday morning and weekday afternoon peak hours, respectively. North State Street, east of Lafayette Street, had a one-way volume of 9 and 21 vehicles during the same two peak hours noted above, respectively. South State Street, south of North State Street, had a one-way volume of 47 and 124 vehicles during the two peak hours noted above, respectively. Table 1 provides a summary of the recorded two-way volumes.

Table 1
2021 TRAFFIC VOLUMES - PEAK HOURS
Mixed-Use Development
819 East Main Street
Stamford, Connecticut

| LOCATION | VEHICLES |  |
| :--- | :---: | :---: |
|  | Weekday <br> Morning | Weekday <br> Afternoon |
| U.S. Route 1, West of Lafayette Street | 1,325 | 1,411 |
| U.S. Route 1, East of Lafayette Street | 1,499 | 1,546 |
| Lafayette Street, North of U.S. Route 1 | 276 | 205 |
| Lafayette Street, South of U.S. Route 1 | 44 | 94 |
| U.S. Route 1, West of North State Street | 1,519 | 1,597 |
| U.S. Route 1, East of North State Street | 1,573 | 1,735 |
| North State Street, South of U.S. Route 1 | 76 | 148 |
| North State Street, West of Lafayette Street/South State Street | 36 | 101 |
| North State Street, East of Lafayette Street/South State Street | 9 | 21 |
| Lafayette Street, North of North State Street | 44 | 94 |
| South State Street, South of North State Street | 47 | 124 |

Sources: Turning movement counts conducted by Hardesty \& Hanover on Tuesday, December 14 and Wednesday, December 15, 2021.

Notes: Based on discussions with CTDOT Planning Division, no COVID adjustments were needed.
Hardesty \& Hanover
Y:ISharedIProjects105498-819 East Main St Stamford1500-Technicall501-Traffic StudyIWordI22-001.t.docx 1/6/2022



## Accident Experience

The latest available accident data was obtained from the Connecticut Crash Data Repository (CTCDR) for a period beginning October 1, 2018 to September 30, 2021 for U.S. Route 1, which is the latest three years available at this time. Accident data from 2020 was included and was during COVID-19 conditions. At the intersection of U.S. Route 1 and Lafayette Street, there were a total of 27 accidents recorded during this three-year period. Data indicates that 74 percent of the accidents involved property damage and 26 percent involved injuries. The collision types were 37 percent involving an angle collision, 22 percent involving a rear-end collision, 19 percent involving a sideswipe in the same direction, 11 percent involving a head-on collision, 7 percent involving a single vehicle crash and 4 percent involving other collision. The contribution factors were 25 percent for failure to stay in lane, 22 percent for running a red light, 15 percent for other action, 11 percent for failure to yield right-of-way, 7 percent for unknown and 4 percent for following too closely, ran STOP sign, improper backing, operate in a reckless manner and operator inattentive. It was found that 70 percent of the accidents occurred during daylight hours and 77 percent occurred on dry road conditions.

For the section of U.S. Route 1, between Lafayette Street and Quintard Terrace, there were a total of 37 accidents recorded during this three-year period. Data indicates that 89 percent of the accidents involved property damage and 11 percent involved injuries. The collision types were 38 percent involving an angle collision and a sideswipe in the same direction, 11 percent involving a rear-end collision, 8 percent involving a sideswipe in the opposite direction and 5 percent involving a single vehicle crash. The contribution factors were 27 percent for failure to stay in lane, 24 percent for failure to yield right-of-way, 14 percent for following too closely, 8 percent for other action, unknown and no contributing action, 5 percent for improper passing and 3 percent for improper backing and improper turning. It was found that 62 percent of the accidents occurred during daylight hours and 83 percent occurred on dry road conditions.

At the intersection of U.S. Route 1 and Quintard Terrace, there were a total of 15 accidents recorded during this three-year period. Data indicates that 60 percent of the accidents involved property damage and 40 percent involved injuries. The collision types were 39 percent involving a rear-end collision, 33 percent involving an angle collision and 7 percent involving a sideswipe in the same and in the opposite directions, a single vehicle crash and other collision. The contribution factors were 33 percent for failure to yield right-ofway, 26 percent for following too closely, 13 percent for improper passing and 7 percent for other action, ran

STOP sign, improper backing and improper passing. It was found that 67 percent of the accidents occurred during daylight hours and 73 percent occurred on dry road conditions.

For the section of U.S. Route 1, between Quintard Terrace and North State Street, there were a total of 11 accidents recorded during this three-year period. Data indicates that 82 percent of the accidents involved property damage and 18 percent involved injuries. The collision types were 46 percent involving a rear-end collision, 27 percent involving a sideswipe in the same direction, 18 percent involving an angle collision and 9 percent involving a sideswipe in the opposite direction. The contribution factors were 28 percent for failure to stay in lane, 18 percent for following too closely, other action and improper backing and 9 percent for failure to yield right-of-way and unknown. It was found that 64 percent of the accidents occurred during daylight hours and 91 percent occurred on dry road conditions.

At the intersection of U.S. Route 1 and North State Street, there were a total of 10 accidents recorded during this three-year period. Data indicates that 70 percent of the accidents involved property damage and 30 percent involved injuries. The collision types were 80 percent involving an angle collision and 10 percent involving a rear-end collision and a sideswipe in the same direction. The contribution factors were 40 percent for failure to yield right-of-way, 20 percent for improper turning and 10 percent for following too closely, failure to stay in lane, no contributing action and wrong side/way. It was found that 80 percent of the accidents occurred during daylight hours and 70 percent occurred on dry road conditions. Table 2 provides a more detailed summary of the accident data. The accident data from the CTCDR is included in the Appendix of this report.

The latest available accident data was requested from the City of Stamford Police Department for a period beginning January 1, 2018 through December 31, 2020 for North State Street, South State Street and Lafayette Street. Once this data is received, it will be summarized in Table 3.
Table 2
ACCIDENT EXPERIENCE SUMMARY - U.S. ROUTE 1
Mixed-Use Development
819 East Main Street
Stamford, Connecticut

| ACCIDENT CHARACTERISTICS | U.S. ROUTE 1 |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Street | Between Lafayette Street and Quintard Terrace (7.86-7.92) |  | At Quintard Terrace (7.93) |  | Between Quintard Terrace and North State Street (7.94-7.97) |  | At North Street (7.98) |  |
|  | Total | \% | Total | \% | Total | \% | Total | \% | Total | \% |
| Year <br> - 2018/2019 <br> - 2019/2020 <br> - 2020/2021 <br> - Total | $\begin{gathered} 11 \\ 12 \\ 4 \\ 27 \end{gathered}$ | $\begin{gathered} 41 \\ 44 \\ 15 \\ 100 \end{gathered}$ | $\begin{aligned} & 11 \\ & 13 \\ & 13 \\ & 37 \end{aligned}$ | $\begin{gathered} 30 \\ 35 \\ 35 \\ 100 \end{gathered}$ | $\begin{gathered} 7 \\ 3 \\ 5 \\ 15 \end{gathered}$ | $\begin{array}{r} 47 \\ 20 \\ 33 \\ 100 \\ \hline \end{array}$ | $\begin{gathered} 3 \\ 4 \\ 4 \\ 11 \end{gathered}$ | $\begin{gathered} 28 \\ 36 \\ 36 \\ 100 \\ \hline \end{gathered}$ | $\begin{gathered} 5 \\ 3 \\ 2 \\ 10 \\ \hline \end{gathered}$ | $\begin{gathered} 50 \\ 30 \\ 20 \\ 100 \\ \hline \end{gathered}$ |
| Accident Severity <br> - Property Damage <br> - Injury | $\begin{gathered} 20 \\ 7 \\ \hline \end{gathered}$ | $\begin{aligned} & 74 \\ & 26 \\ & \hline \end{aligned}$ | $\begin{gathered} 33 \\ 4 \\ \hline \end{gathered}$ | $\begin{aligned} & 89 \\ & 11 \\ & \hline \end{aligned}$ | $\begin{aligned} & 9 \\ & 6 \\ & \hline \end{aligned}$ | $\begin{aligned} & 60 \\ & 40 \\ & \hline \end{aligned}$ | $\begin{aligned} & 9 \\ & 2 \\ & \hline \end{aligned}$ | $\begin{aligned} & 82 \\ & 18 \\ & \hline \end{aligned}$ | $\begin{aligned} & 7 \\ & 3 \end{aligned}$ | 70 30 |
| Collision Type <br> - Rear End <br> - Angle <br> - Sideswipe-Same Direction <br> - Sideswipe-Opposite Direction <br> - Head On <br> - Single Vehicle <br> - Other | $\begin{gathered} 6 \\ 10 \\ 5 \\ 0 \\ 3 \\ 2 \\ 1 \end{gathered}$ | $\begin{gathered} 22 \\ 37 \\ 19 \\ 0 \\ 11 \\ 7 \\ 4 \end{gathered}$ | $\begin{gathered} 4 \\ 14 \\ 14 \\ 3 \\ 0 \\ 2 \\ 0 \end{gathered}$ | $\begin{gathered} 11 \\ 38 \\ 38 \\ 8 \\ 0 \\ 5 \\ 0 \end{gathered}$ | $\begin{aligned} & 6 \\ & 5 \\ & 1 \\ & 1 \\ & 0 \\ & 1 \\ & 1 \end{aligned}$ | $\begin{gathered} 39 \\ 33 \\ 7 \\ 7 \\ 0 \\ 7 \\ 7 \end{gathered}$ | $\begin{aligned} & 5 \\ & 2 \\ & 3 \\ & 1 \\ & 0 \\ & 0 \\ & 0 \end{aligned}$ | $\begin{aligned} & 46 \\ & 18 \\ & 27 \\ & 9 \\ & 0 \\ & 0 \\ & 0 \end{aligned}$ | $\begin{aligned} & 1 \\ & 8 \\ & 1 \\ & 0 \\ & 0 \\ & 0 \\ & 0 \end{aligned}$ | 10 80 10 0 0 0 0 |

Table 2 Cont'd

Table 2 Cont'd

| ACCIDENT CHARACTERISTICS | U.S. ROUTE 1 |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | At Lafayette Street <br> (7.85) |  | Between Lafayette Street and Quintard Terrace (7.86-7.92) |  | At Quintard Terrace (7.93) |  | Between Quintard Terrace and North State Street (7.94-7.97) |  | At North Street (7.98) |  |
|  | Total | \% | Total | \% | Total | \% | Total | \% | Total | \% |
| Weather Conditions |  |  |  |  |  |  |  |  |  |  |
| - Clear | 20 | 74 | 30 | 81 | 12 | 80 | 7 | 64 | 9 | 90 |
| - Cloudy | 2 | 7 | 3 | 8 | 0 | 0 | 3 | 27 | 0 | 0 |
| - Rain | 4 | 15 | 4 | 11 | 3 | 20 | 1 | 9 | 1 | 10 |
| - Snow | 1 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |

[^2]> October 1, 2018 to September 30, 2021 is the latest three years of accident data available. 1)
2)
4)
Notes:

Table 3
ACCIDENT EXPERIENCE SUMMARY - NORTH STATE STREET/LAFAYETTE STREET Mixed-Use Development
819 East Main Street
Stamford, Connecticut

| ACCIDENT CHARACTERISTICS | NORTH STATE STREET |  |  |  | LAFAYETTE STREET |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{gathered} \text { Between U.S. Route } 1 \\ \text { and Lafayette } \\ \text { Street/South State Street } \end{gathered}$ |  | At Lafayette Street/South State Street |  | Between North State Street and U.S. Route 1 |  |
|  | Total | \% | Total | \% | Total | \% |
| Year $=$ $=$ - 2019 $=$ - - Total |  |  |  |  |  |  |
| Accident Severity <br> - Property Damage <br> - Injury | DATA NOT RECEIVED |  |  |  |  |  |
| Collision Type <br> - Rear End <br> - Head On <br> - Angle <br> - Sideswipe-Same Direction |  |  |  |  |  |  |
| Contributing Factor <br> - Following Too Closely <br> - Failure to Yield ROW <br> - Failure to Stay in Lane <br> - Ran Off Roadway <br> - Improper Turn <br> - Improper Backing |  |  |  |  |  |  |
| Light Condition <br> - Daylight <br> - Dark - Lighted <br> - Dark - Not Lighted |  |  |  |  |  |  |
| Surface Condition <br> - Dry <br> - Wet |  |  |  |  |  |  |
| Weather Conditions <br> - Clear <br> - Cloudy <br> - Rain |  |  |  |  |  |  |

Source: Stamford Police Department from January 1, 2019 to December 31, 2021.
Hardesty \& Hanover, LLC
Y:ISharedIProjects105498-819 East Main St Stamford1500-Technical1501-Traffic Study1Word|22-003.stc.docx 1/6/22

## FUTURE TRAFFIC IMPACTS

This section of the report describes the future 2024 traffic conditions for the Study Area. It includes 2024 no-build traffic volumes, estimates for site traffic generation, distribution and assignment of the proposed site traffic, future build traffic volumes and the results of capacity analyses. The capacity analyses are completed for a no-build and build condition, which provides a basis for determining potential impact to area roads and nearby intersections and the need for mitigation, if necessary.

## No-Build Traffic Volumes

The 2021 existing traffic volumes, which were previously described, were expanded to reflect a 2024 traffic condition for each of the intersections by applying an annual growth rate of 0.6 percent, as per discussions with CTDOT Planning Division, to account for general growth in the immediate vicinity of the surrounding area. Based on discussions with the CTDOT Planning Division and the City of Stamford, no other developments were identified. The 2024 no-build traffic volumes are graphically illustrated in Figures 4 and 5 for the weekday morning and weekday afternoon peak hours, respectively.

## Estimation of Site Traffic Generation

The proposal is to construct 130 -units of multifamily housing (mid-rise) and 2,950 square feet of commercial space. Based on trip rates from "Trip Generation," $11^{\text {th }}$ Edition, published by ITE, 2021, it is estimated that the proposed 130 -units would generate a total of 48 and 51 vehicle trip ends during the weekday morning and weekday afternoon peak hours, respectively. The proposed 2,950 square feet of commercial space is estimated to generate a total of 7 and 19 vehicle trip ends during the weekday morning and weekday afternoon peak hours, respectively. It is estimated that the total proposed site will generate a total of 55 and 70 vehicle trip ends during the weekday morning and weekday afternoon peak hours, respectively. To be conservative no pass-by credit was applied to the trips generated by the commercial space.

It should be noted that there is a recent approval for 85 -units of multifamily housing (mid-rise) and 2,900 square feet of commercial space on the site. For comparison purposes, that approved development would generate a total of 38 and 52 vehicle trip ends during the weekday morning and weekday afternoon peak hours, respectively. The proposed site will generate a total of 17 and 18 additional vehicle trip ends during the weekday morning and weekday afternoon peak hours, respectively, when compared to the approved


development. The traffic analysis is based on the full proposal. Table 4 illustrates the details of the site traffic generation by entering and exiting.

## Distribution and Assignment of Site Traffic

A review of current traffic patterns at the Study Area intersections and in the vicinity of the project influence area was conducted to determine trip distribution for the proposed development. For arriving site traffic, it was found that 40 percent will arrive from the east on U.S. Route 1, 35 percent will arrive from the west on U.S. Route 1, 15 percent will arrive from the north on Lafayette Street and 10 percent will arrive from the south on South State Street. For departing site traffic, it was found that 40 percent will depart to the east on U.S. Route 1 and 60 percent will depart to the west on U.S. Route 1.

Figure 6 provides the site traffic distribution of the proposed development. Figures 7 and 8 graphically illustrate the site traffic generation and assignment for the proposed development for the weekday morning and weekday afternoon peak hours, respectively.

## Build Traffic Volumes

Build traffic volumes for a 2024 condition are graphically illustrated in Figures 9 and 10 for the weekday morning and weekday afternoon peak hours, respectively. The 2024 build traffic volumes include the 2024 no-build traffic volumes and the site traffic generation volumes for the proposed development for each time period.

## Capacity Analysis Procedures

Capacity analysis procedures are provided in the Appendix of this report. The analyses is based on a SYNCHRO computer model and information provided by the Transportation Research Board (TRB) and the Highway Capacity Manual (HCM) $6^{\text {th }}$ Edition.

## Capacity Analysis Results - Existing, No-Build and Build Conditions

The following is a summary of the results of analyses for an existing, no-build and build conditions at the Study Area intersections and site access drive for each of the time periods included in this analysis.

Table 4
SITE TRAFFIC GENERATION - PEAK HOURS Mixed-Use Development 819 East Main Street Stamford, Connecticut

| LAND USE | SIZE | TRAFFIC DIRECTION | VEHICLE TRIP ENDS |  |
| :---: | :---: | :---: | :---: | :---: |
|  |  |  | Weekday Morning | Weekday Afternoon |
| Approved Multifamily Housing (Mid-Rise) | 85 Dwelling Units | Enter Exit <br> Total | $\begin{gathered} \hline 7 \\ \underline{24} \\ \hline 31 \\ \hline \end{gathered}$ | $\begin{aligned} & 20 \\ & \frac{13}{33} \\ & \hline \end{aligned}$ |
| Approved Strip Retail Plaza | 2,900 S.F. | Enter <br> Exit <br> Total | $\begin{array}{r} 4 \\ \frac{3}{7} \end{array}$ | $\begin{gathered} 9 \\ \frac{10}{19} \end{gathered}$ |
| 1) Approved Total Site Traffic |  | Enter <br> Exit <br> Total | $\begin{array}{r} 11 \\ \underline{27} \\ \hline 38 \end{array}$ | $\begin{aligned} & 29 \\ & \frac{23}{52} \\ & \hline \end{aligned}$ |
| Proposed Multifamily Housing (Mid-Rise) | 130 Dwelling Units | Enter <br> Exit <br> Total | $\begin{array}{r} 11 \\ \frac{37}{48} \\ \hline \end{array}$ | $\begin{aligned} & 31 \\ & \frac{20}{51} \\ & \hline \end{aligned}$ |
| Proposed Strip Retail Plaza | 2,950 S.F. | Enter Exit <br> Total | $\begin{aligned} & 4 \\ & \frac{3}{7} \end{aligned}$ | $\begin{gathered} 9 \\ \frac{10}{19} \\ \hline \end{gathered}$ |
| 2) Proposed Total Site Traffic |  | Enter Exit <br> Total | $\begin{array}{r} 15 \\ \frac{40}{55} \\ \hline \end{array}$ | $\begin{aligned} & 40 \\ & 30 \\ & \hline 70 \\ & \hline \end{aligned}$ |
| Net Increase Site Traffic (2-1) |  | Enter Exit <br> Total | $\begin{gathered} \hline 4 \\ \frac{13}{17} \\ \hline \end{gathered}$ | $\begin{aligned} & 11 \\ & \frac{7}{18} \end{aligned}$ |

Sources: "Trip Generation," 11th Edition, published by the Institute of Transportation Engineers (ITE), 2021 using Multifamily Housing (Mid-Rise), Code \#221 average rates and Strip Retail Plaza, Code \#822 average rates.

Note: $\quad$ No pass-by credit was applied to the commercial space, to be conservative.
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Y:ISharedXProjects105498-819 East Main St Stamford1500-Technicall1501-Traffic StudyWord122-004.stc.docx 1/3/22






## 1. U.S. Route 1 at Lafayette Street

Existing - Results of the analysis of this signalized intersection indicate that it currently operates at an overall Level of Service "B" and "C" during the weekday morning and weekday afternoon peak hours, respectively. The southbound left turn lane group and approach operate at a Level of Service "E" during both peak hours.

No-Build - Results of the analysis of this signalized intersection indicate that it will operate at an overall Level of Service " $B$ " and " $C$ " during the weekday morning and weekday afternoon peak hours, respectively. The southbound left turn lane group and approach operate at a Level of Service " $E$ " during both peak hours.

Build - Results of the analysis indicate that with the site-generated traffic added to this signalized intersection it will continue to operate at the same overall Level of Service during both peak hours with a minimal increase in vehicle delay. The westbound and northbound lane groups and approaches will have an acceptable change in Level of Service from "A" to "B" and "B" to "C," respectively, during the weekday morning peak hour. The eastbound lane group and approach will have an acceptable change in Level of Service from "A" to "B" during the weekday afternoon peak hour. All remaining lane groups and approaches will maintain the same Levels of Service during all peak hours.

## 2. U.S. Route 1 at North State Street

Existing - Results of the analysis of this signalized intersection indicate that it currently operates at an overall Level of Service "A" during both the weekday morning and weekday afternoon peak hours. No-Build - Results of the analysis of this signalized intersection indicate that it will operate at an overall Level of Service "A" during both the weekday morning and weekday afternoon peak hours.
Build - Results of the analysis indicate that with the site-generated traffic added to this signalized intersection it will continue to operate at the same overall Level of Service during both peak hours. All lane groups and approaches will maintain the same Levels of Service during all peak hours.

## 3. North State Street at Lafayette Street/South State Street

Existing - Results of the analysis of this all-way STOP-controlled intersection indicate that it currently operates at an overall Level of Service "A" during both the weekday morning and weekday afternoon peak hours.

No-Build - Results of the analysis of this all-way STOP-controlled intersection indicate that it will operate at an overall Level of Service "A" during both the weekday morning and weekday afternoon peak hours.

Build - Results of the analysis indicate that with the site-generated traffic added to this all-way STOP-controlled intersection it will continue to operate at the same overall Level of Service during both peak hours with a minimal increase in vehicle delays. All lanes will maintain the same Levels of Service during all peak hours.

## 4. North State Street at Site Access Drive

Build - Results of the analysis of this two-way STOP controlled intersection indicate that it will operate at a Level of Service "A" during both the weekday morning and weekday afternoon peak hours.

Table 5 provides a more detailed summary of the results of the capacity analyses for the Study Area intersections, as described above. This table provides Level of Service, average vehicle delay and volume to capacity ratio for each lane group, approach, intersection overall and lane during each of the peak hours for the existing, no-build and build conditions. It also provides a project assessment between the no-build and build conditions, which identifies the potential impact. The results of the Storage/Queue analyses for the Study Area intersections are also provided for each lane group and lane during each of the peak hours for the existing, no-build and build conditions. The capacity worksheets are included in the Appendix of this report.

## Findings

This Traffic Access and Impact Study was prepared to provide the City of Stamford, the Connecticut Department of Transportation (CTDOT) and the Office of the State Traffic Administration (OSTA) with a detailed analysis to determine potential traffic impacts from the proposed mixed-use development located at 819 East Main Street. The proposal is to construct 130-units of multifamily housing (mid-rise) and 2,950 square feet of commercial space. The site has a previous approval for 85 -units of multifamily housing (midrise) and 2,900 square feet of commercial space from September 2021. Site access is proposed via right turn in/right turn out to North State Street.
S\&กOH Y甘ヨd - 1 NZWSS Mixed-Use Development
819 East Main Street

| INTERSECTION | CONTROL TYPE | $\begin{gathered} \text { STORAGE } \\ \text { LINK } \\ \text { LENGTH } \\ \hline \end{gathered}$ | PHYSICAL UNITS | 2021 EXISTING CONDITIONS (BASELINE) |  |  |  |  |  | 2024 NO-BUILD CONDITIONS |  |  |  |  |  | 2024 BUILD CONDITIONS |  |  |  |  |  | PROJECT IMPACTS (NO-BUILD TO BUILD) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Weekday Morning |  |  | Weekday Afterncon |  |  | Weekday Morning |  |  | Weekday Afternoon |  |  | Weekday Morning |  |  | Weekday Aftemoon |  |  | Weekday Morning |  | Weekday Afternoon |  |
|  |  |  |  | LOS/ <br> Delay | VIC <br> Ratio | Queue Length (Feet) | $\begin{array}{\|l} \text { LOS/ } \\ \text { Delay } \\ \hline \end{array}$ | VIC Ratio | Queue Length (Feet) | $\begin{array}{\|l\|l\|l\|l\|l\|l\|} \hline \text { LOSI } \\ \hline \end{array}$ | V/C Ratio | Queue Length (Feet) | $\begin{aligned} & \text { LOS/ } \\ & \text { Delay } \end{aligned}$ | $\begin{aligned} & \text { V/C } \\ & \text { Ratio } \\ & \hline \end{aligned}$ | Queue Length (Feet) | $\begin{aligned} & \text { LOS/ } \\ & \text { Delay } \end{aligned}$ | $\begin{gathered} \text { V/C } \\ \text { Ratio } \\ \hline \end{gathered}$ | Queue Length (Feet) | $\begin{array}{\|l} \text { LOS/ } \\ \text { Delay } \\ \hline \end{array}$ | VIC Ratio | Queue Length (Feet) | Deterioration in LOS | Project Delay (Seconds) | Deterioration in LOS | Project Delay (Seconds) |
| U.S. Route 1 at | Traffic | 285 | EB TR | A/8.0 | 0.22 | 116 | A/9.6 | 0.44 | 154 | A/8.2 | 0.23 | 120 | A/10.0 | 0.46 | 157 | A88.7 | 0.23 | 125 | B/12.1 | 0.49 | 162 | No | 0.5 | A-B | 2.1 |
| Lafayette Street | Signal | -- | APP. | A/8.0 | - | -- | A/9.6 | -- | -- | A8. 2 | -- | -- | A/10.0 | -- | -- | A88.7 | - | - | B/12.1 | - | - | No | 0.5 | A-B | 2.1 |
|  |  | 360 | WB LT | A/9.5 | 0.42 | 274 | C/21.4 | 0.70 | 403 | A/9.9 | 0.43 | 288 | C/24.1 | 0.72 | 455 | B/10.8 | 0.43 | 305 | C/29.4 | 0.76 | 474 | A-B | 0.9 | No | 5.3 |
|  |  | -- | APP. | A/9.5 | - | - | C/21.4 | -- | -- | A19.9 | -- | - | C/24.1 | -- | -- | B/10.8 | - | -- | C/29.4 | - | -- | A-B | 0.9 | No | 5.3 |
|  |  | 300 | NB LR | B/16.5 | 0.11 | 34 | C/25.8 | 0.29 | 70 | B/16.7 | 0.11 | 34 | C/26.1 | 0.29 | 73 | C/21.8 | 0.23 | 68 | C/30.0 | 0.37 | 108 | B-C | 5.1 | No | 3.9 |
|  |  | -- | APP. | B/16.5 | - | -- | C/25.8 | - | -- | B/16.7 | - | -- | C/26.1 | -- | -- | C/21.8 | - | -- | C/30.0 | - | -- | B-C | 5.1 | No | 3.9 |
|  |  | 1,230 | SB L | E/68.0 | 0.84 | 255 | E/73.7 | 0.86 | 212 | E167.7 | 0.84 | 260 | E/73.0 | 0.86 | 217 | E/67.7 | 0.85 | 261 | E/64.0 | 0.82 | 249 | No | 0.0 | No | -9.0 |
|  |  | 200 | TR | A/10.0 | 0.16 | 33 | B/12.5 | 0.12 | 22 | A/9.9 | 0.16 | 33 | B/42.3 | 0.13 | 22 | A/9.5 | 0.15 | 33 | B/11.9 | 0.11 | 22 | No | -0.4 | No | -0.4 |
|  |  | - | APP. | E/56.7 | - | -- | E/63.9 | - | -- | E/56.5 | -- | -- | E/63.1 | -- | -- | E/56.5 | - | -- | E/55.7 | - | -- | No | 0.0 | No | -7.4 |
|  |  | -- | Overall | B/17.5 | - | -- | C/21.7 | - | -- | B/17.8 | -- | -- | C/22.8 | -- | -- | B/18.6 | - | -- | C/25.0 | - | -- | No | 0.8 | No | 2.2 |
| U.S. Route 1 at | Traffic | 160 | EB TR | A11.2 | 0.25 | 37 | A/2.6 | 0.38 | 127 | A/1.2 | 0.25 | 37 | A/2.7 | 0.39 | 131 | A1. 2 | 0.26 | 38 | A/3.3 | 0.41 | 129 | No | 0.0 | No | 0.6 |
| North State | Signal | -- | APP. | A/1.2 | - | -- | A/2.6 | -- | -- | A/1.2 | -- | -- | A/2.7 | - | -- | A/1.2 | - | -- | A/3.3 | - | -- | No | 0.0 | No | 0.6 |
| Street |  | 105 | WB LT | A/2.0 | 0.29 | 74 | A/2.4 | 0.27 | 74 | A/2.0 | 0.30 | 76 | A/2.5 | 0.28 | 79 | A/2.0 | 0.31 | 77 | A/2.7 | 0.31 | 85 | No | 0.0 | No | 0.2 |
|  |  | -- | APP. | A/2.0 | -- | -- | A/2.4 | - | -- | A/2.0 | - | -- | A/2.5 | -- | - | A/2.0 | - | - | A/2.7 | -- | -- | No | 0.0 | No | 0.2 |
|  |  | 1,915 | NB LR | C/25.7 | 0.43 | 55 | C/23.9 | 0.64 | 74 | C/25.5 | 0.43 | 55 | C/26.3 | 0.65 | 81 | C/25.5 | 0.43 | 55 | C/27.1 | 0.66 | 84 | No | 0.0 | No | 0.8 |
|  |  | -- | APP. | C/25.7 | - | -- | C/23.9 | - | -- | C/25.5 | - | -- | C/26.3 | -- | - | C/25.5 | - | -- | C/27.1 | - | -- | No | 0.0 | No | 0.8 |
|  |  | -- | Overall | A/2.6 | -- | -- | A/4.2 | - | -- | A 2.6 | -- | -- | A/4.5 | -- | -- | A/2.6 | - | -- | A/4.9 | - | - | No | 0.0 | No | 0.4 |
| North State | AWSC | 280 | EB Ln1 | A/7.3 | 0.007 | 0 | A/7.7 | 0.015 | 0 | A/7.3 | 0.007 | 0 | A/7. 7 | 0.015 | 0 | A/7.4 | 0.007 |  | A/7.7 | 0.015 | 0 | No | 0.1 | No | 0.0 |
| Street at |  | 760 | WB Ln1 | A/6.9 | 0.011 | 0 | A17. 2 | 0.030 | 3 | A/6.9 | 0.011 | 0 | A7. 2 | 0.030 | 3 | A66.8 | 0.057 | 5 | A/7.2 | 0.070 | 5 | No | -0.1 | No | 0.0 |
| Lafayette Street/ |  | 110 | NB Ln1 | A/7.9 | 0.030 | 3 | A/8.5 | 0.114 | 10 | A/7.9 | 0.030 | 3 | A/8. 5 | 0.115 | 10 | A/8.0 | 0.030 | 3 | A/8.6 | 0.117 | 10 | No | 0.1 | No | 0.1 |
| South State |  | 1,160 | NB Ln2 | A7. 5 | 0.043 | 3 | A17.8 | 0.096 | 8 | A17. 5 | 0.044 | 3 | A/7.8 | 0.100 | 8 | A17.6 | 0.048 | 5 | A/8.0 | 0.107 | 10 | No | 0.1 | No | 0.2 |
| Street |  | 300 | SB Ln1 | A/6.6 | 0.008 | 0 | A/6.8 | 0.019 | 3 | A/6.6 | 0.008 | 0 | A/6.8 | 0.019 | 3 | A/6.7 | 0.008 | 0 | A/6.9 | 0.020 | 3 | No | 0.1 | No | 0.1 |
|  |  | -- | Overall | A/7. 5 | -- | -- | A7. 9 | - | -- | A/7. 5 | - | -- | A/7.9 | -- | -- | A87. 2 | - | - | A/7. 9 | -- | -- | No | -0.3 | No | 0.0 |
| North State Street at Site Access Drive | TWSC | 100 | SB Ln1 | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A | A/8. 5 | 0.041 | 3 | A/8.6 | 0.032 | 3 | N/A | N/A | N/A | N/A |

Notes:
Synchro $10.0 / \mathrm{HCM} 6^{\text {th }}$ Edition results are used for capacity analysis.
For Signalized Intersections: Level of Service/Average Total delay per vehicle (seconds/vehicle).
For TWSC and AWSC Intersections: Level of Service/Average Control delay per vehicle (seconds/vehicle). desirable). Levels of Service E and F are normally undesirable. V/C ratio indicates the amount of congestion for each Lane Group, Movement and The Queue Length rows show the $95^{\text {th }}$ percentile maximum queue length in feet.
The Queue Length is for each lane. The total queue length is divided by the numb
The Queue Length is for each lane. The total queue length is divided by the number of lanes and the lane utilization factor.
The $95^{\text {th }}$ percentile queue is the maximum back of the queue with the $955^{\text {th }}$ percentile traffic volumes. Bolded $95^{\text {th }}$ percentile queue exceeds the storage available.
TWSC = Two-Way STOP Control. TWSC = Two-Way STOP Control.
AWSC $=$ All-Way STOP Control.
N/A $=$ Not Avaliable.
Physical Units consist of the following:

1. Lane Group and Intersection Overall for Traffic Signal Controlled Intersections. 2. TWSC Intersections: Critical Lane and Critical Movement.
2. AWSC Intersections: Lane and Intersection Overall.
[^3]This Study addresses traffic conditions for the 2021 existing, 2024 no-build and 2024 build conditions during the weekday morning and weekday afternoon peak hours. Traffic counts were conducted at the Study Area intersections by Hardesty \& Hanover, LLC in December 2021. Based on discussions CTDOT Planning Division, no COVID adjustments are needed.

The 2024 future projected traffic volumes, without the proposed development, employed a 0.6 percent annual growth rate, as per discussions with CTDOT Planning Division. Based on discussions with CTDOT Planning Division and the City of Stamford, no other nearby developments were identified.

Based on trip rates from "Trip Generation," $11^{\text {th }}$ Edition, published by ITE, 2021, it is estimated that the proposed 130 -units will generate a total of 48 and 51 vehicle trip ends during the weekday morning and weekday afternoon peak hours, respectively. The proposed 2,950 square feet of commercial space is estimated to generate a total of 7 and 19 vehicle trip ends during the weekday morning and weekday afternoon peak hours, respectively. Therefore, the total proposed site will generate a total of 55 and 70 vehicle trip ends during the weekday morning and weekday afternoon peak hours, respectively. To be conservative no pass-by credit was applied to the commercial space.

This site received an approval for 85 -units of multifamily housing and 2,900 square feet of commercial space in September 2021. For comparison purposes, that approved development would generate a total of 38 and 52 vehicle trip ends during the weekday morning and weekday afternoon peak hours, respectively. The proposed site will generate a total of 17 and 18 additional vehicle trip ends during the weekday morning and weekday afternoon peak hours, respectively, when compared to the approved development. However, this traffic analysis is based on the full proposal.

A review of current traffic patterns at the Study Area intersections and in the vicinity of the project influence area was conducted to determine trip distribution for the proposed development. For arriving site traffic, it was found that 40 percent will arrive from the east on U.S. Route 1, 35 percent will arrive from the west on U.S. Route 1, 15 percent will arrive from the north on Lafayette Street and 10 percent will arrive from the south on South State Street. For departing site traffic, it was found that 40 percent will depart to the east on U.S. Route 1 and 60 percent will depart to the west on U.S. Route 1.

SYNCHRO 10 capacity analyses were conducted for 2021 existing, 2024 no-build and 2024 build conditions to identify incremental impacts and needs that the proposed development will generate. Results of the analyses indicate that the signalized intersection of U.S. Route 1 at Lafayette Street will continue to operate at the same overall Level of Service during both peak hours with a minimal increase in vehicle delay. The westbound and northbound lane groups and approaches will have an acceptable change in Level of Service from "A" to "B" and "B" to "C," respectively, during the weekday morning peak hour. The eastbound lane group and approach will have an acceptable change in Level of Service from "A" to "B" during the weekday afternoon peak hour. The southbound left turn lane group and approach will maintain a Level of Service "E" during both peak hours.

The signalized intersection of U.S. Route 1 and North State Street will continue to operate at the same overall Level of Service during both peak hours with a minimal increase in vehicle delay. All lane groups and approaches will maintain the same Levels of Service during both peak hours. At the all-way STOP-controlled intersection of North State Street and Lafayette Street/South State Street, all critical lanes and the intersection overall will maintain the same Level of Service during both peak hours. The proposed site driveway will operate at Level of Service "A" during both peak hours.

## APPENDIX

PHOTOGRAPHS

U.S. Route 1 at Lafayette Street Looking East

U.S. Route 1 at Lafayette Street Looking West


Lafayette Street at U.S. Route 1 Looking North


Lafayette Street at U.S. Route 1 Looking South


## U.S. Route 1 at North State Street Looking East


U.S. Route 1 at North State Street Looking West


North State Street at U.S. Route 1 Looking South


North State Street at South State Street-Lafayette Street Looking East


North State Street at South State Street-Lafayette Street Looking West


Lafayette Street at North State Street Looking North


South State Street at North State Street Looking South

TURNING MOVEMENT COUNTS
MIXED-USE DEVELOPMENT, 819 EAST MAIN STREET, STAMFORD, CONNECTICUT (\#05498.00)
FIELD DATA SUMMARY - U.S. Route 1 (East Main St) at Lafayette St

| Wednesday 15-Dec-21 |  | Eastbound - U.S. Route 1 |  |  |  | Westbound-U.S. Route 1 |  |  |  | Northbound - Lafayefte St |  |  |  | Southbound - Lafayette St |  |  |  | Total | $\begin{gathered} \hline \text { Last 4 } \\ \text { Quarters } \\ \hline \end{gathered}$ | Pedestrians |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Left | Thru | Right | Total | Left | Thru | Right | Total | Left | Thru | Right | Total | Left | Thru | Right | Total |  |  | EB | WB 1 | NB 2 | ${ }^{\text {SB }}$ |
| 7:00 AM | 7:15 AM | 0 | 75 | 0 | 75 | 0 | 132 | 0 | 132 | 3 | 0 | 2 | 5 | 34 |  | 16 | 51 | 263 |  |  |  |  |  |
| 7:15 AM | 7.30 AM | 0 | 134 | 1 | 135 | 0 | 183 | 9 | 192 | 2 | 0 | 10 | 12 | 87 | 1 | 14 | 102 | 441 |  | 5 | 0 | 1 | 5 |
| 7:30 AM | 7:45 AM | 0 | 107 | 0 | 107 | 1 | 213 | 0 | 214 | 5 | 0 | 0 | 5 | 42 | 0 | 15 | 57 | 383 |  | 1 | 0 | 1 | 2 |
| 7:45 AM | 8:00 AM | 0 | 101 | 0 | 101 | 1 | 217 | 0 | 218 | 6 | 0 | 2 | 8 | 61 | 0 | 12 | 73 | 400 | 1,487 | 0 | 2 | 2 | 2 |
| 8:00 AM | 8:15 AM | 0 | 108 | 0 | 108 | 1 | 183 | 0 | 184 | 5 | 0 | 7 | 12 | 32 | ${ }^{2}$ | 10 | 44 | 348 | 4.772. | 0 | 0 | 0 |  |
| 8:15 AM | 8:30 AM | 0 | 93 | 7 | 100 | 0 | 233 | 0 | 233 | 9 | 0 | 3 | 12 | 46 | 3 | 11 | 60 | 405 | 1,536 | 1 | 1 | 3 | 2 |
| 8:30 AM | 8:45 AM | 0 | 86 | 0 | 86 | 0 | 154 | 0 | 154 | 6 | 0 | 1 | 7 | 35 | 2 | 3 | 40 | 287 | 1,440 | 0 | 0 | 0 | 2 |
| 8:45 AM | 9:00 AM | 0 | 111 | 3 | 114 | 1 | 196 | 0 | 197 | 10 | 0 | 4 | 14 | 33 | 1 | 8 | 42 | 367 | 1,407 | 1 | 1 | 2 | 3 |
| AM Paak Hour Vol. |  | 0 | 450 | 1 | 451 | 3 | 796 |  | 808 | 18 | 0 | 19 | 37 | 222 | 3 | 51 | 276 | 1,572 |  | 6 | 2 | 4 | 10 |
| Peak Hour Factor |  |  |  | - 0.84 |  |  |  |  | 0.93 |  |  |  | 0.77 |  |  |  | 0.68 | 0.89 |  |  |  |  |  |
| Tuesday 14-Dec-21 |  | Eastbound - U.S. Route 1 |  |  |  | Westhound - U.S. Route 1 |  |  |  | Northbound - Lafayette St |  |  |  | Southbound - Lafayette St |  |  |  | Total | Last 4 Quarters | Pedestrians |  |  |  |
|  |  | Left | Thru | Right | Total | Left | Thru | Right | Total | Left | Thru | Right | Total | Left | Thru | Right | Total |  |  | EB | WB | NB | SB |
| 400 P4919 | 4:15 PM | 1 | 160 | 4 | 165 | 0 | 132 | 1 | 133 | 8 | 6 | 9 | 17 | 40 | 1 | 6 | 47 | 362 |  | 3 | 0 | 3 | 2 |
| 4:15 PM | 4:30 PM | 0 | 189 | 2 | 191 | 0 | 158 | 1 | 158 | 12 | 0 | 10 | 22 | 33 | 1 | 8 | 42 | 414 |  | 0 | 2 | 2 | 7 |
| 4:30 PM | 4:45 PM | 0 | 260 | 2 | 262 | 2 | 200 | 0 | 202 | 11 | 0 | 13 | 24 | 60 | 0 | 12 | 72 | 560 |  | 4 | 3 | 4 | 7 |
| 4:45 PM | 5:00 PM | 0 | 114 | 2 | 116 | 1 | 115 | 0 | 116 | 8 | 0 | 8 | 16 | 39 | 0 | 5 | 44 | 292 | 1,628 | , | 2 | 3 | 2 |
| 5:00 PM | 5:15 PM | 0 | 175 | 0 | 175 | 0 | 141 | 0 | 141 | 5 | 0 | 9 | 14 | 38 | 0 | 19 | 57 | 387 | 1,653 | 3 | 2 | 5 | 3 |
| 5:15 PM | 5:30 PM | 0 | 169 | 1 | 170 | 0 | 147 | 0 | 147 | 6 | 0 | 9 | 15 | 39 | 1 | 14 | 54 | 386 | 1,625 | 0 | 0 | 2 | 0 |
| 5:30 PM | 5:45 PM | 0 | 210 | 1 | 211 | 3 | 195 | 0 | 198 | 15 | 0 | 14 | 29 | 44 | 0 | 11 | 55 | 493 | 1,558 | 5 | 0 | 6 | 3 |
| 5:45 PM | 6:00 PM | 0 | 168 | 0 | 168 | 0 | 111 | 0 | 111 | 7 | 0 | 5 | 12 | 52 | 0 | 12 | 64 | 355 | 1,621 | 1 | 1 | 5 | 0 |
| PM Peak Hour Yol. |  | 1 | 723 | 10 | 734 | 3 | 605 | 2 | 610 | 39 | 0 | 40 | 79 | 172 | 2 | 31 | 205 | 1,628 |  | 10 | 7 | 12 | 18 |
| Peak Hour Factor |  |  |  |  | 0.70 |  |  |  | 0.75 |  |  |  | 0.82 |  |  |  | 0.71 | 0.73 |  |  |  |  |  |

MIXED-USE DEVELOPMENT, 819 EAST MAN STREET, STAMFORD, CONNECTICUT (\#05498.00)
FIELD DATA SUMMARY - U.S. Route 1 (East Main St) at N State St

| $\begin{aligned} & \text { Wednesday } \\ & \text { 15-Dec-21 } \end{aligned}$ |  | Eastbound - U.S. Route 1 |  |  |  | Westbound - U.S. Route 1 |  |  |  | Northbound - N State St |  |  |  | Southbound - Commercial Drivaway |  |  |  | Total | Last 4 Quarters | Pedestrians |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Left | Thru | $\begin{array}{r} \text { Right } \\ 0 \end{array}$ | Total | Left | Thru | RIght | Total | Left | Thru | Right | Total | Left | Thru | Right | Total |  |  | EB | WB NB ${ }^{\text {W }}$ |  |  |
| 7:00 AM | 7:15 AM | 8 | 103 |  |  | 0 | 134 | 12 | 146 | 0 | 1 | 12 | 13 | 6 | 0 | 6 | 12 | 282 |  | 3 | 0 | 3 |  |
| 7:15 AM | 7:30 AM | 3 | 186 | 0 | 189 | 1 | 185 | 11 | 197 | 0 | 1 | 11 | 12 | 8 | 0 | 11 | 19 | 417 |  | 5 | 0 | 0 | 1 |
| 7:30 AM | 7:45 AM | 6 | 150 | 0 | 156 | 2 | 185 | 13 | 200 | 3 | 0 | 17 | 20 | 12 | 0 | 4 | 16. | 392 |  | 2 | 0 | 0 | 0 |
| 7:45 AM | 8:00 AM | 3 | 179 | 0 | 182 | 1 | 197 | 12 | 210 | 1 | 1 | 14 | 16 | 16 | 1 | 5 | 22 | 430 | 1.521 | 3 | 0 | 0 | 0 |
| 8:00 AM | 8:15 AM | 4 | 182 | 0 | 186 | 4 | 182 | 10 | 196 | 3 | 2 | 15 | 20 | 11 | 0 | 5 | ${ }^{4} 5$ | 418 | 1,657 | 1 | 0 | 0 | 1 |
| 8:15 AM | 8:30 AM | 3 | 146 | 0 | 149 | 2 | 212 | 9 | 223 | 2 | 1 | 21 | 24 | 5 | 0 | 5 | 10 | 406 | 1,646 | 2 | 0 | 0 | 3 |
| 8:30 AM | 8:45 AM | 2 | 152 | 2 | 156 | 2 | 151 | 7 | 160 | 1 | 0 | 8 | 9 | 5 | 0 | 2 | 7 | 332 | 1,586 | 1 | 0 | 0 | 0 |
| 8:45 AM | 9:00 AM | 6 | 130 | 1 | 137 | 2 | 180 | 2 | 184 | 0 | 0 | 9 | 9 | 5 | 0 | 3 | 8 | 338 | 1,494 | 0 | 0 | 0 | 0 |
| AM Peak Hour Vol. |  | 16 | 697 | 0 | 713 | 8 | 749 | 46 | 803 | 7 | 4 | 57 | 68 | 47 | 1 | 25 | 73 | 1,657 |  | 11 | 0 | 0 | 2 |
| Peak Hour Factor |  |  |  |  | 0.94 |  |  |  | 0.96 |  |  |  | 0.85 |  |  |  | 0.83 | 0.96 |  |  |  |  |  |
| Tuesday 14-Dec-21 |  | Easthound- -U.S. Route 1 |  |  |  | Westbound - U.S. Route 1 |  |  |  | Northbound - N State St |  |  |  | Southbound - Commercial Drivoway |  |  |  | Total | $\begin{array}{\|c\|} \hline \text { Last 4 } \\ \text { Quarters } \\ \hline \end{array}$ | Pedestrians |  |  |  |
|  |  | Left | Thru | Right | Total | Left | Thru | Right | Total | Left | Thru | Right | Total | Left | Thru | Right | Total |  |  | EB | WB | NB | SB |
| 4:00 PM | 4:15PM | 3 | 224 |  | 228 | 3 | 149 | 8 | 160 | 0 | 0 | 38 | 38 | 3 | 0 | 5 | 8 | 134 |  | 1 | 0 | 0 | 4 |
| 4:15 PM | 4:30 PM | 2 | 199 | 2 | 203 | 1 | 147 | 1 | 149 | 0 | 0 | 31 | 31 | 4 | 0 | 2 | 6 | 389 |  | 0 | 0 | 0 | 8 |
| 4:30 FM | 4:45 PM | 5 | 233 | 0 | 238 | 1 | 157 | 2 | 160 | 0 | 0 | 32 | 32 | 3 | 0 | 3 | 6 | 436 |  | 0 | 0 | 0 | 2 |
| $4: 45$ PM | 5:00. PM | 1 | 285 | 0 | 286 | 1 | 173 | 3 | 177 | 1 | 1 | 36 | 38 | 7 | 0 | 2 | 9 | 510 | 1.769 | 2 | 0 | 2 | 3 |
| 5:00 PM | 5:15 PM | 3 | 137 | 0 | 140 | 1 | 146 | 3 | 150 | 4 | 2 | 16 | 22 | 1 | 0 | 2 | 3 | 315 | 1,650 | 0 | 0 | 0 | 1 |
| 5:15 PM | 5:30 PM | 5 | 215 | 1 | 221 | 0 | 162 | 8 | 170 | 3 | 0 | 28 | 31 | 5 | 0 | 6 | 11 | 433 | 1,694 | 1 | 0 | 0 | 1 |
| 5:30 PM | 5:45 PM | 1 | 252 | 2 | 255 | 1 | 158 | 6 | 165 | 1 | 2 | 52 | 55 | 7 | 0 | 9 | 16 | 491 | 1,749 | 2 | 0 | 0 | 3 |
| 5:45 PM | 6:00 PM | 3 | 167 | 2 | 172 | 0 | 154 | 4 | 158 | 0 | 1 | 29 | 30 | 2 | 0 | 4 | 6 | 366 | 1,605 | 2 | 0 | 0 | 2 |
| PM Peak Hour Vol. |  | 11 | 941 | 3 | 955 | 6 | 626 | 14 | 646 |  | 1 | 137 | 139 | 17 | 0 | 12 | 29 | 1,769 |  | 3 | 0 | 2 | 17 |
| Peak Hour Factor |  |  |  |  | 0.83 |  |  |  | 0.91 |  |  |  | 0.91 |  |  |  | 0.81 | 0.87 |  |  |  |  |  |

MIXED-USE DEVELOPMENT, 819 EAST MAIN STREET, STAMFORD, CONNECTICUT (\#05498.00)
FIELD DATA SUMMARY - Lafayette St at $N$ State St

MIXED-USE DEVELOPMENT, 819 EAST MAIN STREET, STAMFORD, CONNECTICUT (\#05498.00)
FIELD DATA SUMMARY - Lafayette St at S State St


# Connecticut Counts LLC <br> Kensington, Connecticut 06037 <br> (860) 828-1693 

Route 1 at Lafayette Street Stamford, Connecticut

File Name : 22487
Site Code : 22487
Start Date : 12/15/2021
Page No : 1

|  | Lafayette Street From North |  |  |  |  | Route 1 <br> From East |  |  |  |  | Lafayette Street From South |  |  |  |  | Route 1 From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | ATs. Total | Right | Thru | Left | Peds | Af. Toral | Ripht | Thru | Left | Peds | $A_{\text {a }}$. Toul | Ripht | Thru | Left | Peds | AT: Toal | Int. Toal |
| 07:00 AM | 16 | 1 | 34 | 4 | 55 | 0 | 132 | 0 | 1 | 133 | 2 | 0 | 3 | 2 | 7 | 0 | 75 | 0 | 1 | 76 | 271 |
| 07:15 AM | 14 | 1 | 87 | 5 | 107 | 9 | 183 | 0 | 0 | 192 | 10 | 0 | 2 | 1 | 13 | 1 | 134 | 0 | 5 | 140 | 452 |
| 07:30 AM | 15 | 0 | 42 | 2 | 59 | 0 | 213 | 1 | 0 | 214 | 0 | 0 | 5 | 1 | 6 | 0 | 107 | 0 | 1 | 108 | 387 |
| 07:45 AM | 12 | 0 | 61 | 2 | 75 | 0 | 217 | 1 | 2 | 220 | 2 | 0 | 6 | 2 | 10 | 0 | 101 | 0 | 0 | 101 | 406 |
| Total | 57 | 2 | 224 | 13 | 296 | 9 | 745 | 2 | 3 | 759 | 14 | 0 | 16 | 6 | 36 | 1 | 417 | 0 | 7 | 425 | 1516 |
| 08:00 AM | 10 | 2 | 32 | 1 | 45 | 0 | 183 | 1 | 0 | 184 | 7 | 0 | 5 | 0 | 12 | 0 | 108 | 0 | 0 | 108 | 349 |
| 08:15 AM | 11 | 3 | 46 | 2 | 62 | 0 | 233 | 0 | 1 | 234 | 3 | 0 | 9 | 3 | 15 | 7 | 93 | 0 | 1 | 101 | 412 |
| 08:30 AM | 3 | 2 | 35 | 2 | 42 | 0 | 154 | 0 | 0 | 154 | 1 | 0 | 6 | 0 | 7 | 0 | 86 | 0 | 0 | 86 | 289 |
| 08:45 AM | 8 | 1 | 33 | 3 | 45 | 0 | 196 | 1 | 1 | 198 | 4 | 0 | 10 | 2 | 16 | 3 | 111 | 0 | 1 | 115 | 374 |
| Total | 32 | 8 | 146 | 8 | 194 | 0 | 766 | 2 | 2 | 770 | 15 | 0 | 30 | 5 | 50 | 10 | 398 | 0 | 2 | 410 | 1424 |
| Grand Total | 89 | 10 | 370 | 21 | 490 | 9 | 1511 | 4 | 5 | 1529 | 29 | 0 | 46 | 11 | 86 | 11 | 815 | 0 | 9 | 835 | 2940 |
| Apprch \% | 18.2 | 2 | 75.5 | 4.3 |  | 0.6 | 98.8 | 0.3 | 0.3 |  | 33.7 | 0 | 53.5 | 12.8 |  | 1.3 | 97.6 | 0 | 1.1 |  |  |
| Total \% | 3 | 0.3 | 12.6 | 0.7 | 16.7 | 0.3 | 51.4 | 0.1 | 0.2 | 52 | 1 | 0 | 1.6 | 0.4 | 2.9 | 0.4 | 27.7 | 0 | 0.3 | 28.4 |  |
| Lights | 85 | 7 | 355 | 21 | 468 | 9 | 1477 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| \% Lights | 95.5 | 70 | 95.9 | 100 | 95.5 | 100 | 97.7 | 100 | 100 | 97.8 | 96.6 | 0 | 95.7 | 100 | 96.5 | 90.9 | 95.1 | 0 | 100 | 95.1 | 96.6 |
| Trucks | 0 | 0 | 6 | 0 | 6 | 0 | 18 | 0 | 0 | 18 | 0 | 0 | 2 | 0 | 2 | 0 | 9 | 0 | 0 | 9 | 35 |
| \% Trucks | 0 | 0 | 1.6 | 0 | 1.2 | 0 | 1.2 | 0 | 0 | 1.2 | 0 | 0 | 4.3 | 0 | 2.3 | 0 | 1.1 | 0 | 0 | 1.1 | 1.2 |
| Buses | 4 | 3 | 9 | 0 | 16 | 0 | 16 | 0 | 0 | 16 | 1 | 0 | 0 | 0 | 1 | 1 | 31 | 0 | 0 | 32 | 65 |
| \% Buses | 4.5 | 30 | 2.4 | 0 | 3.3 | 0 | 1.1 | 0 | 0 | 1 | 3.4 | 0 | 0 | 0 | 1.2 | 9.1 | 3.8 | 0 | 0 | 3.8 | 2.2 |

## Connecticut Counts LLC Kensington, Connecticut 06037 (860) 828-1693

File Name : 22487
Site Code : 22487
Start Date : 12/15/2021
Page No : 2

|  | Lafayette Street From North |  |  |  |  | Route 1 <br> From East |  |  |  |  | Lafayette Street From South |  |  |  |  | Route 1 From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start <br> Time | Right | Thru | Left | Peds | App. Trial | Right | Thru | Left | Peds | App. Tral | Right | Thru | Left | Peds | App. Toual | Right | Thru | Left | Peds | App. Toua | lnt. Toral |
| Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 07:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 07:15 AM | 14 | 1 | 87 | 5 | 107 | 9 | 183 | 0 | 0 | 192 | 10 | 0 | 2 | 1 | 13 | 1 | 134 | 0 | 5 | 140 | 452 |
| 07:30 AM | 15 | 0 | 42 | 2 | 59 | 0 | 213 | 1 | 0 | 214 | 0 | 0 | 5 | 1 | 6 | 0 | 107 | 0 | 1 | 108 | 387 |
| 07:45 AM | 12 | 0 | 61 | 2 | 75 | 0 | 217 | 1 | 2 | 220 | 2 | 0 | 6 | 2 | 10 | 0 | 101 | 0 | 0 | 101 | 406 |
| 08:00 AM | 10 | 2 | 32 | 1 | 45 | 0 | 183 | 1 | 0 | 184 | 7 | 0 | 5 | 0 | 12 | 0 | 108 | 0 | 0 | 108 | 349 |
| Total Volume | 51 | 3 | 222 | 10 | 286 | 9 | 796 | 3 | 2 | 810 | 19 | 0 | 18 | 4 | 41 | 1 | 450 | 0 | 6 | 457 | 1594 |
| \% App. Total | 17.8 | 1 | 77.6 | 3.5 |  | 1.1 | 98.3 | 0.4 | 0.2 |  | 46.3 | 0 | 43.9 | 9.8 |  | 0.2 | 98.5 | 0 | 1.3 |  |  |
| PHF | . 850 | . 375 | . 638 | . 500 | . 668 | . 250 | . 917 | . 750 | . 250 | . 920 | . 475 | . 000 | . 750 | . 500 | . 788 | . 250 | . 840 | . 000 | . 300 | . 816 | . 882 |



## Connecticut Counts LLC <br> Kensington, Connecticut 06037 <br> (860) 828-1693

File Name : 22487
Site Code : 22487
Start Date : 12/15/2021
Page No : 3

|  | Lafayette Street From North |  |  |  |  | Route 1 <br> From East |  |  |  |  | Lafayette Street From South |  |  |  |  | Route 1 From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start <br> Time | Right | Thru | Left | Peds | App. Toul | Right | Thru | Left | Peds | App. Tepl | Right | Thru | Left | Peds | Appr. Tual | Right | Thru | Left | Peds | App. Toal | Int. Total |

Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

|  | 07:00 AM |  |  |  |  | 07:30 AM |  |  |  |  | 08.00 AM |  |  |  |  | 07:15 AM |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| +0 mins. | 16 | 1 | 34 | 4 | 55 | 0 | 213 | 1 | 0 | 214 | 7 | 0 | 5 | 0 | 12 | 1 | 134 | 0 | 5 | 140 |
| +15 mins. | 14 | 1 | 87 | 5 | 107 | 0 | 217 | 1 | 2 | 220 | 3 | 0 | 9 | 3 | 15 | 0 | 107 | 0 | 1 | 108 |
| +30 mins. | 15 | 0 | 42 | 2 | 59 | 0 | 183 | 1 | 0 | 184 | 1 | 0 | 6 | 0 | 7 | 0 | 101 | 0 | 0 | 101 |
| +45 mins. | 12 | 0 | 61 | 2 | 75 | 0 | 233 | 0 | 1 | 234 | 4 | 0 | 10 | 2 | 16 | 0 | 108 | 0 | 0 | 108 |
| Total Volume | 57 | 2 | 224 | 13 | 296 | 0 | 846 | 3 | 3 | 852 | 15 | 0 | 30 | 5 | 50 | 1 | 450 | 0 | 6 | 457 |
| \% App. Total | 19.3 | 0.7 | 75.7 | 4.4 |  | 0 | 99.3 | 0.4 | 0.4 |  | 30 | 0 | 60 | 10 |  | 0.2 | 98.5 | 0 | 1.3 |  |
| PHF | . 891 | . 500 | . 644 | . 650 | . 692 | . 000 | . 908 | . 750 | . 375 | . 910 | . 536 | . 000 | . 750 | 417 | . 781 | . 250 | . 840 | . 000 | . 300 | . 816 |


|  |  |  |
| :---: | :---: | :---: |
|  | Peak Hour Data <br> Lights <br> Trucks <br> Buses |  |
|  | In - Peak Hour: 08:00 AM Lafavette Street |  |

## Connecticut Counts LLC <br> Kensington, Connecticut 06037 <br> (860) 828-1693

Route 1 at Lafayette Street
Stamford, Connecticut
File Name : 22488
Site Code : 22488
Start Date : 12/14/2021
Page No : 1

|  | Lafayette Street From North |  |  |  |  | Route 1 <br> From East |  |  |  |  | Lafayette Street From South |  |  |  |  | Route 1 From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Ripht | Thru | Left | Peds | Av. Total | Right | Thru | Left | Peds | Apra Toral | Right | Thru | Left | Peds | As. Tolal | Right | Thru | Left | Peds | An Total | Int. Total |
| 04:00 PM | 6 | 1 | 40 | 2 | 49 | 1 | 132 | 0 | 0 | 133 | 9 | 0 | 8 | 3 | 20 | 4 | 160 | , | 3 | 168 | 370 |
| 04:15 PM | 8 | 1 | 33 | 7 | 49 | 1 | 158 | 0 | 2 | 161 | 10 | 0 | 12 | 2 | 24 | 2 | 189 | 0 | 0 | 191 | 425 |
| 04:30 PM | 12 | 0 | 60 | 7 | 79 | 0 | 200 | 2 | 3 | 205 | 13 | 0 | 11 | 4 | 28 | 2 | 260 | 0 | 4 | 266 | 578 |
| 04:45 PM | 5 | 0 | 39 | 2 | 46 | 0 | 115 | 1 | 2 | 118 | 8 | 0 | 8 | 3 | 19 | 2 | 114 | 0 | 3 | 119 | 302 |
| Total | 31 | 2 | 172 | 18 | 223 | 2 | 605 | 3 | 7 | 617 | 40 | 0 | 39 | 12 | 91 | 10 | 723 | 1 | 10 | 744 | 1675 |
| 05:00 PM | 19 | 0 | 38 | 3 | 60 | 0 | 141 | 0 | 2 | 143 | 9 | 0 | 5 | 5 | 19 | 0 | 175 | 0 | 3 | 178 | 400 |
| 05:15 PM | 14 | 1 | 39 | 0 | 54 | 0 | 147 | 0 | 0 | 147 | 9 | 0 | 6 | 2 | 17 | 1 | 169 | 0 | 0 | 170 | 388 |
| 05:30 PM | 11 | 0 | 44 | 3 | 58 | 0 | 195 | 3 | 0 | 198 | 14 | 0 | 15 | 6 | 35 | 1 | 210 | 0 | 5 | 216 | 507 |
| 05:45 PM | 12 | 0 | 52 | 0 | 64 | 0 | 111 | 0 | 1 | 112 | 5 | 0 | 7 | 5 | 17 | 0 | 168 | 0 | 1 | 169 | 362 |
| Total | 56 | 1 | 173 | 6 | 236 | 0 | 594 | 3 | 3 | 600 | 37 | 0 | 33 | 18 | 88 | 2 | 722 | 0 | 9 | 733 | 1657 |
| Grand Total | 87 | 3 | 345 | 24 | 459 | 2 | 1199 | 6 | 10 | 1217 | 77 | 0 | 72 | 30 | 179 | 12 | 1445 | 1 | 19 | 1477 | 3332 |
| Apprch \% | 19 | 0.7 | 75.2 | 5.2 |  | 0.2 | 98.5 | 0.5 | 0.8 |  | 43 | 0 | 40.2 | 16.8 |  | 0.8 | 97.8 | 0.1 | 1.3 |  |  |
| Total \% | 2.6 | 0.1 | 10.4 | 0.7 | 13.8 | 0.1 | 36 | 0.2 | 0.3 | 36.5 | 2.3 | 0 | 2.2 | 0.9 | 5.4 | 0.4 | 43.4 | 0 | 0.6 | 44.3 |  |
| Lights | 85 | 3 | 339 | 24 | 451 | 1 | 1186 |  |  |  |  |  |  |  |  |  | 1427 |  |  |  |  |
| \% Lights | 97.7 | 100 | 98.3 | 100 | 98.3 | 50 | 98.9 | 100 | 100 | 98.8 | 100 | 0 | 100 | 100 | 100 | 100 | 98.8 | 100 | 100 | 98.8 | 98.8 |
| Trucks | 0 | 0 | 4 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 0 | 5 | 9 |
| \% Trucks | 0 | 0 | 1.2 | 0 | 0.9 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0.3 | 0 | 0 | 0.3 | 0.3 |
| Buses | 2 | 0 | 2 | 0 | 4 | 1 | 13 | 0 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 13 | 0 | 0 | 13 | 31 |
| \% Buses | 2.3 | 0 | 0.6 | 0 | 0.9 | 50 | 1.1 | 0 | 0 | 1.2 | 0 | 0 | 0 | 0 | 0 | 0 | 0.9 | 0 | 0 | 0.9 | 0.9 |

## Connecticut Counts LLC <br> Kensington, Connecticut 06037 <br> (860) 828-1693

File Name : 22488
Site Code : 22488
Start Date : 12/14/2021
Page No : 2

|  | Lafayette Street From North |  |  |  |  | Route 1 From East |  |  |  |  | Lafayette Street From South |  |  |  |  | Route 1 From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start <br> Time | Right | Thru | Left | Peds | App. Tonal | Right | Thru | Left | Peds | App Tomi | Right | Thru | Left | Peds | App Tusa | Right | Thru | Left | Peds | App. Toal | Int. Tetal |
| Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 04:15 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 04:15 PM | 8 | 1 | 33 | 7 | 49 | 1 | 158 | 0 | 2 | 161 | 10 | 0 | 12 | 2 | 24 | 2 | 189 | 0 | 0 | 191 | 425 |
| 04:30 PM | 12 | 0 | 60 | 7 | 79 | 0 | 200 | 2 | 3 | 205 | 13 | 0 | 11 | 4 | 28 | 2 | 260 | 0 | 4 | 266 | 578 |
| 04:45 PM | 5 | 0 | 39 | 2 | 46 | 0 | 115 | 1 | 2 | 118 | 8 | 0 | 8 | 3 | 19 | 2 | 114 | 0 | 3 | 119 | 302 |
| 05:00 PM | 19 | 0 | 38 | 3 | 60 | 0 | 141 | 0 | 2 | 143 | 9 | 0 | 5 | 5 | 19 | 0 | 175 | 0 | 3 | 178 | 400 |
| Total Volume | 44 | 1 | 170 | 19 | 234 | 1 | 614 | 3 | 9 | 627 | 40 | 0 | 36 | 14 | 90 | 6 | 738 | 0 | 10 | 754 | 1705 |
| \% App. Total | 18.8 | 0.4 | 72.6 | 8.1 |  | 0.2 | 97.9 | 0.5 | 1.4 |  | 44.4 | 0 | 40 | 15.6 |  | 0.8 | 97.9 | 0 | 1.3 |  |  |
| PHF | . 579 | . 250 | . 708 | . 679 | . 741 | . 250 | . 768 | . 375 | . 750 | . 765 | . 769 | . 000 | . 750 | . 700 | . 804 | . 750 | . 710 | . 000 | . 625 | . 709 | . 737 |



## Connecticut Counts LLC <br> Kensington, Connecticut 06037 <br> (860) 828-1693

File Name : 22488
Site Code : 22488
Start Date : 12/14/2021
Page No : 3

|  | Lafayette Street From North |  |  |  |  | Route 1 From East |  |  |  |  | Lafayette Street From South |  |  |  |  | Route 1 From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | ${ }_{\text {Anp }}$ Toral | Right | Thru | Left | Peds | Apm. Toas | Right | Thru | Left | Peds | Aff. Toal | Right | Thru | Left | Peds | App. Tetal | In. Tonal |

Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

|  | 04:30 PM |  |  |  |  | 04:15 PM |  |  |  |  | 04:00 PM |  |  |  |  | 04:15PM |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| +0 mins. | 12 | 0 | 60 | 7 | 79 | 1 | 158 | 0 | 2 | 161 | 9 | 0 | 8 | 3 | 20 | 2 | 189 | 0 | 0 | 191 |
| +15 mins. | 5 | 0 | 39 | 2 | 46 | 0 | 200 | 2 | 3 | 205 | 10 | 0 | 12 | 2 | 24 | 2 | 260 | 0 | 4 | 266 |
| + 30 mins. | 19 | 0 | 38 | 3 | 60 | 0 | 115 | 1 | 2 | 118 | 13 | 0 | 11 | 4 | 28 | 2 | 114 | 0 | 3 | 119 |
| +45 mins. | 14 | 1 | 39 | 0 | 54 | 0 | 141 | 0 | 2 | 143 | 8 | 0 | 8 | 3 | 19 | 0 | 175 | 0 | 3 | 178 |
| Total Volume | 50 | 1 | 176 | 12 | 239 | 1 | 614 | 3 | 9 | 627 | 40 | 0 | 39 | 12 | 91 | 6 | 738 | 0 | 10 | 754 |
| \% App. Total | 20.9 | 0.4 | 73.6 | 5 |  | 0.2 | 97.9 | 0.5 | 1.4 |  | 44 | 0 | 42.9 | 13.2 |  | 0.8 | 97.9 | 0 | 1.3 |  |
| PHF | . 658 | . 250 | . 733 | . 429 | . 756 | . 250 | . 768 | . 375 | . 750 | . 765 | . 769 | . 000 | . 813 | . 750 | . 813 | . 750 | . 710 | . 000 | . 625 | . 709 |



# Connecticut Counts LLC <br> Kensington, Connecticut 06037 <br> (860) 828-1693 

Route 1 at N. State St/Private Dr
Stamford, Connecticut

File Name : 22489
Site Code : 22489
Start Date : 12/15/2021
Page No : 1

|  | Private Drive From North |  |  |  |  | Route 1 <br> From East |  |  |  |  | N. State Street From South |  |  |  |  | Route 1 From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | Afp. Tolal | Ripht | Thru | Left | Peds | Afl. Total | Right | Thru | Left | Peds | Afy Tosal | Right | Thru | Left | Peds | Amp Tata | In. Toral |
| 07:00 AM | 6 | 0 | 6 | 2 | 14 | 12 | 134 | 0 | 0 | 146 | 12 | 1 | 0 | 3 | 16 | 0 | 103 | 8 | 3 | 114 | 290 |
| 07:15 AM | 11 | 0 | 8 | 1 | 20 | 11 | 185 | 1 | 0 | 197 | 11 | 1 | 0 | 0 | 12 | 0 | 186 | 3 | 5 | 194 | 423 |
| 07:30 AM | 4 | 0 | 12 | 0 | 16 | 13 | 185 | 2 | 0 | 200 | 17 | 0 | 3 | 0 | 20 | 0 | 150 | 6 | 2 | 158 | 394 |
| 07:45 AM | 5 | 1 | 16 | 0 | 22 | 12 | 197 | 1 | 0 | 210 | 14 | 1 | 1 | 0 | 16 | 0 | 179 | 3 | 3 | 185 | 433 |
| Total | 26 | 1 | 42 | 3 | 72 | 48 | 701 | 4 | 0 | 753 | 54 | 3 | 4 | 3 | 64 | 0 | 618 | 20 | 13 | 651 | 1540 |
| 08:00 AM | 5 | 0 | 11 | 1 | 17 | 10 | 182 | 4 | 0 | 196 | 15 | 2 | 3 | 0 | 20 | 0 | 182 | 4 | 1 | 187 | 420 |
| 08:15 AM | 5 | 0 | 5 | 3 | 13 | 9 | 212 | 2 | 0 | 223 | 21 | 1 | 2 | 0 | 24 | 0 | 146 | 3 | 2 | 151 | 411 |
| 08:30 AM | 2 | 0 | 5 | 0 | 7 | 7 | 151 | 2 | 0 | 160 | 8 | 0 | 1 | 0 | 9 | 2 | 152 | 2 | 1 | 157 | 333 |
| 08:45 AM | 3 | 0 | 5 | 0 | 8 | 2 | 180 | 2 | 0 | 184 | 9 | 0 | 0 | 0 | 9 | 1 | 130 | 6 | 0 | 137 | 338 |
| Total | 15 | 0 | 26 | 4 | 45 | 28 | 725 | 10 | 0 | 763 | 53 | 3 | 6 | 0 | 62 | 3 | 610 | 15 | 4 | 632 | 1502 |
| Grand Total | 41 | 1 | 68 | 7 | 117 | 76 | 1426 | 14 | 0 | 1516 | 107 | 6 | 10 | 3 | 126 | 3 | 1228 | 35 | 17 | 1283 | 3042 |
| Apprch \% | 35 | 0.9 | 58.1 | 6 |  | 5 | 94.1 | 0.9 | 0 |  | 84.9 | 4.8 | 7.9 | 2.4 |  | 0.2 | 95.7 | 2.7 | 1.3 |  |  |
| Total \% | 1.3 | 0 | 2.2 | 0.2 | 3.8 | 2.5 | 46.9 | 0.5 | 0 | 49.8 | 3.5 | 0.2 | 0.3 | 0.1 | 4.1 | 0.1 | 40.4 | 1.2 | 0.6 | 42.2 |  |
| Lights | 41 | 1 | 68 | 7 | 117 | 76 | 1397 |  |  |  |  |  |  |  |  |  | 1177 |  |  |  |  |
| \% Lights | 100 | 100 | 100 | 100 | 100 | 100 | 98 | 85.7 | 0 | 98 | 100 | 100 | 90 | 100 | 99.2 | 100 | 95.8 | 100 | 100 | 96 | 97.3 |
| Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 14 | 0 | 0 | 14 | 0 | 0 | 1 | 0 | 1 | 0 | 8 | 0 | 0 | 8 | 23 |
| \% Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0.9 | 0 | 0 | 10 | 0 | 0.8 | 0 | 0.7 | 0 | 0 | 0.6 | 0.8 |
| Buses | 0 | 0 | 0 | 0 | 0 | 0 | 15 | 2 | 0 | 17 | 0 | 0 | 0 | 0 | 0 | 0 | 43 | 0 | 0 | 43 | 60 |
| \% Buses | 0 | 0 | 0 | 0 | 0 | 0 | 1.1 | 14.3 | 0 | 1.1 | 0 | 0 | 0 | 0 | 0 | 0 | 3.5 | 0 | 0 | 3.4 | 2 |

## Connecticut Counts LLC <br> Kensington, Connecticut 06037 <br> (860) 828-1693

File Name : 22489
Site Code : 22489
Start Date : 12/15/2021
Page No : 2

|  | Private Drive From North |  |  |  |  | Route 1 From East |  |  |  |  | N. State Street From South |  |  |  |  | Route 1 From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start <br> Time | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | Afp. Tora | Right | Thru | Left | Peds | Aff. Tmal | Right | Thru | Left | Peds | ${ }_{\text {App }}$ Tonal | Int. Total |
| Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 07:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 07:15 AM | 11 | 0 | 8 | 1 | 20 | 11 | 185 | 1 | 0 | 197 | 11 | 1 | 0 | 0 | 12 | 0 | 186 | 3 | 5 | 194 | 423 |
| 07:30 AM | 4 | 0 | 12 | 0 | 16 | 13 | 185 | 2 | 0 | 200 | 17 | 0 | 3 | 0 | 20 | 0 | 150 | 6 | 2 | 158 | 394 |
| 07:45 AM | 5 | 1 | 16 | 0 | 22 | 12 | 197 | 1 | 0 | 210 | 14 | 1 | 1 | 0 | 16 | 0 | 179 | 3 | 3 | 185 | 433 |
| 08:00 AM | 5 | 0 | 11 | 1 | 17 | 10 | 182 | 4 | 0 | 196 | 15 | 2 | 3 | 0 | 20 | 0 | 182 | 4 | 1 | 187 | 420 |
| Total Volume | 25 | 1 | 47 | 2 | 75 | 46 | 749 | 8 | 0 | 803 | 57 | 4 | 7 | 0 | 68 | 0 | 697 | 16 | 11 | 724 | 1670 |
| \% App. Total | 33.3 | 1.3 | 62.7 | 2.7 |  | 5.7 | 93.3 | 1 | 0 |  | 83.8 | 5.9 | 10.3 | 0 |  | 0 | 96.3 | 2.2 | 1.5 |  |  |
| PHF | . 568 | . 250 | . 734 | . 500 | . 852 | . 885 | . 951 | . 500 | . 000 | . 956 | . 838 | . 500 | . 583 | . 000 | . 850 | . 000 | . 937 | . 667 | . 550 | . 933 | . 964 |


|  |  |  |
| :---: | :---: | :---: |
|  | Peak Hour Data <br> Peak Hour Begins at 07:15 AM <br> Lights <br> Trucks <br> Buses |  |
|  |  |  |

## Connecticut Counts LLC <br> Kensington, Connecticut 06037 (860) 828-1693

File Name : 22489
Site Code : 22489
Start Date : 12/15/2021
Page No : 3

|  | Private Drive From North |  |  |  |  | Route 1 <br> From East |  |  |  |  | N. State Street From South |  |  |  |  | Route 1 From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start <br> Time | Right | Thru | Left | Peds | Afp. Toral | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toral | Right | Thru | Left | Peds | App. Teal | Int. Tolal |

Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

|  | 07:15 AM |  |  |  |  | 07:30 AM |  |  |  |  | 07:30 AM |  |  |  |  | 07:15 AM |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| +0 mins. | 11 | 0 | 8 | 1 | 20 | 13 | 185 | 2 | 0 | 200 | 17 | 0 | 3 | 0 | 20 | 0 | 186 | 3 | 5 | 194 |
| +15 mins. | 4 | 0 | 12 | 0 | 16 | 12 | 197 | 1 | 0 | 210 | 14 | 1 | 1 | 0 | 16 | 0 | 150 | 6 | 2 | 158 |
| +30 mins. | 5 | 1 | 16 | 0 | 22 | 10 | 182 | 4 | 0 | 196 | 15 | 2 | 3 | 0 | 20 | 0 | 179 | 3 | 3 | 185 |
| +45 mins. | 5 | 0 | 11 | 1 | 17 | 9 | 212 | 2 | 0 | 223 | 21 | 1 | 2 | 0 | 24 | 0 | 182 | 4 | 1 | 187 |
| Total Volume | 25 | 1 | 47 | 2 | 75 | 44 | 776 | 9 | 0 | 829 | 67 | 4 | 9 | 0 | 80 | 0 | 697 | 16 | 11 | 724 |
| \% App. Total | 33.3 | 1.3 | 62.7 | 2.7 |  | 5.3 | 93.6 | 1.1 | 0 |  | 83.8 | 5 | 11.2 | 0 |  | 0 | 96.3 | 2.2 | 1.5 |  |
| PHF | . 568 | . 250 | . 734 | . 500 | . 852 | . 846 | . 915 | . 563 | . 000 | . 929 | . 798 | . 500 | . 750 | . 000 | . 833 | . 000 | . 937 | . 667 | . 550 | . 933 |



# Connecticut Counts LLC <br> Kensington, Connecticut 06037 <br> (860) 828-1693 

Route 1 at N. State St/Private Dr Stamford, Connecticut

File Name : 22490
Site Code : 22490
Start Date: 12/14/2021
Page No : 1

| Groups Printed- Lights - Trucks - Buses |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | T. Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Private Drive From North |  |  |  |  | Route 1 From East |  |  |  |  | N. State Street From South |  |  |  |  | Route 1 From West |  |  |  |  |  |
| Start Time | Ripht | Thru | Left | Peds | AIf. Tolas | Right | Thru | Left | Peds | Afy Toral | Right | Thru | Left | Peds | AFT. Toul | Right | Thru | Left | Peds | Afle Taral |  |
| 04:00 PM | 5 | 0 | 3 | 4 | 12 | 8 | 149 | 3 | 0 | 160 | 38 | 0 | 0 | 0 | 38 | 1 | 224 | 3 | 1 | 229 | 439 |
| 04:15 PM | 2 | 0 | 4 | 8 | 14 | 1 | 147 | 1 | 0 | 149 | 31 | 0 | 0 | 0 | 31 | 2 | 199 | 2 | 0 | 203 | 397 |
| 04:30 PM | 3 | 0 | 3 | 2 | 8 | 2 | 157 | 1 | 0 | 160 | 32 | 0 | 0 | 0 | 32 | 0 | 233 | 5 | 0 | 238 | 438 |
| 04:45 PM | 2 | 0 | 7 | 3 | 12 | 3 | 173 | 1 | 0 | 177 | 36 | 1 | 1 | 2 | 40 | 0 | 285 | 1 | 2 | 288 | 517 |
| Total | 12 | 0 | 17 | 17 | 46 | 14 | 626 | 6 | 0 | 646 | 137 | 1 | 1 | 2 | 141 | 3 | 941 | 11 | 3 | 958 | 1791 |
| 05:00 PM | 2 | 0 | 1 | 1 | 4 | 3 | 146 | 1 | 0 | 150 | 16 | 2 | 4 | 0 | 22 | 0 | 137 | 3 | 0 | 140 | 316 |
| 05:15 PM | 6 | 0 | 5 | 1 | 12 | 8 | 162 | 0 | 0 | 170 | 28 | 0 | 3 | 0 | 31 | 1 | 215 | 5 | 1 | 222 | 435 |
| 05:30 PM | 9 | 0 | 7 | 3 | 19 | 6 | 158 | 1 | 0 | 165 | 52 | 2 | 1 | 0 | 55 | 2 | 252 | 1 | 2 | 257 | 496 |
| 05:45 PM | 4 | 0 | 2 | 2 | 8 | 4 | 154 | 0 | 0 | 158 | 29 | 1 | 0 | 0 | 30 | 2 | 167 | 3 | 2 | 174 | 370 |
| Total | 21 | 0 | 15 | 7 | 43 | 21 | 620 | 2 | 0 | 643 | 125 | 5 | 8 | 0 | 138 | 5 | 771 | 12 | 5 | 793 | 1617 |
| Grand Total | 33 | 0 | 32 | 24 | 89 | 35 | 1246 | 8 | 0 | 1289 | 262 | 6 | 9 | 2 | 279 | 8 | 1712 | 23 | 8 | 1751 | 3408 |
| Apprch \% | 37.1 | 0 | 36 | 27 |  | 2.7 | 96.7 | 0.6 | 0 |  | 93.9 | 2.2 | 3.2 | 0.7 |  | 0.5 | 97.8 | 1.3 | 0.5 |  |  |
| Total \% | 1 | 0 | 0.9 | 0.7 | 2.6 | 1 | 36.6 | 0.2 | 0 | 37.8 | 7.7 | 0.2 | 0.3 | 0.1 | 8.2 | 0.2 | 50.2 | 0.7 | 0.2 | 51.4 |  |
| Lights | 33 | 0 | 32 | 24 | 89 | 35 | 1229 |  |  |  |  |  |  |  |  |  | 1692 |  |  |  |  |
| \% Lights | 100 | 0 | 100 | 100 | 100 | 100 | 98.6 | 100 | 0 | 98.7 | 100 | 100 | 100 | 100 | 100 | 100 | 98.8 | 100 | 100 | 98.9 | 98.9 |
| Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 4 | 5 |
| \% Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 0.1 | 0 | 0 | 0.1 | 0 | 0 | 0 | 0 | 0 | 0 | 0.2 | 0 | 0 | 0.2 | 0.1 |
| Buses | 0 | 0 | 0 | 0 | 0 | 0 | 16 | 0 | 0 | 16 | 0 | 0 | 0 | 0 | 0 | 0 | 16 | 0 | 0 | 16 | 32 |
| \% Buses | 0 | 0 | 0 | 0 | 0 | 0 | 1.3 | 0 | 0 | 1.2 | 0 | 0 | 0 | 0 | 0 | 0 | 0.9 | 0 | 0 | 0.9 | 0.9 |

## Connecticut Counts LLC <br> Kensington, Connecticut 06037 <br> (860) 828-1693

File Name : 22490
Site Code : 22490
Start Date: 12/14/2021
Page No : 2

|  | Private Drive From North |  |  |  |  | Route 1 From East |  |  |  |  | N. State Street From South |  |  |  |  | Route 1 From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toul | Right | Thru | Left | Peds | App. Taxal | Right | Thru | Left | Peds | App. Toal | Int. Total |
| Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 04:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 04:00 PM | 5 | 0 | 3 | 4 | 12 | 8 | 149 | 3 | 0 | 160 | 38 | 0 | 0 | 0 | 38 | 1 | 224 | 3 | 1 | 229 | 439 |
| 04:15 PM | 2 | 0 | 4 | 8 | 14 | 1 | 147 | 1 | 0 | 149 | 31 | 0 | 0 | 0 | 31 | 2 | 199 | 2 | 0 | 203 | 397 |
| 04:30 PM | 3 | 0 | 3 | 2 | 8 | 2 | 157 | 1 | 0 | 160 | 32 | 0 | 0 | 0 | 32 | 0 | 233 | 5 | 0 | 238 | 438 |
| 04:45 PM | 2 | 0 | 7 | 3 | 12 | 3 | 173 | 1 | 0 | 177 | 36 | 1 | 1 | 2 | 40 | 0 | 285 | 1 | 2 | 288 | 517 |
| Total Volume | 12 | 0 | 17 | 17 | 46 | 14 | 626 | 6 | 0 | 646 | 137 | 1 | 1 | 2 | 141 | 3 | 941 | 11 | 3 | 958 | 1791 |
| \% App. Total | 26.1 | 0 | 37 | 37 |  | 2.2 | 96.9 | 0.9 | 0 |  | 97.2 | 0.7 | 0.7 | 1.4 |  | 0.3 | 98.2 | 1.1 | 0.3 |  |  |
| PHF | . 600 | . 000 | . 607 | . 531 | . 821 | . 438 | . 905 | . 500 | . 000 | . 912 | . 901 | . 250 | . 250 | . 250 | . 881 | . 375 | . 825 | . 550 | . 375 | . 832 | . 866 |


|  |  |  |
| :---: | :---: | :---: |
|  | Peak Hour Data <br> Peak Hour Begins at 04:00 PM <br> Lights <br> Trucks <br> Buses |  |
|  |  |  |

## Connecticut Counts LLC <br> Kensington, Connecticut 06037 (860) 828-1693

File Name : 22490
Site Code : 22490
Start Date : 12/14/2021
Page No : 3

|  | Private Drive From North |  |  |  |  | Route 1 <br> From East |  |  |  |  | N. State Street From South |  |  |  |  | Route 1 From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start <br> Time | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Taral | Right | Thru | Left | Peds | App. Tona | Right | Thru | Left | Peds | App. Total | lnt. Total |

Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

| +0 mins. <br> +15 mins. <br> +30 mins. <br> +45 mins. | 04:45 PM |  |  |  |  | 04:45 PM |  |  |  |  | 04:45 PM |  |  |  |  | 04:00 Pm |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 2 | 0 | 7 | 3 | 12 | 3 | 173 | 1 | 0 | 177 | 36 | 1 | 1 | 2 | 40 | 1 | 224 | 3 | 1 | 229 |
|  | 2 | 0 | 1 | 1 | 4 | 3 | 146 | 1 | 0 | 150 | 16 | 2 | 4 | 0 | 22 | 2 | 199 | 2 | 0 | 203 |
|  | 6 | 0 | 5 | 1 | 12 | 8 | 162 | 0 | 0 | 170 | 28 | 0 | 3 | 0 | 31 | 0 | 233 | 5 | 0 | 238 |
|  | 9 | 0 | 7 | 3 | 19 | 6 | 158 | 1 | 0 | 165 | 52 | 2 | 1 | 0 | 55 | 0 | 285 | 1 | 2 | 288 |
| Total Volume | 19 | 0 | 20 | 8 | 47 | 20 | 639 | 3 | 0 | 662 | 132 | 5 | 9 | 2 | 148 | 3 | 941 | 11 | 3 | 958 |
| \% App. Total | 40.4 | 0 | 42.6 | 17 |  | 3 | 96.5 | 0.5 | 0 |  | 89.2 | 3.4 | 6.1 | 1.4 |  | 0.3 | 98.2 | 1.1 | 0.3 |  |
| PHF | . 528 | . 000 | . 714 | . 667 | . 618 | . 625 | . 923 | . 750 | . 000 | . 935 | . 635 | . 625 | . 563 | . 250 | . 673 | . 375 | . 825 | . 550 | . 375 | . 832 |



## Connecticut Counts LLC

## Kensington, Connecticut 06037 (860) 828-1693

Lafayette St at N. State Street
File Name : 22491
Stamford, Connecticut
Site Code : 22491
Start Date : 12/15/2021
Page No : 1

| Groups Printed-Lights - Trucks - Buses |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Lafayette Street From North |  |  |  |  | N. State Street From East |  |  |  |  | Lafayette Street From South |  |  |  |  | N. State Street From West |  |  |  |  |  |
| Start Time | Right | Thru | Left | Peds | Ans Tomal | Ripht | Thru | Left | Peds | Am. Taid | Right | Thru | Left | Peds | Am. Tonat | Right | Thru | Left | Peds | Art Toal | In. Toual |
| 07:00 AM | 2 | 0 | 0 | 2 | 4 | 3 | 0 | 0 | 0 | 3 | 0 | 1 | 3 | 0 | 4 | 0 | 0 | 2 | 0 | 2 | 13 |
| 07:15 AM | 0 | 0 | 0 | 0 | 0 | 1 | 3 | 0 | 0 | 4 | 0 | 4 | 4 | 0 | 8 | 0 | 0 | 2 | 2 | 4 | 16 |
| 07:30 AM | 0 | 0 | 0 | 1 | 1 | 2 | 2 | 0 | 0 | 4 | 0 | 3 | 7 | 0 | 10 | 0 | 2 | 0 | 1 | 3 | 18 |
| 07:45 AM | 1 | 0 | 0 | 1 | 2 | 0 | 1 | 0 | 0 | 1 | 0 | 6 | 5 | 0 | 11 | 0 | 0 | 0 | 0 | 0 | 14 |
| Total | 3 | 0 | 0 | 4 | 7 | 6 | 6 | 0 | 0 | 12 | 0 | 14 | 19 | 0 | 33 | 0 | 2 | 4 | 3 | 9 | 61 |
| 08:00 AM | 1 | 0 | 0 | 1 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 2 | 1 | 11 | 0 | 0 | 1 | 2 | 3 | 16 |
| 08:15 AM | 1 | 0 | 0 | 4 | 5 | 2 | 2 | 0 | 0 | 4 | 0 | 11 | 2 | 0 | 13 | 0 | 0 | 2 | 0 | 2 | 24 |
| 08:30 AM | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 1 | 0 | 5 | 4 | 0 | 9 | 0 | 0 | 2 | 0 | 2 | 13 |
| 08:45 AM | 3 | 0 | 0 | 1 | 4 | 4 | 1 | 0 | 1 | 6 | 0 | 8 | 6 | 0 | 14 | 0 | 0 | 2 | 0 | 2 | 26 |
| Total | 6 | 0 | 0 | 6 | 12 | 6 | 4 | 0 | 1 | 11 | 0 | 32 | 14 | 1 | 47 | 0 | 0 | 7 | 2 | 9 | 79 |
| Grand Total | 9 | 0 | 0 | 10 | 19 | 12 | 10 | 0 | 1 | 23 | 0 | 46 | 33 | 1 | 80 | 0 | 2 | 11 | 5 | 18 | 140 |
| Apprch \% | 47.4 | 0 | 0 | 52.6 |  | 52.2 | 43.5 | 0 | 4.3 |  | 0 | 57.5 | 41.2 | 1.2 |  | 0 | 11.1 | 61.1 | 27.8 |  |  |
| Total \% | 6.4 | 0 | 0 | 7.1 | 13.6 | 8.6 | 7.1 | 0 | 0.7 | 16.4 | 0 | 32.9 | 23.6 | 0.7 | 57.1 | 0 | 1.4 | 7.9 | 3.6 | 12.9 |  |
| Lights | 6 | 0 | 0 | 10 | 16 | 12 | 8 | 0 | 1 | 21 | 0 | 46 | 32 | 1 | 79 | 0 | 2 | 10 | 5 | 17 | 133 |
| \% Lights | 66.7 | 0 | 0 | 100 | 84.2 | 100 | 80 | 0 | 100 | 91.3 | 0 | 100 | 97 | 100 | 98.8 | 0 | 100 | 90.9 | 100 | 94.4 | 95 |
| Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| \% Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Buses | 3 | 0 | 0 | 0 | 3 | 0 | 2 | 0 | 0 | 2 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 7 |
| \% Buses | 33.3 | 0 | 0 | 0 | 15.8 | 0 | 20 | 0 | 0 | 8.7 | 0 | 0 | 3 | 0 | 1.2 | 0 | 0 | 9.1 | 0 | 5.6 | 5 |

## Connecticut Counts LLC <br> Kensington, Connecticut 06037 <br> (860) 828-1693

File Name : 22491
Site Code : 22491
Start Date : 12/15/2021
Page No : 2

|  | Lafayette Street From North |  |  |  |  | N. State Street From East |  |  |  |  | Lafayette Street From South |  |  |  |  | N. State Street From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | Agp Tolal | Right | Thru | Left | Peds | App. Toral | Right | Thru | Left | Peds | App. Toual | Right | Thru | Left | Peds | App. Total | Int. Total |
| Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 08:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 08:00 AM | 1 | 0 | 0 | 1 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 2 | 1 | 11 | 0 | 0 | 1 | 2 | 3 | 16 |
| 08:15 AM | 1 | 0 | 0 | 4 | 5 | 2 | 2 | 0 | 0 | 4 | 0 | 11 | 2 | 0 | 13 | 0 | 0 | 2 | 0 | 2 | 24 |
| 08:30 AM | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 1 | 0 | 5 | 4 | 0 | 9 | 0 | 0 | 2 | 0 | 2 | 13 |
| 08:45 AM | 3 | 0 | 0 | 1 | 4 | 4 | 1 | 0 | 1 | 6 | 0 | 8 | 6 | 0 | 14 | 0 | 0 | 2 | 0 | 2 | 26 |
| Total Volume | 6 | 0 | 0 | 6 | 12 | 6 | 4 | 0 | 1 | 11 | 0 | 32 | 14 | 1 | 47 | 0 | 0 | 7 | 2 | 9 | 79 |
| \% App. Total | 50 | 0 | 0 | 50 |  | 54.5 | 36.4 | 0 | 9.1 |  | 0 | 68.1 | 29.8 | 2.1 |  | 0 | 0 | 77.8 | 22.2 |  |  |
| PHF | . 500 | . 000 | . 000 | . 375 | . 600 | . 375 | . 500 | . 000 | . 250 | . 458 | . 000 | . 727 | . 583 | . 250 | 839 | . 000 | . 000 | . 875 | . 250 | 750 | 760 |



## Connecticut Counts LLC <br> Kensington, Connecticut 06037 (860) 828-1693

File Name : 22491
Site Code : 22491
Start Date : 12/15/2021
Page No : 3

|  | Lafayette Street From North |  |  |  |  | N. State Street From East |  |  |  |  | Lafayette Street From South |  |  |  |  | N. State Street From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start <br> Time | Right | Thru | Left | Peds | Afp. Toial | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toual | Int. Total |

Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

|  | 08:00 AM |  |  |  |  | 07:00 AM |  |  |  |  | 08:00 AM |  |  |  |  | 07:15 AM |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| +0 mins. | 1 | 0 | 0 | 1 | 2 | 3 | 0 | 0 | 0 | 3 | 0 | 8 | 2 | 1 | 11 | 0 | 0 | 2 | 2 | 4 |
| +15 mins. | 1 | 0 | 0 | 4 | 5 | 1 | 3 | 0 | 0 | 4 | 0 | 11 | 2 | 0 | 13 | 0 | 2 | 0 | 1 | 3 |
| +30 mins. | 1 | 0 | 0 | 0 | 1 | 2 | 2 | 0 | 0 | 4 | 0 | 5 | 4 | 0 | 9 | 0 | 0 | 0 | 0 | 0 |
| +45 mins. | 3 | 0 | 0 | 1 | 4 | 0 | 1 | 0 | 0 | 1 | 0 | 8 | 6 | 0 | 14 | 0 | 0 | 1 | 2 | 3 |
| Total Volume | 6 | 0 | 0 | 6 | 12 | 6 | 6 | 0 | 0 | 12 | 0 | 32 | 14 | 1 | 47 | 0 | 2 | 3 | 5 | 10 |
| \% App. Total | 50 | 0 | 0 | 50 |  | 50 | 50 | 0 | 0 |  | 0 | 68.1 | 29.8 | 2.1 |  | 0 | 20 | 30 | 50 |  |
| PHF | . 500 | . 000 | . 000 | . 375 | . 600 | . 500 | . 500 | . 000 | . 000 | . 750 | . 000 | . 727 | . 583 | . 250 | . 839 | . 000 | 250 | 375 | 625 | . 625 |



## Connecticut Counts LLC <br> Kensington, Connecticut 06037 (860) 828-1693

Lafayette St at N. State Street
File Name : 22492
Site Code : 22492
Start Date : 12/14/2021
Page No : 1

Groups Printed- Lights - Trucks - Buses

|  | Lafayette Street From North |  |  |  |  | N. State Street From East |  |  |  |  | Lafayette Street From South |  |  |  |  | N. State Street From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | AIf Tonal | Rieht | Thru | Left | Peds | As. Toua | Right | Thru | Left | Peds | AT. Tous | Right | Thru | Left | Peds | Afy Toial | Int. Total |
| 04:00 PM | 0 | 0 | 0 | 2 | 2 | 1 | 3 | 0 | 1 | 5 | 0 | 10 | 19 | 0 | 29 | 0 | 0 | 1 | 1 | 2 | 38 |
| 04:15 PM | 2 | 0 | 0 | 0 | 2 | 6 | 4 | 0 | 0 | 10 | 0 | 13 | 13 | 0 | 26 | 0 | 0 | 3 | 0 | 3 | 41 |
| 04:30 PM | 1 | 0 | 0 | 0 | 1 | 1 | 2 | 0 | 0 | 3 | 0 | 10 | 11 | 0 | 21 | 0 | 1 | 2 | 1 | 4 | 29 |
| 04:45 PM | 0 | 0 | 0 | 0 | 0 | 1 | 3 | 0 | 0 | 4 | 1 | 18 | 21 | 0 | 40 | 0 | 0 | 3 | 0 | 3 | 47 |
| Total | 3 | 0 | 0 | 2 | 5 | 9 | 12 | 0 | 1 | 22 | 1 | 51 | 64 | 0 | 116 | 0 | 1 | 9 | 2 | 12 | 155 |
| 05:00 PM | 0 | 0 | 0 | 0 | 0 | 3 | 3 | 0 | 0 | 6 | 0 | 12 | 9 | 0 | 21 | 0 | 0 | 2 | 0 | 2 | 29 |
| 05:15 PM | 0 | 0 | 0 | 0 | 0 | 2 | 2 | 0 | 0 | 4 | 0 | 8 | 7 | 1 | 16 | 0 | 0 | 0 | 1 | 1 | 21 |
| 05:30 PM | 2 | 0 | 0 | 0 | 2 | 3 | 7 | 0 | 0 | 10 | 0 | 23 | 19 | 0 | 42 | 0 | 0 | 1 | 3 | 4 | 58 |
| 05:45 PM | 2 | 0 | 0 | 1 | 3 | 4 | 0 | 0 | 0 | 4 | 0 | 10 | 5 | 0 | 15 | 0 | 0 | 3 | 1 | 4 | 26 |
| Total | 4 | 0 | 0 | 1 | 5 | 12 | 12 | 0 | 0 | 24 | 0 | 53 | 40 | 1 | 94 | 0 | 0 | 6 | 5 | 11 | 134 |
| Grand Total | 7 | 0 | 0 | 3 | 10 | 21 | 24 | 0 | 1 | 46 | 1 | 104 | 104 | 1 | 210 | 0 | 1 | 15 | 7 | 23 | 289 |
| Apprch \% | 70 | 0 | 0 | 30 |  | 45.7 | 52.2 | 0 | 2.2 |  | 0.5 | 49.5 | 49.5 | 0.5 |  | 0 | 4.3 | 65.2 | 30.4 |  |  |
| Total \% | 2.4 | 0 | 0 | 1 | 3.5 | 7.3 | 8.3 | 0 | 0.3 | 15.9 | 0.3 | 36 | 36 | 0.3 | 72.7 | 0 | 0.3 | 5.2 | 2.4 | 8 |  |
| Lights | 7 | 0 | 0 | 3 | 10 | 21 | 24 | 0 | 1 | 46 | 1 | 104 | 103 | 1 | 209 | 0 | 1 | 15 | 7 | 23 | 288 |
| \% Lights | 100 | 0 | 0 | 100 | 100 | 100 | 100 | 0 | 100 | 100 | 100 | 100 | 99 | 100 | 99.5 | 0 | 100 | 100 | 100 | 100 | 99.7 |
| Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 1 |
| \% Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0.5 | 0 | 0 | 0 | 0 | 0 | 0.3 |
| Buses | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| \% Buses | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |

## Connecticut Counts LLC <br> Kensington, Connecticut 06037 <br> (860) 828-1693

File Name : 22492
Site Code : 22492
Start Date : 12/14/2021
Page No : 2

|  | Lafayette Street From North |  |  |  |  | N. State Street From East |  |  |  |  | Lafayette Street From South |  |  |  |  | N. State Street From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start <br> Time | Right | Thru | Left | Peds | ${ }_{\text {App }}$ Toual | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | Appr Totis: | Int. Toral |
| Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 04:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 04:00 PM | 0 | 0 | 0 | 2 | 2 | 1 | 3 | 0 | 1 | 5 | 0 | 10 | 19 | 0 | 29 | 0 | 0 | 1 | 1 | 2 | 38 |
| 04:15 PM | 2 | 0 | 0 | 0 | 2 | 6 | 4 | 0 | 0 | 10 | 0 | 13 | 13 | 0 | 26 | 0 | 0 | 3 | 0 | 3 | 41 |
| 04:30 PM | 1 | 0 | 0 | 0 | 1 | 1 | 2 | 0 | 0 | 3 | 0 | 10 | 11 | 0 | 21 | 0 | 1 | 2 | 1 | 4 | 29 |
| 04:45 PM | 0 | 0 | 0 | 0 | 0 | 1 | 3 | 0 | 0 | 4 | 1 | 18 | 21 | 0 | 40 | 0 | 0 | 3 | 0 | 3 | 47 |
| Total Volume | 3 | 0 | 0 | 2 | 5 | 9 | 12 | 0 | 1 | 22 | 1 | 51 | 64 | 0 | 116 | 0 | 1 | 9 | 2 | 12 | 155 |
| \% App. Tolal | 60 | 0 | 0 | 40 |  | 40.9 | 54.5 | 0 | 4.5 |  | 0.9 | 44 | 55.2 | 0 |  | 0 | 8.3 | 75 | 16.7 |  |  |
| PHF | . 375 | . 000 | . 000 | . 250 | . 625 | . 375 | . 750 | . 000 | . 250 | . 550 | . 250 | . 708 | . 762 | . 000 | . 725 | . 000 | . 250 | . 750 | . 500 | 750 | . 824 |



## Connecticut Counts LLC <br> Kensington, Connecticut 06037 (860) 828-1693

File Name : 22492
Site Code : 22492
Start Date : 12/14/2021
Page No : 3

|  | Lafayette Street From North |  |  |  |  | N. State Street From East |  |  |  |  | Lafayette Street From South |  |  |  |  | N. State Street From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start <br> Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App Tual | Right | Thru | Left | Peds | App. Tmal | Right | Thru | Left | Peds | App. Talal | Inc. Tsual |

Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

|  | 04:00 PM |  |  |  |  | 04:45 PM |  |  |  |  | 04:45 PM |  |  |  |  | 04.00 PM |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| +0 mins. | 0 | 0 | 0 | 2 | 2 | 1 | 3 | 0 | 0 | 4 | 1 | 18 | 21 | 0 | 40 | 0 | 0 | 1 | 1 | 2 |
| +15 mins. | 2 | 0 | 0 | 0 | 2 | 3 | 3 | 0 | 0 | 6 | 0 | 12 | 9 | 0 | 21 | 0 | 0 | 3 | 0 | 3 |
| +30 mins. | 1 | 0 | 0 | 0 | 1 | 2 | 2 | 0 | 0 | 4 | 0 | 8 | 7 | 1 | 16 | 0 | 1 | 2 | 1 | 4 |
| +45 mins. | 0 | 0 | 0 | 0 | 0 | 3 | 7 | 0 | 0 | 10 | 0 | 23 | 19 | 0 | 42 | 0 | 0 | 3 | 0 | 3 |
| Total Volume | 3 | 0 | 0 | 2 | 5 | 9 | 15 | 0 | 0 | 24 | 1 | 61 | 56 | 1 | 119 | 0 | 1 | 9 | 2 | 12 |
| \% App. Total | 60 | 0 | 0 | 40 |  | 37.5 | 62.5 | 0 | 0 |  | 0.8 | 51.3 | 47.1 | 0.8 |  | 0 | 8.3 | 75 | 16.7 |  |
| PHF | . 375 | . 000 | . 000 | . 250 | . 625 | . 750 | . 536 | . 000 | . 000 | . 600 | . 250 | . 663 | . 667 | . 250 | . 708 | . 000 | . 250 | . 750 | . 500 | . 750 |



## Connecticut Counts LLC <br> Kensington, Connecticut 06037 (860) 828-1693

Lafayette Street at S. State Street
Stamford, Connecticut
File Name : 22493
Site Code : 22493
Start Date : 12/15/2021
Page No : 1

|  | Lafayette Street From North |  |  |  |  | S. State Street From East |  |  |  |  | From South |  |  |  |  | S. State Street From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Ripht | Thru | Left | Peds | AT. Total | Right | Thru | Left | Peds | Apr Toral | Ripht | Thru | Left | Peds | Ary. Total | Right | Thru | Left | Peds | Apr. Total | In. Tnal |
| 07:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 5 | 0 | 7 | 7 |
| 07:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 6 | 0 | 12 | 13 |
| 07:30 AM | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 9 | 9 | 0 | 18 | 19 |
| 07:45 AM | 1 | 0 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 17 | 11 | 0 | 28 | 30 |
| Total | 1 | 0 | 0 | 0 | 1 | 2 | 1 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 34 | 31 | 0 | 65 | 69 |
| 08:00 AM | 0 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 20 | 21 | 0 | 41 | 42 |
| 08:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 18 | 9 | 0 | 27 | 28 |
| 08:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 20 | 15 | 0 | 35 | 35 |
| Grand Total | 1 | 0 | 0 | 1 | 2 | 2 | 1 | 0 | 0 | 3 | 0 | 0 | 0 | 1 | 1 | 0 | 92 | 76 | 0 | 168 | 174 |
| Apprch \% | 50 | 0 | 0 | 50 |  | 66.7 | 33.3 | 0 | 0 |  | 0 | 0 | 0 | 100 |  | 0 | 54.8 | 45.2 | 0 |  |  |
| Total \% | 0.6 | 0 | 0 | 0.6 | 1.1 | 1.1 | 0.6 | 0 | 0 | 1.7 | 0 | 0 | 0 | 0.6 | 0.6 | 0 | 52.9 | 43.7 | 0 | 96.6 |  |
| Lights | 1 | 0 | 0 | 1 | 2 | 2 | 1 | 0 | 0 | 3 | 0 | 0 | 0 | 1 | 1 | 0 | 92 | 75 | 0 | 167 | 173 |
| \% Lights | 100 | 0 | 0 | 100 | 100 | 100 | 100 | 0 | 0 | 100 | 0 | 0 | 0 | 100 | 100 | 0 | 100 | 98.7 | 0 | 99.4 | 99.4 |
| Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| \% Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Buses | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 1 |
| \% Buses | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1.3 | 0 | 0.6 | 0.6 |

## Connecticut Counts LLC <br> Kensington, Connecticut 06037 (860) 828-1693

File Name : 22493
Site Code : 22493
Start Date : 12/15/2021
Page No : 2

|  | Lafayette Street From North |  |  |  |  | S. State Street From East |  |  |  |  | From South |  |  |  |  | S. State Street From West |  |  |  |  | Inr. Toral |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start <br> Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App Total | Right | Thru | Left | Peds | ${ }_{\text {App }}$ Total | Right | Thru | Left | Peds | App. Toual |  |
| Peak Hour Analysis From 07:00 AM to 08:30 AM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 07:45 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 07:45 AM | 1 | 0 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 17 | 11 | 0 | 28 | 30 |
| 08:00 AM | 0 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 20 | 21 | 0 | 41 | 42 |
| 08:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 18 | 9 | 0 | 27 | 28 |
| 08:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 20 | 15 | 0 | 35 | 35 |
| Total Volume | 1 | 0 | 0 | 1 | 2 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 1 | 0 | 75 | 56 | 0 | 131 | 135 |
| \% App. Total | 50 | 0 | 0 | 50 |  | 100 | 0 | 0 | 0 |  | 0 | 0 | 0 | 100 |  | 0 | 57.3 | 42.7 | 0 |  |  |
| PHF | . 250 | . 000 | . 000 | . 250 | . 500 | . 250 | . 000 | . 000 | . 000 | . 250 | . 000 | . 000 | . 000 | . 250 | . 250 | . 000 | . 938 | . 667 | . 000 | . 799 | . 804 |



## Connecticut Counts LLC <br> Kensington, Connecticut 06037 (860) 828-1693

File Name : 22493
Site Code : 22493
Start Date : 12/15/2021
Page No : 3

|  | Lafayette Street From North |  |  |  |  | S. State Street From East |  |  |  |  | From South |  |  |  |  | S. State Street From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Tola | Right | Thru | Left | Peds | App. Toral | Right | Thru | Left | Peds | App. Tolal | Right | Thru | Left | Peds | App. Total | In. Toral |

Peak Hour Analysis From 07:00 AM to 08:30 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

|  | 07:15 AM |  |  |  |  | 07:00 AM |  |  |  |  | 07:30 AM |  |  |  |  | 07:45 AM |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| +0 mins. | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 17 | 11 | 0 | 28 |
| +15 mins. | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 20 | 21 | 0 | 41 |
| + 30 mins. | 1 | 0 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 18 | 9 | 0 | 27 |
| +45 mins. | 0 | 0 | 0 | 1 | 1 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 1 | 0 | 20 | 15 | 0 | 35 |
| Total Volume | 1 | 0 | 0 | 1 | 2 | 2 | 1 | 0 | 0 | 3 | 0 | 0 | 0 | 1 | 1 | 0 | 75 | 56 | 0 | 131 |
| \% App. Total | 50 | 0 | 0 | 50 |  | 66.7 | 33.3 | 0 | 0 |  | 0 | 0 | 0 | 100 |  | 0 | 57.3 | 42.7 | 0 |  |
| PHF | . 250 | . 000 | . 000 | . 250 | . 500 | . 500 | . 250 | . 000 | . 000 | . 750 | . 000 | . 000 | . 000 | . 250 | . 250 | . 000 | . 938 | . 667 | . 000 | . 799 |



## Connecticut Counts LLC <br> Kensington, Connecticut 06037 (860) 828-1693

Lafayette Street at S. State Street Stamford, Connecticut

File Name : 22494
Site Code : 22494
Start Date: 12/14/2021
Page No : 1

| Groups Printed-Lights - Trucks - Buses |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Lafayette Street From North |  |  |  |  | S. State Street From East |  |  |  |  | From South |  |  |  |  | S. State Street From West |  |  |  |  |  |
| Start Time | Ripht | Thru | Left | Peds | App. Total | Ripht | Thru | Left | Peds | Aprs Totad | Ripht | Thru | Left | Peds | Apt. Toal | Ripht | Thru | Left | Peds | At, Total | In. Total |
| 04:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 27 | 32 | 0 | 59 | 59 |
| 04:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 29 | 21 | 0 | 50 | 50 |
| 04:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 30 | 22 | 0 | 52 | 52 |
| 04:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 47 | 32 | 0 | 79 | 79 |
| Total | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 133 | 107 | 0 | 240 | 240 |
| 05:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 28 | 24 | 0 | 52 | 52 |
| 05:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 43 | 20 | 0 | 63 | 63 |
| 05:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 48 | 42 | 0 | 90 | 90 |
| 05:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 35 | 14 | 0 | 49 | 49 |
| Total | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 154 | 100 | 0 | 254 | 254 |
| Grand Total | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 287 | 207 | 0 | 494 | 494 |
| Apprch \% | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 | 58.1 | 41.9 | 0 |  |  |
| Total \% | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 58.1 | 41.9 | 0 | 100 |  |
| Lights | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 287 | 207 | 0 | 494 | 494 |
| \% Lights | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 100 | 100 | 0 | 100 | 100 |
| Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| \% Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Buses | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| \% Buses | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |

## Connecticut Counts LLC <br> Kensington, Connecticut 06037 (860) 828-1693

File Name : 22494
Site Code : 22494
Start Date: 12/14/2021
Page No : 2

|  | Lafayette Street From North |  |  |  |  | S. State Street From East |  |  |  |  | From South |  |  |  |  | S. State Street From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start <br> Time | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Traal | Right | Thru | Left | Peds | App. Toul | Int. Total |
| Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 04:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 04:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 47 | 32 | 0 | 79 | 79 |
| 05:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 28 | 24 | 0 | 52 | 52 |
| 05:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 43 | 20 | 0 | 63 | 63 |
| 05:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 48 | 42 | 0 | 90 | 90 |
| Total Volume | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 166 | 118 | 0 | 284 | 284 |
| \% App. Total | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 | 58.5 | 41.5 | 0 |  |  |
| PHF | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 865 | . 702 | . 000 | . 789 | . 789 |



## Connecticut Counts LLC <br> Kensington, Connecticut 06037 <br> (860) 828-1693

File Name : 22494
Site Code : 22494
Start Date : 12/14/2021
Page No : 3

|  | Lafayette Street From North |  |  |  |  | S. State Street From East |  |  |  |  | From South |  |  |  |  | S. State Street From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start <br> Time | Right | Thru | Left | Peds | Agp. Toma | Right | Thru | Left | Peds | App. Toral | Right | Thru | Left | Peds | App. Tolat | Right | Thru | Left | Peds | App. Tolat | Int. Total |

Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

| +0 mins . <br> +15 mins. <br> +30 mins. <br> +45 mins. | 04:00. PM |  |  |  |  | 04:00 PM |  |  |  |  | 04:009 PM |  |  |  |  | 04:45 PM |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 47 | 32 | 0 | 79 |
|  | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 28 | 24 | 0 | 52 |
|  | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 43 | 20 | 0 | 63 |
|  | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 48 | 42 | 0 | 90 |
| Total Volume | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 166 | 118 | 0 | 284 |
| \% App. Total | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 | 58.5 | 41.5 | 0 |  |
| PHF | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 865 | . 702 | . 000 | . 789 |

## ACCIDENT HISTORY

| Crash Severity | Top toreuma | Tima andonts of Grashes | dingh conathons | Roatwoy Featurest | Rozdway Features 2 | Cantribu: ng Fasto. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |

Queries Selected: Town: Stamford, Date (Year:All or 10/1/2018 to 9/30/2021), Severity; All, Route Class: US Route, Road Number: 1, Local Road Name: All, Mile Markers: 7.85 to 7.85

These figures display crash-level data only and provide the totals for crashes involving an injury of that type.


Injuries per Crash


Injuries per Crash
0
1
2
Grand Total

$\%$ of All Crashes
Crashes
20.00
$4.00-14.81 \%$
$3.00 \quad 11.11 \%$
27.00

Fatailites per Crash


## Collision Analysis Safety Tables



Time and Date of Crashes
brasil


Reandway
Featuras
Featumes

Roatonaly
Features 2
contrimu: inflactos

Queries Selected: Town: Stamford, Date (Year:All or 10/1/2018 to 9/30/2021), Severity: All, Route Class: US Route, Road Number: 1, Local Road Name: All, Mile Markers: 7.85 to 7.85


## Collision Analysis Safety Tables



Queries Selected: Town: Stamford, Date (Year:All or 10/1/2018 to 9/30/2021), Severity: All, Route Class: US Route, Road Number: 1, Local Road Name: All, Mile Markers: 7.85 to 7.85

## Traffic Surface Conditions



Weather Conditions


## Light Conditions



Collision Analysis Safety Tables


Queries Selected: Town: Starfford, Date (Year:All or 10/1/2018 to 9/30/2021), Severity: All, Route Class: US Route, Road Number: 1, Local Road Name: All, Mile Markers: 7.85 to 7.85

## Location of First Harmful Event



## Collision Analysis Safety Tables



Queries Selected: Town: Stamford, Date (Year:All or 10/1/2018 to 9/30/2021), Severity: All, Route Class: US Route, Road Number: 1, Local Road Name: All, Mile Markers: 7.85 to 7.85


## Collision Analysis Safety Tables



Queries Selected: Town: Stamford, Date (Year:All or 10/1/2018 to 9/30/2021), Severity: All, Route Class: US Route, Road Number: 1, Local Road Name: All, Mile Markers: 7.86 to 7.92

These figures display crash-level data only and provide the totals for crashes involving an injury of that type.


Injuries per Crash


## Collision Analysis Safety Tables



## Time and Date of Crashes

erati
0.endition

Rondway
Fondures 1
Rentrway
Fworuras
Contribent

Queries Selected: Town: Stemford, Date (Year:All or 10/1/2018 to 9/30/2021), Severity: All, Route Class: US Route, Road Number: 1, Local Road Name: All, Mile Markers: 7.86 to 7.92

## Month and Date of Crashes



## Collision Analysis Safety Tables



Queries Selected: Town: Stamford, Date (Year:All or 10/1/2018 to 9/30/2021), Severity: All, Route Class: US Route, Road Number: 1, Local Road Name: All, Mile Markers: 7.86 to 7.92

## Traffic Surface Conditions



Weather Conditions


## Light Conditions



These data are exempi from discovery or admission under 23 U.S.C 409. Data Extracted 10/01/2021

## Collision Analysis Safety Tables




## Location of First Harmful Event

## Manner of Crashes




| Location Of First Harmful Event | Crashes | \% of All Crashes |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Manner Of Crash | Crashes | \% of All Crashes |
| On Roadway | 32.00 | 86.49\% | Sideswipe, Same Direction | 14.00 | 37.84\% |
| In Parking Lane / Zone | 3.00 | 8.11\% | Angle | 14.00 | 37.84\% |
| Shoulder | 1.00 | 2.70\% | Rear End | 4.00 | 10.81\% |
|  |  |  | Sideswipe, Different Direction | 3.00 | 8.14\% |
| Other | 1.00 | 2.70\% |  |  |  |
|  |  |  | Single Vehicle Crash | 2,00 | 5.41\% |
| Grand Total | 37.00 | 100.00\% | Grand Total | 37.00 | 100.00\% |

These data are exempt from discovery or admission under 23 U.S.C 409. Data Extracted 10/01/2021

## Collision Analysis Safety Tables



Queries Selected: Town: Stamford, Date (Year:All or 10/1/2018 to 9/30/2021), Severily: All, Route Class: US Route, Road Number: 1, Local Road Name: All, Mile Markers: 7.86 to 7.92


## Collision Analysis Safety Tables



Queries Selected: Town: Stamford, Date (Year:All or 10/1/2018 to 9/30/2021), Severity: All, Route Class: US Route, Road Number: 1, Local Road Name: All, Mile Markers: 7.93 to 7.93

These figures display crash-level data only and provide the totals for crashes involving an injury of that type.


Injuries per Crash


| Injuries per Crash | Crashes |
| :--- | ---: |
| 0 | 9.00 |
| 1 | 3.00 |
| 2 | 1.00 |
| 3 | 1.00 |
| 4 | 1.00 |
| Grand Total | 15.00 |

## Fatalities per Crash

|  |  |  |  | 15 |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |
| 1 | 1 | 1 | 1 |  |  |
| 2 | 3 | 4 |  | 0 |  |
| Crashes |  | \% of All Crashes | Fatalities per Crash | Crashes | \% of All Crashes |
| 9.00 |  | 60.00\% |  |  |  |
| 3.00 |  | 20.00\% | 0 | 15.00 | 100.00\% |
| 1.00 |  | 6.67\% |  |  |  |
| 1.00 |  | 6.67\% |  |  |  |
| 1.00 |  | 6.67\% |  |  |  |
| 15.00 |  | 100.00\% | Grand Total | 15.00 | 100.00\% |



Queries Selected: Town: Stamford, Date (Year:All or 10/1/2018 to 9/30/2021), Severity: All, Route Class: US Route, Road Number: 1, Local Road Name: All, Mile Markers; 7.93 to 7.93


## Collision Analysis Safety Tables



Queries Selected: Town: Stamford, Date (Year;All or 10/1/2018 to 9/30/2021). Severity: All, Route Class: US Route, Road Number: 1, Local Road Name: All, Mile Markers: 7.93 to 7.93

## Traffic Surface Condítions



## Weather Conditions

|  | 2018 |  | 2019 |  | 2020 |  | 2021 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Crashes | $\%$ of All Crashes | Crashes | \% of All Crashes | Crashes | \% of All Crashes | Crashes | \% of All Crashes |
| Clear | 3.000 | 100.0\% | 4.000 | 80.0\% | 3.000 | 60.0\% | 2.000 | 100.0\% |



| Rain |  | 1.000 | $20.0 \%$ | 2.000 | $40.0 \%$ |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| Grand Total | 3.000 | $100.0 \%$ | 5.000 | $100.0 \%$ | 5.000 | $100.0 \%$ | 2.000 | $100.0 \%$ |



Light Conditions


## Collision Analysis Safety Tables



Queries Selected: Town: Stamford, Date (Year:All or 10/1/2018 to 9/30/2021), Severity: All, Route Class: US Route, Road Number: 1, Local Road Name: All, Mile Markers: 7.93 to 7.93

## Location of First Harmful Event

Manner of Crashes


| Crashes | \% of All Crashes |
| ---: | ---: |
| 6.00 | $40.00 \%$ |
| 5.00 | $33.33 \%$ |
| 1.00 | $6.67 \%$ |
| 1.00 | $6.67 \%$ |
| 1.00 | $6.67 \%$ |
| 1.00 | $6.67 \%$ |
| 15.00 | $100.00 \%$ |

These data are exempl from discovery or admission under 23 U.S.C 409. Data Extracted 10/01/2021

## Collision Analysis Safety Tables



## Erecthon St thes

Driver Actions


Rodnestrans
Motarcycta Cratitues

Queries Selected: Town: Stamford, Date (Year:All or 10/1/2018 to 9/30/2021), Severity: All, Route Class: US Route, Road Number; 1, Local Road Name: All, Mile Markers: 7.93 to 7.93


These figures display crash-level data only and provide the totals for crashes involving an injury of that type.


Injuries per Crash


Injuries per Crash
0
1
2
Grand Total

## 

Fatalities per Crash


Crashes
9.00
11.00



Queries Selected: Town: Stamford, Date (Year:All or 10/1/2018 to 9/30/2021), Severity: All, Route Class: US Route, Road Number: 1 , Local Road Name: All, Mile Markers: 7.94 to 7.97


## Collision Analysis Safety Tables



Queries Selected: Town: Stamford, Date (Year:All or 10/1/2018 to 9/30/2021), Severity: All, Route Class: US Route, Road Number: 1, Local Road Name: All, Mile Markers: 7.94 to 7.97

## Traffic Surface Conditions

|  | 10 | 2018 |  | 2019 |  | 2020 |  | 2021 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Dry |  | Crashes | \% of All Grashes | Crashes | \% of All Crashes | Crashes | \% of All Crashes | Crashes | $\%$ of All <br> Crashes |
|  | Dry | 1.000 | 100.0\% | 4.000 | 100.0\% | 3.000 | 100.0\% | 2.000 | 66.7\% |
| Wet | Wet |  |  |  |  |  |  | 1.000 | 33.3\% |


| 1 | 1.5 | 2 | 3 | 4 | 5 | 7 | 10 | Grand Total | 8.000 | 100.0\% | 4.000 | 100.0\% | 3.000 | 100.0\% | 3.000 | 100.0\% |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Number of Crashes (Logarithmic Scale) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## Weather Conditions



## Light Conditions



## Collision Analysis Safety Tables



Queries Selected: Town: Stamford, Date (Year:All or 10/1/2018 to 9/30/2021), Severity: All, Route Class: US Route, Road Number: 1, Local Road Name: All, Milie Markers: 7.94 to 7.97

## Location of First Harmful Event



| Location Of First Harmful Event | Crashes | \% of All Crashes | Manner Of Crash |
| :---: | :---: | :---: | :---: |
| On Roadway | 9.00 | 81.82\% | Rear End |
|  |  |  | Sideswipe, Same Direction |
| In Parking Lane / Zone | 2.00 | 18.18\% | Angle |
|  |  |  | Sideswipe, Different Direction |
| Grand Total | 11.00 | 100.00\% |  |
|  |  |  | Grand Total |

Manner of Crashes


2
Crashes
\% of All Crashes45.45\%
$100.00 \%$

## Collision Analysis Safety Tables

| grathatiters | 4. then g ploymem | afenten stars ต่ को | Driver Actions | Blyen Distration | Pudtatinis | Motorayelo Crashms |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |

Queries Selected: Town: Stamford, Date (Year:All or 10/1/2018 to 9/30/2021), Severity: All, Route Class: US Route, Road Number: 1, Local Road Name: All, Mile Markers: 7.94 to 7.97


## Collision Analysis Safety Tables



Queries Selected: Town: Stamford, Date (Year:All or 10/1/2018 to 9/30/2021), Severity: All, Route Class: US Route, Road Number: 1, Local Road Name: All, Mile Markers: 7.98 to 7.98

These figures display crash-level data only and provide the totals for crashes involving an injury of that type.


Injuries per Crash


Injuries per Crash
0
1
2.000

Grand Total

## Fatalities per Crash



Time and Date of Crashes


Rosoway
Fommes?

Queries Selected: Town: Stamford, Date (Year:All or 10/1/2018 to 9/30/2021), Severity: All, Route Class: US Route, Road Number: 1, Local Road Name: All, Mile Markers: 7.98 to 7.98


## Collision Analysis Safety Tables



Crash
Conditions

Rotrway Fenturat

Racinay feritareaz

Queries Selected: Town: Stamford, Date (Year:All or 10/1/2018 to $9 / 30 / 2021$ ), Severity: All, Route Class: US Route, Road Number: 1, Local Road Name: All, Mile Markers: 7.98 to 7.98

Traffic Surface Conditions


Weather Conditions


## Collision Analysis Safety Tables



Queries Selected: Town: Stamford, Date (Year:All or 10/1/2018 to $9 / 30 / 2021$ ), Severity: All, Route Class: US Route, Raad Number: 1, Local Road Name: Ali, Mile Markers: 7.98 to 7.98

| Location of First Harmful Event |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- |

These data are exempt from discovery or admission under 23 U.S.C 409. Data Extracted 10/01/2021

## Collision Analysis Safety Tables

| senmeltuse | A Man <br> D $\quad$ aymon | Elction Statis findintures | Driver Actions | Bitger Bratruction | Podostrimy | Motoreyd: Crasifes |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |

Queries Selected: Town: Stamford, Date (Year:All or 10/1/2018 to 9/30/2021), Severity: All, Route Class: US Route, Road Number: 1. Local Road Name: All, Mile Markers: 7.98 to 7.98


Followed Too Closely

| Improper Turn | $2.00 \quad 10.53 \%$ |
| :--- | :--- | :--- |


Speed Related


CAPACITY ANALYSIS PROCEDURES

## CAPACITY ANALYSIS PROCEDURES

Intersections - Four methods of analysis are needed to evaluate different kinds of intersections. These methods are based on procedures found in the Sixth Edition of the Highway Capacity Manual 2016 and are described below.

## Signalized Intersections

This chapter's methodology applies to three-leg and four-leg intersections of two streets or highways where the signalization operates in isolation from nearby intersections.

Performance Measure - An intersection's performance is described by the use of one or more quantitative measures that characterize some aspect of the service provided to a specific road user group. Performance measures include automobile volume-to-capacity ratio, automobile delay, queue storage ratio, pedestrian delay, pedestrian circulation area, pedestrian perception score, bicycle delay, and bicycle perception score. LOS is considered a performance measure. It is computed for the automobile, pedestrian, and bicycle travel modes.

Travel Modes - There are three methodologies that can be used to evaluate intersection performance from the perspective of motorists, pedestrians, and bicyclists. They are referred to as the automobile methodology, the pedestrian methodology, and the bicycle methodology.

Lane Groups and Movement Groups - A separate lane group is established to (a) each lane (or combination of adjacent lanes) that exclusively serves one movement and (b) each lane shared by two or more movements. The concept of movement groups is also established to facilitate data entry. A separate movement group is established for (a) each turn movement with one or more exclusive turn lanes and (b) the through movement (inclusive of any turn movements that share a lane).

LOS Criteria - LOS criteria for the automobile mode are different from those for the nonautomobile modes. The automobile-mode criteria are based on performance measures that are field measurable and perceivable by travelers. The criteria for the non-automobile modes are based on scores reported by travelers indicating their perception of service quality.

Automobile Mode - LOS for Automobile Mode can be characterized for the entire intersection, each intersection approach, and each lane group. Control delay alone is used to characterize LOS for entire intersection or an approach. Control delay and volume-to-capacity ratio are used to characterize LOS for a lane group. Delay quantifies the increase in travel time due to traffic signal control. It is also a surrogate measure of driver discomfort and fuel consumption. The volume-to-capacity ratio quantifies the degree to which a phase's capacity is utilized by a lane group. The following describes each LOS.

Level of Service A - It describes operations with a control delay of 10.0 seconds per vehicle or less and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned
when the volume-to-capacity ratio is low and either progression is exceptionally favorable or the cycle length is very short. If it is due to favorable progression, most vehicles arrive during the green indication and travel through the intersection without stopping.

Level of Service B - It describes operations with control delay between 10 to 20 seconds per vehicle and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is low and either progression is highly favorable or the cycle length is short. More vehicle stop than with LOS A.

Level of Service C - It describes operations with control delay between 20 to 35 seconds per vehicle and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when progression is favorable or the cycle length is moderate. Individual cycle failures (i.e., one or more queued vehicles are not able to depart as a result of insufficient capacity during the cycle) may begin to appear at this level. The number of vehicles stopping is significant, although many vehicles still pass through the intersection without stopping.

Level of Service D - It describes operations with control delay between 35 to 55 seconds per vehicle and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is high and either progression is ineffective or the cycle length is long. Many vehicles stop and individual cycle failures are noticeable.

Level of Service E-It describes operations with control delay between 55 to 80 seconds per vehicle and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is high, progression is unfavorable, and the cycle length is long. Individual cycle failures are frequent.

Level of Service F - It describes operations with control delay between 55 to 80 seconds per vehicle and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is high, progression is unfavorable, and the cycle length is long. Individual cycle failures are frequent.

The LOS thresholds established for automobile mode at a signalized intersection

| CONTROL DELAY <br> (SECONDS PER VEHICLE) | LOS BY VOLUME-TO-CAPACITY <br> RATIO |  |
| :---: | :---: | :---: |
| 1.0 | A | F |
| $\leq 10$ | B | F |
| $>10$ to 20 | C | F |
| $>20$ to 35 | D | F |
| $>35$ to 55 | E | F |
| $>55$ to 80 | F | F |
| 80 |  |  |

Note: For approach-based and intersection-wide assessments, LOS is defined by control delay.

## Two-Way STOP-Controlled Intersections (TWSC)

One typical configuration is a four-leg intersection, where the major street is uncontrolled, while the minor street is controlled by STOP signs. The other typical configuration is a three-leg intersection, where the single minor-street approach is controlled by a STOP sign.

Theoretical Basic - Gap-acceptance models begin with the recognition that TWSC Intersections give no positive indication or control to the driver on the minor street as to when it is appropriate to leave the stop line and enter the major street. The driver must determine when a gap on the major street is large enough to permit entry and when to enter, on the basis of the relative priority of the competing movements. This decision-making process has been formalized analytically into what is commonly known as gap-acceptance theory. Gap-acceptance theory includes three basic elements: the size and distribution (availability) of gaps on the major street, the usefulness of these gaps to the minor-street drivers, and the relative priority of the various movements at the intersection.

Critical Headway and Follow-Up Headway - The critical headway is defined as the minimum interval in the major street traffic stream that allows intersection entry for one minor-street vehicle. Thus, the driver's critical headway is the minimum headway that would be acceptable. Critical headway can be estimated on the basis of observations of the largest rejected and smallest accepted headway for a given intersection. The follow-up headway is defined as the time between the departure of one vehicle from the minor street and the departure of the next vehicle using the same major-street headway, under a condition of continuous queuing on the minor street.

Base Critical Headways for TWSC Intersections

| VEHICLE <br> MOVEMENT | BASE CRITICAL HEADWAY |  |  |
| :---: | :---: | :---: | :---: |
|  | Two Lanes | Four Lanes | Six Lanes |
| Left turn from major | 4.1 | 4.1 | 5.3 |
| U-turn from major | N/A | 6.4 (wide) <br> 6.9 (narrow) | 5.6 |
| Right turn from minor | 6.2 | 6.9 | 7.1 |
| Through traffic on major | 1-stage: 6.5 2-stage, stage l: 5.5 2-stage, Stage II: 5.5 | 1-stage:6.5 2-stage, stage l: 5.5 2-stage, Stage II: 5.5 | 1-stage: $6.5^{*}$ <br> 2-stage, stage I: $5.5^{*}$ <br> 2-stage, Stage II: $5.5^{*}$ |
| Left turn from minor | 1-stage:7.1 2-stage, stage l: 6.1 2-stage, Stage II: 6.1 | 1-stage:7.5 2-stage, stage I: 6.5 2-stage, Stage II: 6.5 | 1-stage:6.4 <br> 2-stage, stage I: 7.3 <br> 2-stage, Stage II: 6.7 |

*Use caution; values estimated
Base Follow-up Headways for TWSC Intersections

| VEHICLE MOVEMENT | BASE FOLLOW-UP HEADWAY |  |  |
| :---: | :---: | :---: | :---: |
|  | Two Lanes | Four Lanes | Six Lanes |
| Left turn from major | 2.2 | 2.2 | 3.1 |
| U-turn from major | N/A | $\begin{gathered} \hline 2.5 \text { (wide) } \\ 3.1 \text { (narrow) } \end{gathered}$ | 2.3 |
| Right turn from minor | 3.3 | 3.3 | 3.9 |
| Through traffic on major | 4.0 | 4.0 | 4.0 |
| Left turn from minor | 3.5 | 3.5 | 3.8 |

Level Of Service Criteria - LOS for a TWSC intersection is determined by the computed or measured control delay. For motor vehicles, LOS is determined for each minor-street movement (or shared movement) as well as major-street left turn. LOS is not defined for the intersection as a whole or for major-street approaches. LOS F is assigned to the movement if the volume-to-capacity ratio for the movement exceeds 1.0 , regardless of the control delay.

Automobile Mode - The methodology applies to TWSC intersections with up to three lanes (either shared or exclusive) on the major-street approaches and up to three lanes on the minor-street
approaches (with no more than one exclusive lane for each movement on the minor-street approach). Effects from other intersections are accounted for only in situations in which a TWSC intersection is located on an urban street segment between coordinated signalized intersections. In this situation, the intersection can be analyzed by using the procedures in urban street segment.

Level-of Service Criteria for Automobile Mode

| CONTROL DELAY (SECONDS PER VEHICLE) | LOS BY VOLUME-TO-CAPACITY RATIO |  |
| :---: | :---: | :---: |
|  | 1.0 | $>1.0$ |
| 0-10 | A | F |
| $>10$ to 15 | B | F |
| $>15$ to 25 | C | F |
| >25 to 35 | D | F |
| >35 to 50 | E | F |
| $>50$ | F | F |

Note: The LOS criteria apply to each lane on a given approach and to each approach on the minor street. LOS is not calculated for major-street approaches or for the intersection as a whole.

## All-Way STOP-Controlled Intersections

AWSC intersections require every vehicle to stop at the intersection before proceeding. Because each driver must stop, the decision to proceed into the intersection is a function of traffic conditions on the other approaches. If no traffic is present on the other approaches, a driver can proceed immediately after stopping. If there is traffic on one or more of the other approaches, a driver proceeds only after determining that no vehicles are currently in the intersection and that it is the driver's turn to proceed.

Level Of Service Criteria - For the assessment of LOS at the approach and intersection levels, LOS is based solely on control delay. LOS F is assigned if volume-to-capacity ratio of a lane exceeds 1.0 , regardless of the control delay.

Level-of Service Criteria for Automobile Mode

|  | LOS BY VOLUME-TO-CAPACITY RATIO* |  |
| :---: | :---: | :---: |
| CONTROL DELAY (SECONDS PER VEHICLE) | $v / c \leq 1.0$ | $\mathrm{v} / \mathrm{C}>1.0$ |
| 0-10 | A | 0-10 |
| $>10$ to 15 | B | $>10$ to 15 |
| $>15$ to 25 | C | $>15$ to 25 |
| >25 to 35 | D | $>25$ to 35 |
| >35 to 50 | E | > 35 to 50 |
| $>50$ | F | $>50$ |

Note: * For approaches and intersection wide assessment, LOS is defined solely by control delay.

Automobile Mode - Methodologies in this chapter apply to isolated AWSC intersection with up to three lanes on each approach. They do not account for intersection effects with other intersections. The methodologies do not apply to AWSC intersections with more than four approaches. In addition, the effect of conflicting pedestrians on automobiles is not considered in this procedure.

Pedestrian and Bicycle Modes - The current methodologies for analyzing LOS and delay at AWSC intersections do not extend to pedestrians and bicycles.

Roundabouts - Roundabouts are intersections with a generally circular shape, characterized by yield on entry and circulation around a central island. The analysis boundaries are the roundabout itself, including associated pedestrian crosswalks. The methodology does not account for the effects of adjacent traffic control devices such as nearby traffic signals or signalized pedestrian crossing.

Level of Service Criteria - Assessment of LOS for automobiles in roundabouts at the approach and intersection levels is based solely on control delay. LOS F is assigned if the volume-to-capacity ratio of a lane exceeds 1.0 regardless of the control delay.

Imitation of the Methodology - The methodology applies to isolated roundabouts with up to two entry lanes and up to one bypass lane per approach.

Level-of Service Criteria for Automobile Mode

| CONTROL DELAY <br> (SECONDS PER VEHICLE) | LOS BY VOLUME-TO-CAPACITY RATIO* |  |
| :---: | :---: | :---: | :---: |
|  | $\mathrm{~V} / \mathrm{c}>1.0$ |  |
| $0-10$ | A | $0-10$ |
| $>10$ to 15 | B | $>10$ to 15 |
| $>15$ to 25 | C | $\gg 15$ to 25 |
| $>25$ to 35 | D | $>25$ to 35 |
| $>35$ to 50 | E | $>35$ to 50 |
| $>50$ | F | $>50$ |

Note: For approaches and intersection wide assessment, LOS is defined solely by control delay.

## CAPACITY ANALYSIS WORKSHEETS

## CAPACITY ANALYSIS WORKSHEETS

## Existing Conditions

|  | ¢ | + |  | r |  | 4 | 4 | 4 |  |  | 1 | $\pm$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | 蚛 |  |  | 44 |  |  | 4 |  | \% | F |  |
| Traffic Volume (vph) | 0 | 450 | 1 | 3 | 805 | 0 | 18 | 0 | 19 | 222 | 3 | 51 |
| Future Volume (vph) | 0 | 450 | 1 | 3 | 805 | 0 | 18 | 0 | 19 | 222 | 3 | 51 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Width (ft) | 10 | 10 | 10 | 10 | 10 | 10 | 15 | 15 | 15 | 12 | 11 | 11 |
| Grade (\%) |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Storage Length (ft) | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 200 |
| Starage Lanes | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 1 |  | 1 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Ped Bike Factor |  | 1.00 |  |  | 1.00 |  |  | 0.98 |  | 0.99 | 0.98 |  |
| Frt |  |  |  |  |  |  |  | 0.931 |  |  | 0.857 |  |
| Flt Protected |  |  |  |  |  |  |  | 0.976 |  | 0.950 |  |  |
| Satd. Flow (prot) | 0 | 3303 | 0 | 0 | 3303 | 0 | 0 | 1837 | 0 | 1770 | 1510 | 0 |
| Flt Permitted |  |  |  |  | 0.954 |  |  | 0.874 |  | 0.730 |  |  |
| Satd. Flow (perm) | 0 | 3303 | 0 | 0 | 3151 | 0 | 0 | 1637 | 0 | 1352 | 1510 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  |  |  |  |  |  |  | 28 |  |  | 57 |  |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 30 |  |  | 30 |  |
| Link Distance ( ft ) |  | 333 |  |  | 411 |  |  | 360 |  |  | 606 |  |
| Travel Time (s) |  | 7.6 |  |  | 9.3 |  |  | 8.2 |  |  | 13.8 |  |
| Confl. Peds. (\#/hr) | 10 |  | 4 | 4 |  | 10 | 6 |  | 2 | 2 |  | 6 |
| Confl. Bikes (\#/hr) |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour Factor | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 |
| Growth Factor | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% |
| Heavy Vehicles (\%) | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% |
| Bus Blockages (\#/hr) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Parking (\#/hr) |  |  |  |  |  |  |  |  |  |  |  |  |
| Mid-Block Traffic (\%) |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Adj. Flow (vph) | 0 | 506 | 1 | 3 | 904 | 0 | 20 | 0 | 21 | 249 | 3 | 57 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 507 | 0 | 0 | 907 | 0 | 0 | 41 | 0 | 249 | 60 | 0 |
| Turn Type |  | NA |  | Perm | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases |  | 2 |  |  | 2 |  |  | 4 |  |  | 4 |  |
| Permitted Phases |  |  |  | 2 |  |  | 4 |  |  | 4 |  |  |
| Detector Phase |  | 2 |  | 2 | 2 |  | 4 | 4 |  | 4 | 4 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial ( s ) |  | 15.0 |  | 15.0 | 15.0 |  | 7.0 | 7.0 |  | 7.0 | 7.0 |  |
| Minimum Split (s) |  | 21.1 |  | 21.1 | 21.1 |  | 12.1 | 12.1 |  | 12.1 | 12.1 |  |
| Total Split (s) |  | 82.0 |  | 82.0 | 82.0 |  | 38.0 | 38.0 |  | 38.0 | 38.0 |  |
| Total Split (\%) |  | 68.3\% |  | 68.3\% | 68.3\% |  | 31.7\% | 31.7\% |  | 31.7\% | 31.7\% |  |
| Yellow Time (s) |  | 4.1 |  | 4.1 | 4.1 |  | 3.6 | 3.6 |  | 3.6 | 3.6 |  |
| All-Red Time (s) |  | 2.0 |  | 2.0 | 2.0 |  | 1.5 | 1.5 |  | 1.5 | 1.5 |  |
| Lost Time Adjust (s) |  | 0.0 |  |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time (s) |  | 6.1 |  |  | 6.1 |  |  | 5.1 |  | 5.1 | 5.1 |  |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode |  | C-Min |  | C-Min | C-Min |  | None | None |  | None | None |  |
| Act Effct Green (s) |  | 82.5 |  |  | 82.5 |  |  | 26.3 |  | 26.3 | 26.3 |  |



|  | $\rightarrow$ | - | 7 |  | 4 | 7 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | 性 |  |  | 44 | M |  |
| Traffic Volume (vph) | 713 | 0 | 8 | 795 | 11 | 57 |
| Future Volume (vph) | 713 | 0 | 8 | 795 | 11 | 57 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Width (ft) | 11 | 11 | 11 | 11 | 14 | 14 |
| Grade (\%) | 0\% |  |  | 0\% | 0\% |  |
| Storage Length (ti) |  | 0 | 0 |  | 0 | 0 |
| Storage Lanes |  | 0 | 0 |  | 1 | 0 |
| Taper Length ( t ) |  |  | 25 |  | 25 |  |
| Lane Util. Factor | 0.95 | 0.95 | 0.95 | 0.95 | 1.00 | 1.00 |
| Ped Bike Factor |  |  |  |  | 1.00 |  |
| Fit |  |  |  |  | 0.886 |  |
| Flt Protected |  |  |  |  | 0.992 |  |
| Satd. Flow (prot) | 3421 | 0 | 0 | 3421 | 1746 | 0 |
| Flt Permitted |  |  |  | 0.948 | 0.992 |  |
| Satd. Flow (perm) | 3421 | 0 | 0 | 3243 | 1741 | 0 |
| Right Turn on Red |  | Yes |  |  |  | Yes |
| Satd. Flow (RTOR) |  |  |  |  | 59 |  |
| Link Speed (mph) | 30 |  |  | 30 | 30 |  |
| Link Distance (t) | 258 |  |  | 323 | 552 |  |
| Travel Time (s) | 5.9 |  |  | 7.3 | 12.5 |  |
| Confl. Peds. (\#/hr) |  |  |  |  | 11 |  |
| Confl. Bikes (\#/hr) |  |  |  |  |  |  |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Growth Factor | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% |
| Heavy Vehicles (\%) | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% |
| Bus Blockages (\#hr) | 0 | 0 | 0 | 0 | 0 | 0 |
| Parking (\#/hr) |  |  |  |  |  |  |
| Mid-Block Trafic (\%) | 0\% |  |  | 0\% | 0\% |  |
| Adj. Flow (vph) | 743 | 0 | 8 | 828 | 11 | 59 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |
| Lane Group Flow (vph) | 743 | 0 | 0 | 836 | 70 | 0 |
| Turn Type | NA |  | Perm | NA | Prot |  |
| Protected Phases | 6 |  |  | 2 | 4 |  |
| Permitted Phases |  |  | 2 |  |  |  |
| Detector Phase | 6 |  | 2 | 2 | 4 |  |
| Switch Phase |  |  |  |  |  |  |
| Minimum Initial (s) | 15.0 |  | 15.0 | 15.0 | 7.0 |  |
| Minimum Split (s) | 20.8 |  | 20.8 | 20.8 | 12.0 |  |
| Total Split (s) | 90.0 |  | 90.0 | 90.0 | 30.0 |  |
| Total Split (\%) | 75.0\% |  | 75.0\% | 75.0\% | 25.0\% |  |
| Yellow Time (s) | 3.8 |  | 3.8 | 3.8 | 3.0 |  |
| All-Red Time (s) | 2.0 |  | 2.0 | 2.0 | 2.0 |  |
| Lost Time Adjust (s) | 0.0 |  |  | 0.0 | 0.0 |  |
| Total Lost Time (s) | 5.8 |  |  | 5.8 | 5.0 |  |
| Lead/Lag |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |
| Recall Mode | C-Min |  | C-Min | C-Min | None |  |
| Act Effct Green (s) | 105.3 |  |  | 105.3 | 7.5 |  |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Intersection Delay, s/veh | 7.5 |  |  |  |  |  |  |  |  |  |  |  |
| Intersection LOS | A |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 4 |  |  |  | F |  | 1 | + |  |  |  | F |
| Traffic Vol, veh/h | 5 | 0 | 0 | 0 | 6 | 3 | 18 | 29 | 0 | 0 | 0 | 7 |
| Future Vol, veh/h | 5 | 0 | 0 | 0 | 6 | 3 | 18 | 29 | 0 | 0 | 0 | 7 |
| Peak Hour Factor | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 6 | 0 | 0 | 0 | 7 | 3 | 21 | 34 | 0 | 0 | 0 | 8 |
| Number of Lanes | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 0 | 0 | 1 |
| Approach | EB |  |  |  | WB |  | NB |  |  |  |  | SB |
| Opposing Approach | WB |  |  |  | EB |  | SB |  |  |  |  | NB |
| Opposing Lanes | 1 |  |  |  | 1 |  | 1 |  |  |  |  | 2 |
| Conflicting Approach Left | SB |  |  |  | NB |  | EB |  |  |  |  | WB |
| Conflicting Lanes Left | 1 |  |  |  | 2 |  | 1 |  |  |  |  | 1 |
| Conflicting Approach Right | NB |  |  |  | SB |  | WB |  |  |  |  | EB |
| Conflicting Lanes Right | 2 |  |  |  | 1 |  | 1 |  |  |  |  | 1 |
| HCM Control Delay | 7.3 |  |  |  | 6.9 |  | 7.7 |  |  |  |  | 6.6 |
| HCM LOS | A |  |  |  | A |  | A |  |  |  |  | A |


| Lane | NBLn1 | NBLn2 | EBLn1 | WBLn1 | SBLn1 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $100 \%$ | $0 \%$ | $100 \%$ | $0 \%$ | $0 \%$ |
| Vol Thru, \% | $0 \%$ | $100 \%$ | $0 \%$ | $67 \%$ | $0 \%$ |
| Vol Right, \% | $0 \%$ | $0 \%$ | $0 \%$ | $33 \%$ | $100 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 18 | 29 | 5 | 9 | 7 |
| LT Vol | 18 | 0 | 5 | 0 | 0 |
| Through Vol | 0 | 29 | 0 | 6 | 0 |
| RT Vol | 0 | 0 | 0 | 3 | 7 |
| Lane Flow Rate | 21 | 34 | 6 | 10 | 8 |
| Geometry Grp | 7 | 7 | 2 | 2 | 5 |
| Degree of Util (X) | 0.029 | 0.043 | 0.007 | 0.011 | 0.008 |
| Departure Headway (Hd) | 5.066 | 4.565 | 4.249 | 3.845 | 3.501 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes |
| Cap | 710 | 788 | 838 | 925 | 1021 |
| Service Time | 2.774 | 2.274 | 2.296 | 1.892 | 1.528 |
| HCM Lane VIC Ratio | 0.03 | 0.043 | 0.007 | 0.011 | 0.008 |
| HCM Control Delay | 7.9 | 7.5 | 7.3 | 6.9 | 6.6 |
| HCM Lane LOS | A | A | A | A | A |
| HCM 95th-tile Q | 0.1 | 0.1 | 0 | 0 | 0 |


| Lane Group | fit | EBT EBT | EBR | WBL | WBT | WBR | NBL | + | + | S | 1 SBT | 4 SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  | 中 |  |  | 4 |  |  | * |  | 4 | W |  |
| Traffic Volume (vph) | 0 | 724 | 10 | 3 | 607 | 0 | 39 | 0 | 40 | 172 | 2 | 31 |
| Future Volume (vph) | 0 | 724 | 10 | 3 | 607 | 0 | 39 | 0 | 40 | 172 | 2 | 31 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Width (ft) | 10 | 10 | 10 | 10 | 10 | 10 | 15 | 15 | 15 | 12 | 11 | 11 |
| Grade (\%) |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Storage Length (ft) | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 200 |
| Storage Lanes | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 1 |  | 1 |
| Taper Length ( ft ) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Ped Bike Factor |  | 1.00 |  |  | 1.00 |  |  | 0.98 |  | 0.99 | 0.97 |  |
| Frt |  | 0.998 |  |  |  |  |  | 0.931 |  |  | 0.860 |  |
| Flt Protected |  |  |  |  |  |  |  | 0.976 |  | 0.950 |  |  |
| Satd. Flow (prot) | 0 | 3294 | 0 | 0 | 1739 | 0 | 0 | 1839 | 0 | 1770 | 1506 | 0 |
| Flt Permitted |  |  |  |  | 0.996 |  |  | 0.839 |  | 0.672 |  |  |
| Satd. Flow (perm) | 0 | 3294 | 0 | 0 | 1732 | 0 | 0 | 1568 | 0 | 1240 | 1506 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 3 |  |  |  |  |  | 40 |  |  | 42 |  |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 30 |  |  | 30 |  |
| Link Distance (ft) |  | 333 |  |  | 411 |  |  | 360 |  |  | 606 |  |
| Travel Time (s) |  | 7.6 |  |  | 9.3 |  |  | 8.2 |  |  | 13.8 |  |
| Confl. Peds. (\#/hr) | 18 |  | 12 | 12 |  | 18 | 10 |  | 7 | 7 |  | 10 |
| Confl. Bikes (\#/hr) |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour Factor | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 |
| Growth Factor | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% |
| Heavy Vehicles (\%) | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% |
| Bus Blockages (\#/hr) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Parking (\#/hr) |  |  |  |  |  |  |  |  |  |  |  |  |
| Mid-Block Traffic (\%) |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Adj. Flow (vph) | 0 | 992 | 14 | 4 | 832 | 0 | 53 | 0 | 55 | 236 | 3 | 42 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 1006 | 0 | 0 | 836 | 0 | 0 | 108 | 0 | 236 | 45 | 0 |
| Turn Type |  | NA |  | Perm | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases |  | 2 |  |  | 2 |  |  | 4 |  |  | 4 |  |
| Permitted Phases |  |  |  | 2 |  |  | 4 |  |  | 4 |  |  |
| Detector Phase |  | 2 |  | 2 | 2 |  | 4 | 4 |  | 4. | 4 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) |  | 15.0 |  | 15.0 | 15.0 |  | 7.0 | 7.0 |  | 7.0 | 7.0 |  |
| Minimum Split (s) |  | 21.1 |  | 21.1 | 21.1 |  | 12.1 | 12.1 |  | 12.1 | 12.1 |  |
| Total Split (s) |  | 88.0 |  | 88.0 | 88.0 |  | 32.0 | 32.0 |  | 32.0 | 32.0 |  |
| Total Split (\%) |  | 73.3\% |  | 73.3\% | 73.3\% |  | 26.7\% | 26.7\% |  | 26.7\% | 26.7\% |  |
| Yellow Time (s) |  | 4.1 |  | 4.1 | 4.1 |  | 3.6 | 3.6 |  | 3.6 | 3.6 |  |
| All-Red Time (s) |  | 2.0 |  | 2.0 | 2.0 |  | 1.5 | 1.5 |  | 1.5 | 1.5 |  |
| Lost Time Adjust (s) |  | 0.0 |  |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time (s) |  | 6.1 |  |  | 6.1 |  |  | 5.1 |  | 5.1 | 5.1 |  |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode |  | C-Min |  | C-Min | C-Min |  | None | None |  | None | None |  |
| Act Effit Green (s) |  | 82.3 |  |  | 82.3 |  |  | 26.5 |  | 26.5 | 26.5 |  |


| Lane Group | EBL | $\rightarrow$ |  |  | - WBT |  | NBL | + ${ }_{\text {NBT }}$ | NBR | 4 SBL | $\downarrow$ | 1 SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Actuated g/C Ratio |  | 0.69 |  |  | 0.69 |  |  | 0.22 |  | 0.22 | 0.22 |  |
| v/c Ratio |  | 0.44 |  |  | 0.70 |  |  | 0.29 |  | 0.86 | 0.12 |  |
| Control Delay |  | 9.6 |  |  | 20.8 |  |  | 25.8 |  | 73.7 | 12.5 |  |
| Queue Delay |  | 0.0 |  |  | 0.6 |  |  | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay |  | 9.6 |  |  | 21.4 |  |  | 25.8 |  | 73.7 | 12.5 |  |
| LOS |  | A |  |  | C |  |  | C |  | E | B |  |
| Approach Delay |  | 9.6 |  |  | 21.4 |  |  | 25.8 |  |  | 63.9 |  |
| Approach LOS |  | A |  |  | C |  |  | C |  |  | E |  |
| Queue Length 50th (ft) |  | 181 |  |  | 408 |  |  | 42 |  | 172 | 2 |  |
| Queue Length 95th (ti) |  | 154 |  |  | 403 |  |  | 70 |  | 212 | 22 |  |
| Internal Link Dist (tt) |  | 253 |  |  | 331 |  |  | 280 |  |  | 526 |  |
| Turn Bay Length ( ft ) |  |  |  |  |  |  |  |  |  |  |  |  |
| Base Capacity (vph) |  | 2302 |  |  | 1210 |  |  | 401 |  | 293 | 388 |  |
| Starvation Cap Reductn |  | 0 |  |  | 123 |  |  | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn |  | 0 |  |  | 0 |  |  | 0 |  | 0 | 0 |  |
| Storage Cap Reductn |  |  |  |  | 0 |  |  | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio |  | 0.44 |  |  | 0.77 |  |  | 0.27 |  | 0.81 | 0.12 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset: $79(66 \%)$, Referenced to phase 2:EBWB, Start of Yellow |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 55 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Actuated-Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 0.86 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay: 21.7 |  |  |  |  | Intersection LOS: C |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 60.0\% ICU Level of Service B |  |  |  |  |  |  |  |  |  |  |  |  |
| Analysis Period (min) 15 |  |  |  |  |  |  |  |  |  |  |  |  |
| Splits and Phases: 1: LAFAYETTE STREET \& U.S. ROUTE 1 |  |  |  |  |  |  |  |  |  |  |  |  |
| $\stackrel{\square}{\square}$ |  |  |  |  |  |  |  |  | 1404 |  |  |  |

Lanes, Volumes, Timings
2: NORTH STATE STREET \& U.S. ROUTE 1

| Lane Group | - ${ }_{\text {EBT }}$ | EBR | WBL | - | 4 NBL | N |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | 中 |  |  | $4{ }^{4}$ | \% |  |
| Traffic Volume (Vph) | 952 | 3 | 6 | 640 | 2 | 137 |
| Future Volume (vph) | 952 | 3 | 6 | 640 | 2 | 137 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Width ( ft ) | 11 | 11 | 11 | 11 | 14 | 14 |
| Grade (\%) | 0\% |  |  | 0\% | 0\% |  |
| Storage Length ( ft ) |  | 0 | 0 |  | 0 | 0 |
| Storage Lanes |  | 0 | 0 |  | 1 | 0 |
| Taper Length (ft) |  |  | 25 |  | 25 |  |
| Lane Util, Factor | 0.95 | 0.95 | 0.95 | 0.95 | 1.00 | 1.00 |
| Ped Bike Factor | 1.00 |  |  | 1.00 | 1.00 |  |
| Frt |  |  |  |  | 0.867 |  |
| Flt Protected |  |  |  |  | 0.999 |  |
| Satd. Flow (prot) | 3421 | 0 | 0 | 3421 | 1721 | 0 |
| Flt Permitted |  |  |  | 0.944 | 0.999 |  |
| Satd. Flow (perm) | 3421 | 0 | 0 | 3230 | 1721 | 0 |
| Right Turn on Red |  | Yes |  |  |  | Yes |
| Satd. Flow (RTOR) | 1 |  |  |  | 139 |  |
| Link Speed (mph) | 30 |  |  | 30 | 30 |  |
| Link Distance (ft) | 258 |  |  | 323 | 552 |  |
| Travel Time (s) | 5.9 |  |  | 7.3 | 12.5 |  |
| Confl. Peds. (\#/hr) |  | 2 | 2 |  | 3 |  |
| Confl. Bikes (\#/hr) |  |  |  |  |  |  |
| Peak Hour Factor | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 |
| Growth Factor | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% |
| Heavy Vehicles (\%) | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% |
| Bus Blockages (\#/hr) | 0 | 0 | 0 | 0 | 0 | 0 |
| Parking (\#/hr) |  |  |  |  |  |  |
| Mid-Block Traffic (\%) | 0\% |  |  | 0\% | 0\% |  |
| Adj. Flow (vph) | 1094 | 3 | 7 | 736 | 2 | 157 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |
| Lane Group Flow (vph) | 1097 | 0 | 0 | 743 | 159 | 0 |
| Turn Type | NA |  | Perm | NA | Prot |  |
| Protected Phases | 6 |  |  | 2 | 4 |  |
| Permitted Phases |  |  | 2 |  |  |  |
| Detector Phase | 6 |  | 2 | 2 | 4 |  |
| Switch Phase |  |  |  |  |  |  |
| Minimum Initial (s) | 15.0 |  | 15.0 | 15.0 | 7.0 |  |
| Minimum Split (s) | 20.8 |  | 20.8 | 20.8 | 12.0 |  |
| Total Split (s) | 90.0 |  | 90.0 | 90.0 | 30.0 |  |
| Total Split (\%) | 75.0\% |  | 75.0\% | 75.0\% | 25.0\% |  |
| Yellow Time (s) | 3.8 |  | 3.8 | 3.8 | 3.0 |  |
| All-Red Time (s) | 2.0 |  | 2.0 | 2.0 | 2.0 |  |
| Lost Time Adjust (s) | 0.0 |  |  | 0.0 | 0.0 |  |
| Total Lost Time (s) | 5.8 |  |  | 5.8 | 5.0 |  |
| Lead/Lag |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |
| Recall Mode | C-Min |  | C-Min | C-Min | None |  |
| Act Effct Green (s) | 100.7 |  |  | 100.7 | 8.5 |  |


| Lane Group | $\rightarrow$ | EBR | $\stackrel{7}{\text { WBL }}$ |  | 4 | NBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Actuated g/C Ratio | 0.84 |  |  | 0.84 | 0.07 |  |
| v/c Ratio | 0.38 |  |  | 0.27 | 0.64 |  |
| Control Delay | 2.5 |  |  | 2.4 | 23.9 |  |
| Queue Delay | 0.1 |  |  | 0.0 | 0.0 |  |
| Total Delay | 2.6 |  |  | 2.4 | 23.9 |  |
| LOS | A |  |  | A | C |  |
| Approach Delay | 2.6 |  |  | 2.4 | 23.9 |  |
| Approach LOS | A |  |  | A | C |  |
| Queue Length 50th ( t ) | 50 |  |  | 41 | 15 |  |
| Queue Length 95th (ft) | 127 |  |  | 74 | 74 |  |
| Internal Link Dist (ft) | 178 |  |  | 243 | 472 |  |
| Turn Bay Length ( t ) |  |  |  |  |  |  |
| Base Capacity (vph) | 2871 |  |  | 2710 | 468 |  |
| Starvation Cap Reductn | 594 |  |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 |  |  | 150 | 0 |  |
| Storage Cap Reductn | 0 |  |  | 0 | 0 |  |
| Reduced vic Ratio | 0.48 |  |  | 0.29 | 0.34 |  |
| Intersection Summary |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |
| Cycle Length: 120 |  |  |  |  |  |  |
| Actuated Cycle Length: 120 |  |  |  |  |  |  |
| Offset: $95(79 \%$, Referenced to phase 2:WBTL and 6:EBT, Start of Yellow |  |  |  |  |  |  |
| Natural Cycle: 40 |  |  |  |  |  |  |
| Control Type: Actuated-Coordinated |  |  |  |  |  |  |
| Maximum v/c Ratio: 0.64 |  |  |  |  |  |  |
| Intersection Signal Delay: 4.2 |  |  |  |  | sectio | OS: A |
| Intersection Capacity Utilization 44.0\% |  |  |  |  | Level | Service A |
| Analysis Period (min) 15 |  |  |  |  |  |  |

Splits and Phases: 2: NORTH STATE STREET \& U.S. ROUTE 1


| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Intersection Delay, s/veh Intersection LOS | $\begin{array}{r} 7.9 \\ \text { A } \end{array}$ |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 4 |  |  |  | F |  | \% | \$ |  |  |  | ${ }^{7}$ |
| Traffic Vol, veh/h | 10 | 0 | 0 | 0 | 12 | 9 | 64 | 60 | 0 | 0 | 0 | 15 |
| Future Vol, veh/h | 10 | 0 | 0 | 0 | 12 | 9 | 64 | 60 | 0 | 0 | 0 | 15 |
| Peak Hour Factor | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 13 | 0 | 0 | 0 | 15 | 11 | 80 | 75 | 0 | 0 | 0 | 19 |
| Number of Lanes | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 0 | 0 | 1 |
| Approach | EB |  |  |  | WB |  | NB |  |  |  |  | SB |
| Opposing Approach | WB |  |  |  | EB |  | SB |  |  |  |  | NB |
| Opposing Lanes | 1 |  |  |  | 1 |  | 1 |  |  |  |  | 2 |
| Conflicting Approach Left | SB |  |  |  | NB |  | EB |  |  |  |  | WB |
| Conflicting Lanes Left | 1 |  |  |  | 2 |  | 1 |  |  |  |  | 1 |
| Conflicting Approach Right | NB |  |  |  | SB |  | WB |  |  |  |  | EB |
| Conflicting Lanes Right | 2 |  |  |  | 1 |  | 1 |  |  |  |  | 1 |
| HCM Control Delay | 7.7 |  |  |  | 7.2 |  | 8.2 |  |  |  |  | 6.8 |
| HCM LOS | A |  |  |  | A |  | A |  |  |  |  | A |


| Lane | NBLn1 | NBLn2 | EBLn1 | WBLn1 | SBLn1 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $100 \%$ | $0 \%$ | $100 \%$ | $0 \%$ | $0 \%$ |
| Vol Thru, \% | $0 \%$ | $100 \%$ | $0 \%$ | $57 \%$ | $0 \%$ |
| Vol Right, \% | $0 \%$ | $0 \%$ | $0 \%$ | $43 \%$ | $100 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 64 | 60 | 10 | 21 | 15 |
| LT Vol | 64 | 0 | 10 | 0 | 0 |
| Through Vol | 0 | 60 | 0 | 12 | 0 |
| RT Vol | 0 | 0 | 0 | 9 | 15 |
| Lane Flow Rate | 80 | 75 | 12 | 26 | 19 |
| Geometry Grp | 7 | 7 | 2 | 2 | 5 |
| Degree of Util (X) | 0.114 | 0.096 | 0.016 | 0.03 | 0.019 |
| Departure Headway (Hd) | 5.111 | 4.61 | 4.581 | 4.11 | 3.611 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes |
| Cap | 702 | 778 | 786 | 876 | 976 |
| Service Time | 2.836 | 2.335 | 2.582 | 2.111 | 1.691 |
| HCM Lane V/C Ratio | 0.114 | 0.096 | 0.015 | 0.03 | 0.019 |
| HCM Control Delay | 8.5 | 7.8 | 7.7 | 7.2 | 6.8 |
| HCM Lane LOS | A | A | A | A | A |
| HCM 95th-tile Q | 0.4 | 0.3 | 0 | 0.1 | 0.1 |

CAPACITY ANALYSIS WORKSHEETS
No-Build Conditions


| Lane Group | $\Rightarrow$ <br> EBL | $\begin{aligned} & \rightarrow \\ & \text { EBT } \end{aligned}$ |  |  | + | $4$ WBR | NBL | $\stackrel{\uparrow}{\text { NBT }}$ | NBR | \$ | $\dagger$ SBT | + |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Actuated g/C Ratio |  | 0.68 |  |  | 0.68 |  |  | 0.22 |  | 0.22 | 0.22 |  |
| v/c Ratio |  | 0.23 |  |  | 0.43 |  |  | 0.11 |  | 0.84 | 0.16 |  |
| Control Delay |  | 8.2 |  |  | 9.9 |  |  | 16.7 |  | 67.7 | 9.9 |  |
| Queue Delay |  | 0.0 |  |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay |  | 8.2 |  |  | 9.9 |  |  | 16.7 |  | 67.7 | 9.9 |  |
| LOS |  | A |  |  | A |  |  | B |  | E | A |  |
| Approach Delay |  | 8.2 |  |  | 9.9 |  |  | 16.7 |  |  | 56.5 |  |
| Approach LOS |  | A |  |  | A |  |  | B |  |  | E |  |
| Queue Length 50th (ft) |  | 72 |  |  | 154 |  |  | 9 |  | 189 | 2 |  |
| Queue Length 95th (ft) |  | 120 |  |  | 288 |  |  | 34 |  | 260 | 33 |  |
| Internal Link Dist (tt) |  | 253 |  |  | 331 |  |  | 280 |  |  | 526 |  |
| Turn Bay Length (t) |  |  |  |  |  |  |  |  |  |  |  |  |
| Base Capacity (vph) |  | 2274 |  |  | 2169 |  |  | 477 |  | 377 | 463 |  |
| Starvation Cap Reductn |  | 0 |  |  | 0 |  |  | 0 |  | , |  |  |
| Spillback Cap Reductn |  | 0 |  |  | 0 |  |  | 0 |  | 0 | 0 |  |
| Storage Cap Reductn |  | 0 |  |  | 0 |  |  | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio |  | 0.23 |  |  | 0.43 |  |  | 0.09 |  | 0.67 | 0.13 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset: 27 (23\%), Referenced to phase 2:EBWB, Start of Yellow |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 45 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Actuated-Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 0.84 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay: 17.8 |  |  |  |  | Intersection LOS: B |  |  |  |  |  |  |  |
| Intersection Capacity Utilization $53.2 \%$Analysis Period (min) 15 |  |  |  |  | ICU Level of Service A |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases: 1: LAFAYETTE STREET \& U.S. ROUTE 1


|  | $\rightarrow$ | \% | 7 |  | 4 | 7 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | 性 |  |  | 4 | M |  |
| Traffic Volume (vph) | 726 | 0 | 8 | 809 | 11 | 58 |
| Future Volume (vph) | 726 | 0 | 8 | 809 | 11 | 58 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Width (ti) | 11 | 11 | 11 | 11 | 14 | 14 |
| Grade (\%) | 0\% |  |  | 0\% | 0\% |  |
| Storage Length (ti) |  | 0 | 0 |  | 0 | 0 |
| Storage Lanes |  | 0 | 0 |  | 1 | 0 |
| Taper Length ( ft ) |  |  | 25 |  | 25 |  |
| Lane Util. Factor | 0.95 | 0.95 | 0.95 | 0.95 | 1.00 | 1.00 |
| Ped Bike Factor |  |  |  |  | 1.00 |  |
| Fit |  |  |  |  | 0.886 |  |
| Flt Protected |  |  |  |  | 0.992 |  |
| Satd. Flow (prot) | 3421 | 0 | 0 | 3421 | 1746 | 0 |
| Flt Permitted |  |  |  | 0.948 | 0.992 |  |
| Satd. Flow (perm) | 3421 | 0 | 0 | 3243 | 1741 | 0 |
| Right Turn on Red |  | Yes |  |  |  | Yes |
| Satd. Flow (RTOR) |  |  |  |  | 60 |  |
| Link Speed (mph) | 30 |  |  | 30 | 30 |  |
| Link Distance ( ft ) | 258 |  |  | 323 | 552 |  |
| Travel Time (s) | 5.9 |  |  | 7.3 | 12.5 |  |
| Confl. Peds. (\#/hr) |  |  |  |  | 11 |  |
| Confl. Bikes (\#hr) |  |  |  |  |  |  |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Growth Factor | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% |
| Heavy Vehicles (\%) | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% |
| Bus Blockages (\#hr) | 0 | 0 | 0 | 0 | 0 | 0 |
| Parking (\#\#hr) |  |  |  |  |  |  |
| Mid-Block Traffic (\%) | 0\% |  |  | 0\% | 0\% |  |
| Adj. Flow (vph) | 756 | 0 | 8 | 843 | 11 | 60 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |
| Lane Group Flow (vph) | 756 | 0 | 0 | 851 | 71 | 0 |
| Turn Type | NA |  | Perm | NA | Prot |  |
| Protected Phases | 6 |  |  | 2 | 4 |  |
| Permitted Phases |  |  | 2 |  |  |  |
| Detector Phase | 6 |  | 2 | 2 | 4 |  |
| Switch Phase |  |  |  |  |  |  |
| Minimum Initial (s) | 15.0 |  | 15.0 | 15.0 | 7.0 |  |
| Minimum Split (s) | 20.8 |  | 20.8 | 20.8 | 12.0 |  |
| Total Split (s) | 90.0 |  | 90.0 | 90.0 | 30.0 |  |
| Total Split (\%) | 75.0\% |  | 75.0\% | 75.0\% | 25.0\% |  |
| Yellow Time (s) | 3.8 |  | 3.8 | 3.8 | 3.0 |  |
| All-Red Time (s) | 2.0 |  | 2.0 | 2.0 | 2.0 |  |
| Lost Time Adjust (s) | 0.0 |  |  | 0.0 | 0.0 |  |
| Total Lost Time (s) | 5.8 |  |  | 5.8 | 5.0 |  |
| Lead/Lag |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |
| Recall Mode | C-Min |  | C-Min | C-Min | None |  |
| Act Effct Green (s) | 105.3 |  |  | 105.3 | 7.5 |  |



Splits and Phases: 2: NORTH STATE STREET \& U.S. ROUTE 1


| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Intersection Delay, s/veh | 7.5 |  |  |  |  |  |  |  |  |  |  |  |
| Intersection LOS | A |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 4 |  |  |  | \% |  | 1 | 4 |  |  |  | \% |
| Traffic Vol, veh/h | 5 | 0 | 0 | 0 | 6 | 3 | 18 | 30 | 0 | 0 | 0 | 7 |
| Future Vol, veh/h | 5 | 0 | 0 | 0 | 6 | 3 | 18 | 30 | 0 | 0 | 0 | 7 |
| Peak Hour Factor | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 6 | 0 | 0 | 0 | 7 | 3 | 21 | 35 | 0 | 0 | 0 | 8 |
| Number of Lanes | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 0 | 0 | 1 |
| Approach | EB |  |  |  | WB |  | NB |  |  |  |  | SB |
| Opposing Approach | WB |  |  |  | EB |  | SB |  |  |  |  | NB |
| Opposing Lanes | 1 |  |  |  | 1 |  | 1 |  |  |  |  | 2 |
| Conflicting Approach Left | SB |  |  |  | NB |  | EB |  |  |  |  | WB |
| Conflicting Lanes Left | 1 |  |  |  | 2 |  | 1 |  |  |  |  | 1 |
| Conflicting Approach Right | NB |  |  |  | SB |  | WB |  |  |  |  | EB |
| Conflicting Lanes Right | 2 |  |  |  | 1 |  | 1 |  |  |  |  | 1 |
| HCM Control Delay | 7.3 |  |  |  | 6.9 |  | 7.7 |  |  |  |  | 6.6 |
| HCM LOS | A |  |  |  | A |  | A |  |  |  |  | A |


| Lane | NBLn1 | NBLLn2 | EBLn1 | WBLn1 | SBLn1 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $100 \%$ | $0 \%$ | $100 \%$ | $0 \%$ | $0 \%$ |
| Vol Thru, \% | $0 \%$ | $100 \%$ | $0 \%$ | $67 \%$ | $0 \%$ |
| Vol Right, \% | $0 \%$ | $0 \%$ | $0 \%$ | $33 \%$ | $100 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 18 | 30 | 5 | 9 | 7 |
| LT Vol | 18 | 0 | 5 | 0 | 0 |
| Through Vol | 0 | 30 | 0 | 6 | 0 |
| RT Vol | 0 | 0 | 0 | 3 | 7 |
| Lane Flow Rate | 21 | 35 | 6 | 10 | 8 |
| Geometry Grp | 7 | 7 | 2 | 2 | 5 |
| Degree of Util (X) | 0.029 | 0.044 | 0.007 | 0.011 | 0.008 |
| Departure Headway (Hd) | 5.066 | 4.565 | 4.251 | 3.847 | 3.502 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes |
| Cap | 710 | 788 | 838 | 925 | 1021 |
| Service Time | 2.774 | 2.274 | 2.298 | 1.894 | 1.528 |
| HCM Lane VIC Ratio | 0.03 | 0.044 | 0.007 | 0.011 | 0.008 |
| HCM Control Delay | 7.9 | 7.5 | 7.3 | 6.9 | 6.6 |
| HCM Lane LOS | A | A | A | A | A |
| HCM 95th-tile Q | 0.1 | 0.1 | 0 | 0 | 0 |


| Lane Group | - | $\begin{aligned} & \rightarrow \infty \\ & \text { EBT } \end{aligned}$ | EBR | WBL | W- WBT | 4 WBR | + NBL | + NBT | NBR | SBL | ¢ SBT | $\downarrow$ SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  | 性 |  |  | 4 |  |  | \$ |  | 1 | F |  |
| Traffic Volume (vph) | 0 | 737 | 10 | 3 | 618 | 0 | 40 | 0 | 41 | 175 | 2 | 32 |
| Future Volume (vph) | 0 | 737 | 10 | 3 | 618 | 0 | 40 | 0 | 41 | 175 | 2 | 32 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Width (ft) | 10 | 10 | 10 | 10 | 10 | 10 | 15 | 15 | 15 | 12 | 11 | 11 |
| Grade (\%) |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Storage Length (ti) | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 200 |
| Storage Lanes | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 1 |  | 1 |
| Taper Length (t) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Ped Bike Factor |  | 1.00 |  |  | 1.00 |  |  | 0.98 |  | 0.99 | 0.97 |  |
| Fit |  | 0.998 |  |  |  |  |  | 0.932 |  |  | 0.860 |  |
| Flt Protected |  |  |  |  |  |  |  | 0.976 |  | 0.950 |  |  |
| Satd. Flow (prot) | 0 | 3294 | 0 | 0 | 1739 | 0 | 0 | 1841 | 0 | 1770 | 1506 | 0 |
| Flt Permitted |  |  |  |  | 0.996 |  |  | 0.836 |  | 0.669 |  |  |
| Satd. Flow (perm) | 0 | 3294 | 0 | 0 | 1732 | 0 | 0 | 1564 | 0 | 1234 | 1506 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 3 |  |  |  |  |  | 39 |  |  | 44 |  |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 30 |  |  | 30 |  |
| Link Distance (t) |  | 333 |  |  | 411 |  |  | 360 |  |  | 606 |  |
| Travel Time (s) |  | 7.6 |  |  | 9.3 |  |  | 8.2 |  |  | 13.8 |  |
| Confl. Peds. (\#/hr) | 18 |  | 12 | 12 |  | 18 | 10 |  | 7 | 7 |  | 10 |
| Confl. Bikes (\#hr) |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour Factor | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 |
| Growth Factor | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% |
| Heavy Vehicles (\%) | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% |
| Bus Blockages (\#/hr) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Parking (\#/hr) |  |  |  |  |  |  |  |  |  |  |  |  |
| Mid-Block Traffic (\%) |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Adj. Flow (vph) | 0 | 1010 | 14 | 4 | 847 | 0 | 55 | 0 | 56 | 240 | 3 | 44 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 1024 | 0 | 0 | 851 | 0 | 0 | 111 | 0 | 240 | 47 | 0 |
| Turn Type |  | NA |  | Perm | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases |  | 2 |  |  | 2 |  |  | 4 |  |  | 4 |  |
| Permitted Phases |  |  |  | 2 |  |  | 4 |  |  | 4 |  |  |
| Detector Phase |  | 2 |  | 2 | 2 |  | 4 | 4 |  | 4 | 4 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) |  | 15.0 |  | 15.0 | 15.0 |  | 7.0 | 7.0 |  | 7.0 | 7.0 |  |
| Minimum Split (s) |  | 21.1 |  | 21.1 | 21.1 |  | 12.1 | 12.1 |  | 12.1 | 12.1 |  |
| Total Split (s) |  | 88.0 |  | 88.0 | 88.0 |  | 32.0 | 32.0 |  | 32.0 | 32.0 |  |
| Total Split (\%) |  | 73.3\% |  | 73.3\% | 73.3\% |  | 26.7\% | 26.7\% |  | 26.7\% | 26.7\% |  |
| Yellow Time (s) |  | 4.1 |  | 4.1 | 4.1 |  | 3.6 | 3.6 |  | 3.6 | 3.6 |  |
| All-Red Time (s) |  | 2.0 |  | 2.0 | 2.0 |  | 1.5 | 1.5 |  | 1.5 | 1.5 |  |
| Lost Time Adjusi (s) |  | 0.0 |  |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time (s) |  | 6.1 |  |  | 6.1 |  |  | 5.1 |  | 5.1 | 5.1 |  |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode |  | C-Min |  | C-Min | C-Min |  | None | None |  | None | None |  |
| Act Effct Green (s) |  | 81.8 |  |  | 81.8 |  |  | 27.0 |  | 27.0 | 27.0 |  |


|  |  |  |  |  |  |  | , | $\dagger$ | P | + | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Actuated g/C Ratio |  | 0.68 |  |  | 0.68 |  |  | 0.22 |  | 0.22 | 0.22 |  |
| v/c Ratio |  | 0.46 |  |  | 0.72 |  |  | 0.29 |  | 0.86 | 0.13 |  |
| Control Delay |  | 10.0 |  |  | 23.1 |  |  | 26.1 |  | 73.0 | 12.3 |  |
| Queue Delay |  | 0.0 |  |  | 1.0 |  |  | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay |  | 10.0 |  |  | 24.1 |  |  | 26.1 |  | 73.0 | 12.3 |  |
| LOS |  | A |  |  | C |  |  | C |  | E | B |  |
| Approach Delay |  | 10.0 |  |  | 24.1 |  |  | 26.1 |  |  | 63.1 |  |
| Approach LOS |  | A |  |  | C |  |  | C |  |  | E |  |
| Queue Length 50th (fi) |  | 190 |  |  | 455 |  |  | 44 |  | 174 | 2 |  |
| Queue Length 95th (fit) |  | 157 |  |  | 455 |  |  | 73 |  | \#217 | 22 |  |
| Internal Link Dist (tt) |  | 253 |  |  | 331 |  |  | 280 |  |  | 526 |  |
| Turn Bay Length (ft) |  |  |  |  |  |  |  |  |  |  |  |  |
| Base Capacity (vph) |  | 2294 |  |  | 1206 |  |  | 403 |  | 295 | 393 |  |
| Starvation Cap Reductn |  | 0 |  |  | 150 |  |  | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn |  | 0 |  |  | 0 |  |  | 0 |  | 0 | 0 |  |
| Storage Cap Reductn |  | 0 |  |  | 0 |  |  | 0 |  | 0 | 0 |  |
| Reduced vic Ratio |  | 0.45 |  |  | 0.81 |  |  | 0.28 |  | 0.81 | 0.12 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset: 79 (66\%), Referenced to phase 2:EBWB, Start of Yellow |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 55 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Actuated-Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 0.86 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay: 22.8 |  |  |  |  | Intersection LOS: C |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 60.7\% ICU Level of Service B |  |  |  |  |  |  |  |  |  |  |  |  |
| Analysis Period (min) 15 |  |  |  |  |  |  |  |  |  |  |  |  |
| \# 95th percentile volume exceeds capacity, queue may be longer. |  |  |  |  |  |  |  |  |  |  |  |  |
| Splits and Phases: 1: LAFAYETTE STREET \& U.S. ROUTE 1 |  |  |  |  |  |  |  |  |  |  |  |  |
| ${ }_{\square}^{4} 00_{0}(\mathrm{R})$ |  |  |  |  |  |  |  |  | 1404 |  |  |  |

2: NORTH STATE STREET \& U.S. ROUTE 1

| Lane Group | $\underset{E B T}{\rightarrow}$ | EBR | WBL |  | $4$ <br> NBL | NBR |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | 性 |  |  | $4{ }_{4}$ | K |  |  |
| Traffic Volume (vph) | 969 | 3 | 6 | 652 | 2 | 139 |  |
| Future Volume (vph) | 969 | 3 | 6 | 652 | 2 | 139 |  |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |  |
| Lane Width ( f ) | 11 | 11 | 11 | 11 | 14 | 14 |  |
| Grade (\%) | 0\% |  |  | 0\% | 0\% |  |  |
| Storage Length (ft) |  | 0 | 0 |  | 0 | 0 |  |
| Storage Lanes |  | 0 | 0 |  | , | 0 |  |
| Taper Length (ft) |  |  | 25 |  | 25 |  |  |
| Lane Util. Factor | 0.95 | 0.95 | 0.95 | 0.95 | 1.00 | 1.00 |  |
| Ped Bike Factor | 1.00 |  |  | 1.00 | 1.00 |  |  |
| Fit |  |  |  |  | 0.867 |  |  |
| Flt Protected |  |  |  |  | 0.999 |  |  |
| Satd. Flow (prot) | 3421 | 0 | 0 | 3421 | 1721 | 0 |  |
| Fll Permitted |  |  |  | 0.944 | 0.999 |  |  |
| Satd. Flow (perm) | 3421 | 0 | 0 | 3230 | 1721 | 0 |  |
| Right Turn on Red |  | Yes |  |  |  | Yes |  |
| Satd. Flow (RTOR) | 1 |  |  |  | 133 |  |  |
| Link Speed (mph) | 30 |  |  | 30 | 30 |  |  |
| Link Distance (ft) | 258 |  |  | 323 | 552 |  |  |
| Travel Time (s) | 5.9 |  |  | 7.3 | 12.5 |  |  |
| Confi. Peds. (\#hr) |  | 2 | 2 |  | 3 |  |  |
| Confl. Bikes (\#/hr) |  |  |  |  |  |  |  |
| Peak Hour Factor | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 |  |
| Growth Factor | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% |  |
| Heavy Vehicles (\%) | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% |  |
| Bus Blockages (\#/hr) | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Parking (\#/hr) |  |  |  |  |  |  |  |
| Mid-Block Traffic (\%) | 0\% |  |  | 0\% | 0\% |  |  |
| Adj. Flow (vph) | 1114 | 3 | 7 | 749 | 2 | 160 |  |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 1117 | 0 | 0 | 756 | 162 | 0 |  |
| Turn Type | NA |  | Perm | NA | Prot |  |  |
| Protected Phases | 6 |  |  | 2 | 4 |  |  |
| Permitted Phases |  |  | 2 |  |  |  |  |
| Detector Phase | 6 |  | 2 | 2 | 4 |  |  |
| Switch Phase |  |  |  |  |  |  |  |
| Minimum Initial (s) | 15.0 |  | 15.0 | 15.0 | 7.0 |  |  |
| Minimum Split (s) | 20.8 |  | 20.8 | 20.8 | 12.0 |  |  |
| Total Split (s) | 90.0 |  | 90.0 | 90.0 | 30.0 |  |  |
| Total Split (\%) | 75.0\% |  | 75.0\% | 75.0\% | 25.0\% |  |  |
| Yellow Time (s) | 3.8 |  | 3.8 | 3.8 | 3.0 |  |  |
| All-Red Time (s) | 2.0 |  | 2.0 | 2.0 | 2.0 |  |  |
| Lost Time Adjust (s) | 0.0 |  |  | 0.0 | 0.0 |  |  |
| Total Lost Time (s) | 5.8 |  |  | 5.8 | 5.0 |  |  |
| Lead/Lag |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |
| Recall Mode | C-Min |  | C-Min | C-Min | None |  |  |
| Act Effct Green (s) | 100.4 |  |  | 100.4 | 8.8 |  |  |
| 01/05/2022 <br> HARDESTY \& HANOVER | C-STC |  |  |  |  |  | Synchro 10 Report Page 3 |



Intersection Summary
Area Type: Other

Cycle Length: 120
Actuated Cycle Length: 120
Offset: 95 (79\%), Referenced to phase 2:WBTL and 6:EBT, Start of Yellow
Natural Cycle: 40
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.65
Intersection Signal Delay: 4.5 Intersection LOS: A
Intersection Capacity Utilization 44.6\% ICU Level of Service A
Analysis Period (min) 15
Splits and Phases: 2: NORTH STATE STREET \& U.S. ROUTE 1


HCM 6th AWSC
MIXED-USE DEVELOPMENT, STAMFORD, CT
3: SOUTH STATE ST/LAFAYETTE STREET \& NORTH STATE ST

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Intersection Delay, s/veh | 7.9 |  |  |  |  |  |  |  |  |  |  |  |
| Intersection LOS | A |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 4 |  |  |  | * |  | 4 | 4 |  |  |  | \% |
| Traffic Vol, veh/h | 10 | 0 | 0 | 0 | 12 | 9 | 65 | 62 | 0 | 0 | 0 | 15 |
| Future Vol, veh/h | 10 | 0 | 0 | 0 | 12 | 9 | 65 | 62 | 0 | 0 | 0 | 15 |
| Peak Hour Factor | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 13 | 0 | 0 | 0 | 15 | 11 | 81 | 78 | 0 | 0 | 0 | 19 |
| Number of Lanes | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 0 | 0 | 1 |
| Approach | EB |  |  |  | WB |  | NB |  |  |  |  | SB |
| Opposing Approach | WB |  |  |  | EB |  | SB |  |  |  |  | NB |
| Opposing Lanes | 1 |  |  |  | 1 |  | 1 |  |  |  |  | 2 |
| Conflicting Approach Left | SB |  |  |  | NB |  | EB |  |  |  |  | WB |
| Conflicting Lanes Left | 1 |  |  |  | 2 |  | 1 |  |  |  |  | 1 |
| Conflicting Approach Right | NB |  |  |  | SB |  | WB |  |  |  |  | EB |
| Conflicting Lanes Right | 2 |  |  |  | 1 |  | 1 |  |  |  |  | 1 |
| HCM Control Delay | 7.7 |  |  |  | 7.2 |  | 8.2 |  |  |  |  | 6.8 |
| HCM LOS | A |  |  |  | A |  | A |  |  |  |  | A |


| Lane | NBLn1 | NBLn2 | EBLn1 | WBLn1 | SBLn1 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $100 \%$ | $0 \%$ | $100 \%$ | $0 \%$ | $0 \%$ |
| Vol Thru, \% | $0 \%$ | $100 \%$ | $0 \%$ | $57 \%$ | $0 \%$ |
| Vol Right, \% | $0 \%$ | $0 \%$ | $0 \%$ | $43 \%$ | $100 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 65 | 62 | 10 | 21 | 15 |
| LT Vol | 65 | 0 | 10 | 0 | 0 |
| Through Vol | 0 | 62 | 0 | 12 | 0 |
| RT Vol | 0 | 0 | 0 | 9 | 15 |
| Lane Flow Rate | 81 | 78 | 12 | 26 | 19 |
| Geometry Grp | 7 | 7 | 2 | 2 | 5 |
| Degree of Util (X) | 0.115 | 0.099 | 0.016 | 0.03 | 0.019 |
| Departure Headway (Hd) | 5.111 | 4.61 | 4.588 | 4.117 | 3.614 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes |
| Cap | 702 | 778 | 785 | 875 | 975 |
| Service Time | 2.836 | 2.335 | 2.589 | 2.118 | 1.694 |
| HCM Lane VIC Ratio | 0.115 | 0.1 | 0.015 | 0.03 | 0.019 |
| HCM Control Delay | 8.5 | 7.8 | 7.7 | 7.2 | 6.8 |
| HCM Lane LOS | A | A | A | A | A |
| HCM 95th-tile Q | 0.4 | 0.3 | 0 | 0.1 | 0.1 |

## CAPACITY ANALYSIS WORKSHEETS

## Build Conditions

| Lane Group | $\begin{aligned} & \ngtr \\ & \text { EBL } \end{aligned}$ | EBT |  | WBL | WBT | WBR | NBL | NBT | NBR |  |  | 4SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Configurations |  | 中* |  |  | 4 |  |  | 4 |  | \% | F |  |
| Traffic Volume (vph) | 0 | 463 | 1 | 3 | 819 | 0 | 42 | 0 | 38 | 228 | 3 | 52 |
| Future Volume (vph) | 0 | 463 | 1 | 3 | 819 | 0 | 42 | 0 | 38 | 228 | 3 | 52 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Width (ft) | 10 | 10 | 10 | 10 | 10 | 10 | 15 | 15 | 15 | 12 | 11 | 11 |
| Grade (\%) |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Storage Length (ft) | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 200 |
| Storage Lanes | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 1 |  | 1 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Ped Bike Factor |  | 1.00 |  |  | 1.00 |  |  | 0.98 |  | 1.00 | 0.98 |  |
| Frt |  |  |  |  |  |  |  | 0.935 |  |  | 0.857 |  |
| Flt Protected |  |  |  |  |  |  |  | 0.975 |  | 0.950 |  |  |
| Satd. Flow (prot) | 0 | 3303 | 0 | 0 | 3303 | 0 | 0 | 1845 | 0 | 1770 | 1510 | 0 |
| Flt Permitted |  |  |  |  | 0.954 |  |  | 0.831 |  | 0.701 |  |  |
| Satd. Flow (perm) | 0 | 3303 | 0 | 0 | 3151 | 0 | 0 | 1564 | 0 | 1299 | 1510 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  |  |  |  |  |  |  | 38 |  |  | 58 |  |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 30 |  |  | 30 |  |
| Link Distance (tt) |  | 333 |  |  | 411 |  |  | 360 |  |  | 606 |  |
| Travel Time (s) |  | 7.6 |  |  | 9.3 |  |  | 8.2 |  |  | 13.8 |  |
| Confl. Peds. (\#/hr) | 10 |  | 4 | 4 |  | 10 | 6 |  | 2 | 2 |  | 6 |
| Confl. Bikes (\#/hr) |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour Factor | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 |
| Growth Factor | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% |
| Heavy Vehicles (\%) | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% |
| Bus Blockages (\#/hr) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Parking (\#/hr) |  |  |  |  |  |  |  |  |  |  |  |  |
| Mid-Block Traffic (\%) |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Adj. Flow (vph) | 0 | 520 | 1 | 3 | 920 | 0 | 47 | 0 | 43 | 256 | 3 | 58 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 521 | 0 | 0 | 923 | 0 | 0 | 90 | 0 | 256 | 61 | 0 |
| Turn Type |  | NA |  | Perm | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases |  | 2 |  |  | 2 |  |  | 4 |  |  | 4 |  |
| Permitted Phases |  |  |  | 2 |  |  | 4 |  |  | 4 |  |  |
| Detector Phase |  | 2 |  | 2 | 2 |  | 4 | 4 |  | 4 | 4 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) |  | 15.0 |  | 15.0 | 15.0 |  | 7.0 | 7.0 |  | 7.0 | 7.0 |  |
| Minimum Split (s) |  | 21.1 |  | 21.1 | 21.1 |  | 12.1 | 12.1 |  | 12.1 | 12.1 |  |
| Total Split (s) |  | 82.0 |  | 82.0 | 82.0 |  | 38.0 | 38.0 |  | 38.0 | 38.0 |  |
| Total Split (\%) |  | 68.3\% |  | 68.3\% | 68.3\% |  | 31.7\% | 31.7\% |  | 31.7\% | 31.7\% |  |
| Yellow Time (s) |  | 4.1 |  | 4.1 | 4.1 |  | 3.6 | 3.6 |  | 3.6 | 3.6 |  |
| All-Red Time (s) |  | 2.0 |  | 2.0 | 2.0 |  | 1.5 | 1.5 |  | 1.5 | 1.5 |  |
| Lost Time Adjust (s) |  | 0.0 |  |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time (s) |  | 6.1 |  |  | 6.1 |  |  | 5.1 |  | 5.1 | 5.1 |  |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode |  | C-Min |  | C-Min | C-Min |  | None | None |  | None | None |  |
| Act Effct Green (s) |  | 80.9 |  |  | 80.9 |  |  | 27.9 |  | 27.9 | 27.9 |  |


| Lane Group | 4 EBL | $\rightarrow$ |  |  | + WBT |  | NBL | 4 NBT | NBR | SBL | $\downarrow$ SBT | d SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Actuated g/C Ratio |  | 0.67 |  |  | 0.67 |  |  | 0.23 |  | 0.23 | 0.23 |  |
| v/c Ratio |  | 0.23 |  |  | 0.43 |  |  | 0.23 |  | 0.85 | 0.15 |  |
| Control Delay |  | 8.7 |  |  | 10.8 |  |  | 21.8 |  | 67.7 | 9.5 |  |
| Queue Delay |  | 0.0 |  |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay |  | 8.7 |  |  | 10.8 |  |  | 21.8 |  | 67.7 | 9.5 |  |
| LOS |  | A |  |  | B |  |  | C |  | E | A |  |
| Approach Delay |  | 8.7 |  |  | 10.8 |  |  | 21.8 |  |  | 56.5 |  |
| Approach LOS |  | A |  |  | B |  |  | C |  |  | E |  |
| Queue Length 50th (ft) |  | 75 |  |  | 160 |  |  | 32 |  | 190 | 2 |  |
| Queue Length 95th (ft) |  | 125 |  |  | 305 |  |  | 68 |  | 261 | 33 |  |
| Internal Link Dist (ti) |  | 253 |  |  | 331 |  |  | 280 |  |  | 526 |  |
| Turn Bay Length (tt) |  |  |  |  |  |  |  |  |  |  |  |  |
| Base Capacity (vph) |  | 2252 |  |  | 2148 |  |  | 468 |  | 366 | 467 |  |
| Starvation Cap Reductn |  | 0 |  |  | 0 |  |  | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn |  | 0 |  |  | 0 |  |  | 0 |  | 0 | 0 |  |
| Storage Cap Reductn |  | 0 |  |  | 0 |  |  |  |  | 0 | 0 |  |
| Reduced v/c Ratio |  | 0.23 |  |  | 0.43 |  |  | 0.19 |  | 0.70 | 0.13 |  |

Intersection Summary
Area Type: Other

Cycle Length: 120
Actuated Cycle Length: 120
Offset: 27 (23\%), Referenced to phase 2:EBWB, Start of Yellow
Natural Cycle: 45
Control Type: Actuated-Coordinated
Maximum vic Ratio: 0.85
Intersection Signal Delay: 18.6
Intersection Capacity Utilization 53.4\%
Intersection LOS: B
ICU Level of Sevice A
Analysis Period (min) 15
Splits and Phases: 1: LAFAYETTE STREET \& U.S. ROUTE 1


|  | $\rightarrow$ | $\geqslant$ | 1 |  | 4 | 7 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |  |
| Lane Configurations | 中* |  |  | $4{ }^{4}$ | \% |  |  |
| Traffic Volume (vph) | 742 | 9 | 14 | 809 | 11 | 58 |  |
| Future Volume (vph) | 742 | 9 | 14 | 809 | 11 | 58 |  |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |  |
| Lane Width (t) | 11 | 11 | 11 | 11 | 14 | 14 |  |
| Grade (\%) | 0\% |  |  | 0\% | 0\% |  |  |
| Storage Length ( t ) |  | 0 | 0 |  | 0 | 0 |  |
| Storage Lanes |  | 0 | 0 |  | 1 | 0 |  |
| Taper Length (tt) |  |  | 25 |  | 25 |  |  |
| Lane Util. Factor | 0.95 | 0.95 | 0.95 | 0.95 | 1.00 | 1.00 |  |
| Ped Bike Factor |  |  |  |  | 1.00 |  |  |
| Frt | 0.998 |  |  |  | 0.886 |  |  |
| Flt Protected |  |  |  | 0.999 | 0.992 |  |  |
| Satd. Flow (prot) | 3414 | 0 | 0 | 3418 | 1746 | 0 |  |
| Flt Permitted |  |  |  | 0.936 | 0.992 |  |  |
| Satd. Flow (perm) | 3414 | 0 | 0 | 3202 | 1741 | 0 |  |
| Right Turn on Red |  | Yes |  |  |  | Yes |  |
| Satd. Flow (RTOR) | 2 |  |  |  | 60 |  |  |
| Link Speed (mph) | 30 |  |  | 30 | 30 |  |  |
| Link Distance (tt) | 258 |  |  | 323 | 552 |  |  |
| Travel Time (s) | 5.9 |  |  | 7.3 | 12.5 |  |  |
| Confl. Peds. (\#hr) |  |  |  |  | 11 |  |  |
| Confl. Bikes (\#hr) |  |  |  |  |  |  |  |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |  |
| Growth Factor | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% |  |
| Heavy Vehicles (\%) | 2\% | $2 \%$ | 2\% | 2\% | 2\% | 2\% |  |
| Bus Blockages (\#hr) | 0 | - | 0 | 0 | 0 | 0 |  |
| Parking (\#/hr) |  |  |  |  |  |  |  |
| Mid-Block Traffic (\%) | 0\% |  |  | 0\% | 0\% |  |  |
| Adj. Flow (vph) | 773 | 9 | 15 | 843 | 11 | 60 |  |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 782 | 0 | 0 | 858 | 71 | 0 |  |
| Turn Type | NA |  | Perm | NA | Prot |  |  |
| Protected Phases | 6 |  |  | 2 | 4 |  |  |
| Permitted Phases |  |  | 2 |  |  |  |  |
| Detector Phase | 6 |  | 2 | 2 | 4 |  |  |
| Switch Phase |  |  |  |  |  |  |  |
| Minimum Initial (s) | 15.0 |  | 15.0 | 15.0 | 7.0 |  |  |
| Minimum Split (s) | 20.8 |  | 20.8 | 20.8 | 12.0 |  |  |
| Total Split (s) | 90.0 |  | 90.0 | 90.0 | 30.0 |  |  |
| Total Split (\%) | 75.0\% |  | 75.0\% | 75.0\% | 25.0\% |  |  |
| Yellow Time (s) | 3.8 |  | 3.8 | 3.8 | 3.0 |  |  |
| All-Red Time (s) | 2.0 |  | 2.0 | 2.0 | 2.0 |  |  |
| Lost Time Adjust (s) | 0.0 |  |  | 0.0 | 0.0 |  |  |
| Total Lost Time (s) | 5.8 |  |  | 5.8 | 5.0 |  |  |
| Lead/Lag |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| Recall Mode | C-Min |  | C-Min | C-Min | None |  |  |
| Act Effct Green (s) | 105.3 |  |  | 105.3 | 7.5 |  |  |
| $01 / 05 / 2022$ <br> HARDESTY \& HANOVER | C-STC |  |  |  |  |  | Synchro 10 Report Page 3 |



Splits and Phases: 2: NORTH STATE STREET \& U.S. ROUTE 1


|  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


| Lane | NBLn1 | NBLn2 | EBLn1 | WBLn1 | SBLn1 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $100 \%$ | $0 \%$ | $100 \%$ | $0 \%$ | $0 \%$ |
| Vol Thru, \% | $0 \%$ | $100 \%$ | $0 \%$ | $12 \%$ | $0 \%$ |
| Vol Right, \% | $0 \%$ | $0 \%$ | $0 \%$ | $88 \%$ | $100 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 18 | 32 | 5 | 49 | 7 |
| LT Vol | 18 | 0 | 5 | 0 | 0 |
| Through Vol | 0 | 32 | 0 | 6 | 0 |
| RT Vol | 0 | 0 | 0 | 43 | 7 |
| Lane Flow Rate | 21 | 37 | 6 | 57 | 8 |
| Geometry Grp | 7 | 7 | 2 | 2 | 5 |
| Degree of Util (X) | 0.03 | 0.048 | 0.007 | 0.056 | 0.008 |
| Departure Headway (Hd) | 5.149 | 4.648 | 4.291 | 3.524 | 3.587 |
| Convergence, YN | Yes | Yes | Yes | Yes | Yes |
| Cap | 698 | 772 | 828 | 1007 | 994 |
| Service Time | 2.863 | 2.363 | 2.346 | 1.576 | 1.622 |
| HCM Lane V/C Ratio | 0.03 | 0.048 | 0.007 | 0.057 | 0.008 |
| HCM Control Delay | 8 | 7.6 | 7.4 | 6.8 | 6.7 |
| HCM Lane LOS | A | A | A | A | A |
| HCM 95th-ile Q | 0.1 | 0.2 | 0 | 0.2 | 0 |


| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 5.3 |  |  |  |  |  |
| Movement E | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  |  | t |  |  | F |
| Traffic Vol, veh/h | 0 | 0 | 9 | 15 | 0 | 40 |
| Future Vol, veh/h | 0 | 0 | 9 | 15 | 0 | 40 |
| Conflicting Peds, \#hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | - | 0 |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 0 | 10 | 16 | 0 | 43 |
| Major/Minor |  |  | Major2 |  | Minor2 |  |
| Conflicting Flow All |  |  | - | 0 | - | 18 |
| Stage 1 |  |  | - | - | - | - |
| Stage 2 |  |  | - | - | - | - |
| Critical Hdwy |  |  | - | - | - | 6.22 |
| Critical Hdwy Stg 1 |  |  | - | - | - | - |
| Critical Hdwy Stg 2 |  |  | - | - | - | - |
| Follow-up Hdwy |  |  | - | - | - | 3.318 |
| Pot Cap-1 Maneuver |  |  | - | - | 0 | 1061 |
| Stage 1 |  |  | - | - | 0 | - |
| Stage 2 |  |  | - | - | 0 | - |
| Platoon blocked, \% |  |  | - | - |  |  |
| Mov Cap-1 Maneuver |  |  | - | - | - | 1061 |
| Mov Cap-2 Maneuver |  |  | - | - | - | - |
| Stage 1 |  |  | - | - | - | - |
| Stage 2 |  |  | - | - | - | - |


| Approach | WB | SB |
| :--- | ---: | ---: |
| HCM Control Delay, s | 0 | 8.5 |
| HCM LOS |  | A |


| Minor Lane/Major Mvmi | WBT | WBR SBLn1 |
| :--- | ---: | ---: |
| Capacity (veh/h) | - | -1061 |
| HCM Lane V/C Ratio | - | -0.041 |
| HCM Control Delay (s) | - | -8.5 |
| HCM Lane LOS | - | A |
| HCM 95th \%tile Q(veh) | - | - |
| A | 0.1 |  |


| Lane Group | A EBL | $\begin{aligned} & \rightarrow \\ & \text { EBT } \end{aligned}$ | EBR | WBL | + | WBR | 4 NBL | ¢ <br> NBT | NBR | - SBL | 1 SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  | \$\% |  |  | $\uparrow$ |  |  | * |  | \% | \% |  |
| Traffic Volume (vph) | 0 | 751 | 10 | 3 | 618 | 0 | 58 | 0 | 57 | 181 | 2 | 32 |
| Future Volume (vph) | 0 | 751 | 10 | 3 | 618 | 0 | 58 | 0 | 57 | 181 | 2 | 32 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Width (ft) | 10 | 10 | 10 | 10 | 10 | 10 | 15 | 15 | 15 | 12 | 11 | 11 |
| Grade (\%) |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Storage Length (t) | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 200 |
| Storage Lanes | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 1 |  | 1 |
| Taper Length ( ft ) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Ped Bike Factor |  | 1.00 |  |  | 1.00 |  |  | 0.98 |  | 0.99 | 0.97 |  |
| Frt |  | 0.998 |  |  |  |  |  | 0.933 |  |  | 0.860 |  |
| Fit Protected |  |  |  |  |  |  |  | 0.975 |  | 0.950 |  |  |
| Satd. Flow (prot) | 0 | 3294 | 0 | 0 | 1739 | 0 | 0 | 1841 | 0 | 1770 | 1506 | 0 |
| Flt Permitted |  |  |  |  | 0.996 |  |  | 0.825 |  | 0.629 |  |  |
| Satd. Flow (perm) | 0 | 3294 | 0 | 0 | 1732 | 0 | 0 | 1545 | 0 | 1162 | 1506 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 2 |  |  |  |  |  | 38 |  |  | 44 |  |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 30 |  |  | 30 |  |
| Link Distance ( ft ) |  | 333 |  |  | 411 |  |  | 360 |  |  | 606 |  |
| Travel Time (s) |  | 7.6 |  |  | 9.3 |  |  | 8.2 |  |  | 13.8 |  |
| Confl. Peds. (\#/hr) | 18 |  | 12 | 12 |  | 18 | 10 |  | 7 | 7 |  | 10 |
| Confl. Bikes (\#/hr) |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour Factor | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 |
| Growth Factor | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% |
| Heavy Vehicles (\%) | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% |
| Bus Blockages (\#/hr) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Parking (\#/hr) |  |  |  |  |  |  |  |  |  |  |  |  |
| Mid-Block Traffic (\%) |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Adj. Flow (vph) | 0 | 1029 | 14 | 4 | 847 | 0 | 79 | 0 | 78 | 248 | 3 | 44 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 1043 | 0 | 0 | 851 | 0 | 0 | 157 | 0 | 248 | 47 | 0 |
| Turn Type |  | NA |  | Perm | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases |  | 2 |  |  | 2 |  |  | 4 |  |  | 4 |  |
| Permitted Phases |  |  |  | 2 |  |  | 4 |  |  | 4 |  |  |
| Detector Phase |  | 2 |  | 2 | 2 |  | 4 | 4 |  | 4 | 4 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial $(\mathbf{s}$ ) |  | 15.0 |  | 15.0 | 15.0 |  | 7.0 | 7.0 |  | 7.0 | 7.0 |  |
| Minimum Split (s) |  | 21.1 |  | 21.1 | 21.1 |  | 12.1 | 12.1 |  | 12.1 | 12.1 |  |
| Total Split (s) |  | 88.0 |  | 88.0 | 88.0 |  | 32.0 | 32.0 |  | 32.0 | 32.0 |  |
| Total Split (\%) |  | 73.3\% |  | 73.3\% | 73.3\% |  | 26.7\% | 26.7\% |  | 26.7\% | 26.7\% |  |
| Yellow Time (s) |  | 4.1 |  | 4.1 | 4.1 |  | 3.6 | 3.6 |  | 3.6 | 3.6 |  |
| All-Red Time (s) |  | 2.0 |  | 2.0 | 2.0 |  | 1.5 | 1.5 |  | 1.5 | 1.5 |  |
| Lost Time Adjust (s) |  | 0.0 |  |  | 0.0 |  |  | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time (s) |  | 6.1 |  |  | 6.1 |  |  | 5.1 |  | 5.1 | 5.1 |  |
| Lead/Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode |  | C-Min |  | C-Min | C-Min |  | None | None |  | None | None |  |
| Act Effct Green (s) |  | 77.5 |  |  | 77.5 |  |  | 31.3 |  | 31.3 | 31.3 |  |



Splits and Phases: 1: LAFAYETTE STREET \& U.S. ROUTE 1


|  | , |  |  |  |  | 1 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | 中t |  |  | * ${ }^{+}$ | H |  |
| Traffic Volume (vph) | 981 | 27 | 22 | 652 | 2 | 139 |
| Future Volume (vph) | 981 | 27 | 22 | 652 | 2 | 139 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Width (ft) | 11 | 11 | 11 | 11 | 14 | 14 |
| Grade (\%) | 0\% |  |  | 0\% | 0\% |  |
| Storage Length (ft) |  | 0 | 0 |  | 0 | 0 |
| Storage Lanes |  | 0 | 0 |  | 1 | 0 |
| Taper Length ( ft ) |  |  | 25 |  | 25 |  |
| Lane Util. Factor | 0.95 | 0.95 | 0.95 | 0.95 | 1.00 | 1.00 |
| Ped Bike Factor | 1.00 |  |  | 1.00 | 1.00 |  |
| Fit | 0.996 |  |  |  | 0.867 |  |
| Fit Protected |  |  |  | 0.998 | 0.999 |  |
| Satd. Flow (prot) | 3405 | 0 | 0 | 3414 | 1721 | 0 |
| Flt Permitted |  |  |  | 0.883 | 0.999 |  |
| Satd. Flow (perm) | 3405 | 0 | 0 | 3021 | 1721 | 0 |
| Right Turn on Red |  | Yes |  |  |  | Yes |
| Satd. Flow (RTOR) | 5 |  |  |  | 130 |  |
| Link Speed (mph) | 30 |  |  | 30 | 30 |  |
| Link Distance ( ft ) | 258 |  |  | 323 | 552 |  |
| Travel Time (s) | 5.9 |  |  | 7.3 | 12.5 |  |
| Confl. Peds. (\#/hr) |  | 2 | 2 |  | 3 |  |
| Confl. Bikes (\#/hr) |  |  |  |  |  |  |
| Peak Hour Factor | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 | 0.87 |
| Growth Factor | 100\% | 100\% | 100\% | 100\% | 100\% | 100\% |
| Heavy Vehicles (\%) | 2\% | 2\% | 2\% | 2\% | 2\% | 2\% |
| Bus Blockages (\#/hr) | 0 | 0 | 0 | 0 | 0 | 0 |
| Parking (\#/hr) |  |  |  |  |  |  |
| Mid-Block Traffic (\%) | 0\% |  |  | 0\% | 0\% |  |
| Adj. Flow (vph) | 1128 | 31 | 25 | 749 | 2 | 160 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |
| Lane Group Flow (vph) | 1159 | 0 | 0 | 774 | 162 | 0 |
| Turn Type | NA |  | Perm | NA | Prot |  |
| Protected Phases | 6 |  |  | 2 | 4 |  |
| Permitted Phases |  |  | 2 |  |  |  |
| Detector Phase | 6 |  | 2 | 2 | 4 |  |
| Switch Phase |  |  |  |  |  |  |
| Minimum Initial (s) | 15.0 |  | 15.0 | 15.0 | 7.0 |  |
| Minimum Split (s) | 20.8 |  | 20.8 | 20.8 | 12.0 |  |
| Total Split (s) | 90.0 |  | 90.0 | 90.0 | 30.0 |  |
| Total Split (\%) | 75.0\% |  | 75.0\% | 75.0\% | 25.0\% |  |
| Yellow Time (s) | 3.8 |  | 3.8 | 3.8 | 3.0 |  |
| All-Red Time (s) | 2.0 |  | 2.0 | 2.0 | 2.0 |  |
| Lost Time Adjust (s) | 0.0 |  |  | 0.0 | 0.0 |  |
| Total Lost Time (s) | 5.8 |  |  | 5.8 | 5.0 |  |
| Lead/Lag |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |
| Recall Mode | C-Min |  | C-Min | C-Min | None |  |
| Act Effct Green (s) | 100.3 |  |  | 100.3 | 8.9 |  |



Splits and Phases: 2: NORTH STATE STREET \& U.S. ROUTE 1


| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Intersection Delay, s/veh Intersection LOS | $\begin{array}{r} 7.9 \\ \text { A } \end{array}$ |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 4 |  |  |  | $\uparrow$ |  | 4 | 4 |  |  |  | 「 |
| Traffic Vol, veh/h | 10 | 0 | 0 | 0 | 12 | 39 | 65 | 66 | 0 | 0 | 0 | 15 |
| Future Vol, veh/h | 10 | 0 | 0 | 0 | 12 | 39 | 65 | 66 | 0 | 0 | 0 | 15 |
| Peak Hour Factor | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 13 | 0 | 0 | 0 | 15 | 49 | 81 | 83 | 0 | 0 | 0 | 19 |
| Number of Lanes | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 0 | 0 | 1 |
| Approach | EB |  |  |  | WB |  | NB |  |  |  |  | SB |
| Opposing Approach | WB |  |  |  | EB |  | SB |  |  |  |  | NB |
| Opposing Lanes | 1 |  |  |  | 1 |  | 1 |  |  |  |  | 2 |
| Conflicting Approach Left | SB |  |  |  | NB |  | EB |  |  |  |  | WB |
| Conflicting Lanes Left | 1 |  |  |  | 2 |  | 1 |  |  |  |  | 1 |
| Conflicting Approach Right | NB |  |  |  | SB |  | WB |  |  |  |  | EB |
| Conflicting Lanes Right | 2 |  |  |  | 1 |  | 1 |  |  |  |  | 1 |
| HCM Control Delay | 7.7 |  |  |  | 7.2 |  | 8.3 |  |  |  |  | 6.9 |
| HCM LOS | A |  |  |  | A |  | A |  |  |  |  | A |


| Lane | NBLn1 | NBLn2 | EBLn1 | WBLn1 | SBLn1 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $100 \%$ | $0 \%$ | $100 \%$ | $0 \%$ | $0 \%$ |
| Vol Thru, \% | $0 \%$ | $100 \%$ | $0 \%$ | $24 \%$ | $0 \%$ |
| Vol Right, \% | $0 \%$ | $0 \%$ | $0 \%$ | $76 \%$ | $100 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 65 | 66 | 10 | 51 | 15 |
| LT Vol | 65 | 0 | 10 | 0 | 0 |
| Through Vol | 0 | 66 | 0 | 12 | 0 |
| RT Vol | 0 | 0 | 0 | 39 | 15 |
| Lane Flow Rate | 81 | 82 | 12 | 64 | 19 |
| Geometry Grp | 7 | 7 | 2 | 2 | 5 |
| Degree of Util (X) | 0.117 | 0.107 | 0.016 | 0.07 | 0.019 |
| Departure Headway (Hd) | 5.178 | 4.678 | 4.641 | 3.932 | 3.686 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes |
| Cap | 691 | 765 | 775 | 917 | 952 |
| Service Time | 2.915 | 2.414 | 2.644 | 1.932 | 1.783 |
| HCM Lane VIC Ratio | 0.117 | 0.107 | 0.015 | 0.07 | 0.02 |
| HCM Control Delay | 8.6 | 8 | 7.7 | 7.2 | 6.9 |
| HCM Lane LOS | A | A | A | A | A |
| HCM 95th-tile Q | 0.4 | 0.4 | 0 | 0.2 | 0.1 |


| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 2.8 |  |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  |  | A |  |  | \% |
| Traffic Vol, veh/h | 0 | 0 | 21 | 40 | 0 | 30 |
| Future Vol, veh/h | 0 | 0 | 21 | 40 | 0 | 30 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control F | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | - | 0 |
| Veh in Median Storage, \# | \# | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mumt Flow | 0 | 0 | 23 | 43 | 0 | 33 |


| MajorMMinor | Major2 | Minor2 |  |  |
| :--- | :---: | :---: | :---: | :---: |
| Conflicting Flow All | - | 0 | - | 45 |
| $\quad$ Stage 1 | - | - | - | - |
| Stage 2 | - | - | - | - |
| Critical Hdwy | - | - | - | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - |
| Critical Hdwy Stg 2 | - | - | - | - |
| Follow-up Hdwy | - | - | -3.318 |  |
| Pot Cap-1 Maneuver | - | - | 0 | 1025 |
| $\quad$ Stage 1 | - | - | 0 | - |
| Stage 2 | - | - | 0 | - |
| Platoon blocked, \% | - | - |  |  |
| Mov Cap-1 Maneuver | - | - | - | 1025 |
| Mov Cap-2 Manevver | - | - | - | - |
| Stage 1 | - | - | - | - |
| Stage 2 | - | - | - | - |


| Approach | WB | SB |
| :--- | ---: | ---: |
| HCM Control Delay, s | 0 | 8.6 |
| HCM LOS |  | A |

Minor Lane/Major Mvmt WBT WBR SBLn1

| Capacity (veh/h) | - | -1025 |
| :--- | :--- | ---: |
| HCM Lane VIC Ratio | - | -0.032 |
| HCM Control Delay (s) | - | -8.6 |
| HCM Lane LSS | - | - |
| HCM 95th \%tile Q(veh) | - | - |
|  | 0.1 |  |

# STORMWATER MANAGEMENT REPORT 

819, 825, 827 \& 831 EAST MAIN STREET + 15, 27 \& 29, LAFAYETTE STREET, STAMFORD, CT<br>(1.15 acres)

prepared for 819 EAST MAIN STREET, LLC

Date: 2/03/2022


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| :--- | :--- |
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Appendix I: Waiver Covering Storm Sewer Connection
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## Introduction

The property owner is proposing a re-development of seven (7) contiguous parcels at 821, 825, 827, 831 East Main Street and 15, 27, 29 Lafayette Street for the construction of a five (5) story mixed-use building. The re-development includes one hundred thirty (130) dwelling units, 2,950 square feet (SF) of commercial area, off-street parking and associated site utilities. A total of one hundred fifty (150) parking spaces are proposed with on-site at-grade parking and within a parking garage below the first floor of the proposed building. Streetscape improvements are proposed along the street frontage of East Main Street, Lafayette Street, and North State Street.

The total project site area is 1.15 acres. The project is proposed to be re-zoned to MX-D. The contiguous parcels shall be consolidated into one corner lot. The project lot is bounded by East Main Street to the north, Lafayette Street to the west, and North State Street to the south. The commercial property of 835 East Main Street abuts the property to the east. The seven existing parcels are developed with buildings, surface parking, various hardscapes and various entrance drives on East Main Street, Lafayette Street, and North State Street. The properties are served by public water and City sewer.

Based on a review of the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map Community Panel No. 09001C0517G map effective July 8, 2013, the re-development site is not located within a Flood Hazard Area. The site is tributary to the Southwest Shoreline basin and ultimately to Long Island Sound. The water quality classification for proximate surface water and groundwater is SB and GB, respectively, per the Connecticut Department of Energy \& Environmental Protection. The Natural Resources Conservation Service (NRCS) information indicates the soils are in the D Hydrologic Soil Group. Refer to Appendix A for the NRCS web soil survey and the FEMA Flood Insurance map.

Reference is made to the project's Site Plan drawing sheets C-1, C-2, C-3, C-4, C-5, C-6, C-7, C-8, and C-9 prepared by DiMarzo \& Bereczky, Inc. dated 2/03/2022. Our firm also prepared a Property and Topographic Survey dated 12/14/2021 and a Zoning Location Survey dated 2/03/2022. The anticipated construction schedule is $3 / 1 / 2022$ to $2 / 28 / 2023$.

## Existing Conditions

The existing conditions of the project site consists of five (5) buildings, asphalt pavement, gravel pavement, lawn and planting beds. Stormwater runoff flows from the site in three directions. Runoff from the vast majority of the property flows overland to the south, and its tributary to an existing storm drain at the intersection of North State Street and Lafayette Street. Runoff from a
small on-site area abutting East Main Street flows to the north. A small runoff area along the eastern property line flows to the abutting commercial property ( BevMax ) to an existing private stormwater management system.

The USDA Natural Resources Conservation Service's Websoil Survey indicates the soils on the property are labeled as Urban Land within Hydrologic Soils Group D. Deep test pits and borehole infiltration tests were performed on-site to identify any sub-grade restrictive soil conditions (ledge, groundwater, etc.). A total of six (6) deep test pits were performed. No groundwater, mottling or ledge were encountered in any of the test pits. A well-draining sand and gravel was found under a fill layer in the six deep test pits. Three borehole infiltration tests were conducted to determine if the insitu soil can adequately infiltrate stormwater. The field infiltration rates were 5.1, 4.6 and 8.6 inches per hour. Test pit and infiltration test results can be reviewed on site plan sheet C-5. The locations are shown on utility plan C-2.

The current onsite impervious coverage is 39,380 square feet (SF). Runoff for the on-site drainage analysis is calculated using the computer program HydroCAD version 10.0 produced by HydroCAD Software Solutions, LLC. The 24-hour design storms analyzed include the 1, 2, 5, 10, 25, and 50 year storm events, with rainfall depths of $2.96,3.58,4.60,5.45,6.61$ and 7.47 inches respectively. The method used is USDA, NRCS TR-55. The rainfall information is provided by NOAA Atlas 14.

Refer to Appendix B for the Onsite HydroCAD report. The existing drainage basin areas, curve numbers, time of concentrations and 25-year peak flow rates are summarized as follows:

| Existing Conditions - Onsite |  |  |  |  |  |  |  |
| :---: | :---: | :---: | ---: | ---: | ---: | ---: | :---: |
| Basin | Area <br> $($ acres | Sub-Basin | Area <br> $($ acres $)$ | CN | $\mathrm{Tc}(\mathrm{min})$ | $\mathrm{Q}_{25}(\mathrm{cfs})$ |  |
| South | 1.104 | South | 0.744 | 95.25 | 5.0 | 7.30 |  |
| East | 0.031 | East | 0.031 | 84.00 | 5.0 | 0.18 |  |
| North | 0.018 | North | 0.018 | 97.09 | 5.0 | 0.12 |  |

In preparing the offsite drainage study, DiMarzo \& Bereczky conducted site visits, surveyed portions of the existing City/State owned drainage system and used record information obtained from the City Engineering Bureau.

The offsite watershed for both the onsite southern and eastern basins consists of an urban land use, and it is over 19 acres in size. The outfall point of analysis is at the 24 " diameter reinforced
concrete pipe (RCP) storm sewer within South State Street just east of the on-ramp spur to I-95 northbound. Under existing conditions, $97.7 \%$ of the onsite project area is tributary to this system. Refer to the enclosed Watershed Drainage Basin Map in Appendix E.

A hydraulic grade line (HGL) analysis model has been created using StormCAD Connect Edition Update 3 by Bentley Systems for the offsite storm sewer network. The analysis uses a storm event recurrence interval of 25 years based on the NOAA rainfall information. A starting tail-water elevation of 6.4 NAVD'88 is applied at the outfall connection. This tail-water considers the 24 " pipe in South State Street is running at a half-full depth.

The HGL model results in ten (10) of fifteen (15) manholes overflowing within the South State Street, North State Street and Lafayette Street stormwater sewer system. Twelve (12) of twenty-two (22) catch basins are overflowing. Refer to Appendix E for further detail.

## Proposed Conditions

The proposed improvements are classified as a development project with more than a $1 / 2$ acre of disturbance. Thus, the project must comply with Standards 1 through 5 of the Stamford Drainage Manual dated 6/10/2020. Under proposed conditions, the net increase in onsite impervious coverage is $6,093 \mathrm{SF}$. The proposed drainage design is focused on providing pollutant reduction and reducing peak flow rates to the offsite watershed. In addition, the design shall match or decrease both peak flow rates and volume of runoff in the northern basin. The northern basin is tributary to the East Main Street storm sewer. The proposed drainage basin onsite areas, curve numbers, time of concentrations and 25 -year peak flow rates are summarized as follows:

| Proposed Conditions - Onsite |  |  |  |  |  |  |  |
| :---: | :---: | :---: | ---: | ---: | ---: | ---: | :---: |
| Basin | Area <br> (acres) | Sub-Basin | Area <br> (acres) | CN | $\mathrm{Tc}(\mathrm{min})$ | $\mathrm{Q}_{25}(\mathrm{cfs})$ |  |
|  | 1.107 | South -1 | 0.267 | 96.29 | 5.0 | 1.78 |  |
|  |  | 0.203 | 97.28 | 5.0 | 1.36 |  |  |
|  | South Bypass | 0.327 | 97.46 | 5.0 | 4.27 |  |  |
| East | 0.029 | East | 0.029 | 84.00 | 5.0 | 0.16 |  |
| North | 0.018 | North | 0.018 | 87.85 | 5.0 | 0.11 |  |

The runoff from the proposed at-grade parking area shall be collected by an on-site stormwater management system of catch basins and a trench drain. The collected stormwaters shall be conveyed to two separate underground infiltration galleries. The gallery system labeled as BMP-1
consists of twenty (20) four-foot (4') high precast concrete galleries units and crushed stone. The second gallery system is labeled BMP-2, and it consists of sixteen (16) four-foot (4') high precast concrete galleries units and crushed stone. Both infiltration systems are metered with outlet devices within separate downstream manhole connections. Each manhole has a 4' wide weir overflow and two (2) six inch (6") vertical orifices within the weir wall. The orifice outlets are at a lower elevation than the overflow weir. Further downstream, a new manhole and a twelve-inch (12") storm drain are proposed within North State Street. They will connect to the City's existing storm sewer system at the intersection of Lafayette Street and North State Street.

The following table depicts existing and proposed peak rates of runoff and the hydraulic volume comparisons for the three onsite drainage basins and their respective points of concern. Additional information may be found in the HydroCAD report in Appendix B.

| South Basin |  |  |  |
| :---: | :---: | :---: | :---: |
| Storm Event (yrs) | Existing Peak Rate of <br> Runoff (cfs) | Proposed Peak Rate of <br> Runoff (cfs) | $\%$ change |
| 1 | 3.09 | 1.88 | $-39.2 \%$ |
| 2 | 3.82 | 2.28 | $-40.3 \%$ |
| 5 | 5.00 | 2.95 | $-41.0 \%$ |
| 10 | 5.97 | 4.19 | $-29.8 \%$ |
| 25 | 7.30 | 7.12 | $-2.5 \%$ |
| 50 | 8.28 | 7.97 | $-3.7 \%$ |


| South Basin |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Hydraulic Volume (cubic feet) |  | Hydraulic Volume (acre feet) |  |  |  |  |
| Storm <br> Event <br> (yrs) | Existing | Proposed | \% change | Exsiting | Proposed | \% change |  |
| 1 | 9752 | 6918 | -29.1 | 0.224 | 0.159 | -29.1 |  |
| 2 | 12188 | 9385 | -23.0 | 0.280 | 0.215 | -23.0 |  |
| 5 | 16222 | 13454 | -17.1 | 0.372 | 0.309 | -17.1 |  |
| 10 | 19596 | 16851 | -14.0 | 0.450 | 0.387 | -14.0 |  |
| 25 | 24213 | 21490 | -11.2 | 0.556 | 0.493 | -11.2 |  |
| 50 | 27641 | 24933 | -9.8 | 0.635 | 0.572 | -9.8 |  |


| East Basin |  |  |  |
| :---: | :---: | :---: | :---: |
| Storm Event (yrs) | Existing Peak Rate of <br> Runoff (cfs) | Proposed Peak Rate of <br> Runoff (cfs) | $\%$ change |
| 1 | 0.06 | 0.05 | $-16.7 \%$ |
| 2 | 0.08 | 0.07 | $-12.5 \%$ |
| 5 | 0.11 | 0.10 | $-9.1 \%$ |
| 10 | 0.14 | 0.13 | $-7.1 \%$ |
| 25 | 0.18 | 0.16 | $-11.1 \%$ |
| 50 | 0.21 | 0.19 | $-9.5 \%$ |


| East Basin |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Hydraulic Volume (cubic feet) |  | Hydraulic Volume (acre feet) |  |  |  |  |
| Storm <br> Event <br> (yrs) | Existing | Proposed | \% change | Exsiting | Proposed | \% change |  |
| 1 | 167 | 154 | -7.8 | 0.004 | 0.004 | -7.8 |  |
| 2 | 226 | 209 | -7.5 | 0.005 | 0.005 | -7.5 |  |
| 5 | 328 | 303 | -7.6 | 0.008 | 0.007 | -7.6 |  |
| 10 | 415 | 384 | -7.5 | 0.010 | 0.009 | -7.5 |  |
| 25 | 538 | 497 | -7.6 | 0.012 | 0.011 | -7.6 |  |
| 50 | 630 | 582 | -7.6 | 0.014 | 0.013 | -7.6 |  |


| North Basin |  |  |  |
| :---: | :---: | :---: | :---: |
| Storm Event (yrs) | Existing Peak Rate of <br> Runoff (cfs) | Proposed Peak Rate of <br> Runoff (cfs) | \% change |
| 1 | 0.05 | 0.04 | $-20.0 \%$ |
| 2 | 0.07 | 0.05 | $-28.6 \%$ |
| 5 | 0.09 | 0.07 | $-22.2 \%$ |
| 10 | 0.10 | 0.09 | $-10.0 \%$ |
| 25 | 0.12 | 0.11 | $-8.3 \%$ |
| 50 | 0.14 | 0.12 | $-14.3 \%$ |


| North Basin |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Hydraulic Volume (cubic feet) |  | Hydraulic Volume (acre feet) |  |  |  |  |
| Storm <br> Event <br> (yrs) | Existing | Proposed | \% change | Exsiting | Proposed | \% change |  |
| 1 | 175 | 114 | -34.9 | 0.004 | 0.003 | -34.9 |  |
| 2 | 216 | 150 | -30.6 | 0.005 | 0.003 | -30.6 |  |
| 5 | 284 | 211 | -25.7 | 0.007 | 0.005 | -25.7 |  |
| 10 | 341 | 263 | -22.9 | 0.008 | 0.006 | -22.9 |  |
| 25 | 418 | 335 | -19.9 | 0.010 | 0.008 | -19.9 |  |
| 50 | 475 | 389 | -18.1 | 0.011 | 0.009 | -18.1 |  |

Under post construction conditions, the calculations in the HGL analysis indicate that the City's existing system will continue to operate under current conditions. There are either decreases or no changes in the HGL elevations. Similar to the existing condition results, ten (10) of fifteen (15) manholes are overflowing and twelve (12) of twenty-two (22) catch basins are overflowing. The following chart compares the existing and proposed HGL elevations for the storm sewer network to the outfall.

| Offsite Storm Sewer System Comparison <br> from South State Street to the upstream networks of North State St and Lafayette Street |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Hydraulic Grade Line (H.G.L.) Elevation Change in Feet within the Storm Main (NAVD'88) |  |  |  |  |  |
| Pipe | Structure <br> (Upstream) | Rim Elevation | H.G.L. <br> (Ex. Conditions) | H.G.L. <br> (Pr. Conditions) | Change (ft) |
| P-2 | MH-2 | 16.64 | 23.46 | 23.38 | -0.08 |
| P-3 | MH-3 | 15.50 | 21.29 | 21.29 | 0.00 |
| P-4 | MH-4 | 13.70 | 23.55 | 23.54 | -0.01 |
| P-5 | MH-5 | 14.41 | 14.12 | 14.12 | 0.00 |
| P-6 | MH-6 | 15.30 | 14.41 | 14.41 | 0.00 |
| P-7 | MH-7 | 11.75 | 30.45 | 30.38 | -0.07 |
| P-8 | MH-8 | 12.39 | 11.95 | 11.95 | 0.00 |
| P-9 | MH-9 | 11.35 | 24.43 | 24.43 | 0.00 |
| P-10 | MH-10 | 11.46 | 24.00 | 24.00 | 0.00 |
| P-11 | MH-11 | 14.20 | 11.69 | 11.69 | 0.00 |
| P-12 | MH-12 | 13.43 | 31.72 | 31.72 | 0.00 |
| P-13 | MH-13 | 14.80 | 19.44 | 19.44 | 0.00 |
| P-14 | MH-14 | 17.83 | 20.98 | 20.98 | 0.00 |
| P-15 | MH-15 | 19.23 | 20.18 | 20.18 | 0.00 |
| P-16 | MH-16 | 20.20 | 19.96 | 19.96 | 0.00 |

Note: Pr. Conditions represent the proposed development with infiltration galleries.

The offsite watershed flow of runoff at the outfall decreases from 83.61 cfs to 83.42 cfs . Overall, the analysis shows a proposed peak flow rate of runoff decrease of $0.2 \%$ in the 25 -year storm at the 24 " rcp outfall.

Per our hydraulic analysis, the proposed pipe network from the development to the downstream connection at the Lafayette and North State Street intersection is sized to ensure adequate capacity to convey stormwater runoff from the 25 -year storm event. Refer to Appendix D for conveyance calculations.

The project proposes to connect to public utilities such as the public sanitary sewer, public water, gas, electric and communication services within the fronting public right-of-ways of Lafayette Street, East Main Street, and North State Street.

## Compliance with Stormwater Management Standards

## Standard 1. Runoff and Pollutant Reduction

Per section 2.4 of the City Stormwater Drainage Manual, the project is required to retain the full water quality volume (WQV) on-site using non-structural practices or infiltration best management practices.

Provisions shall be made to improve the quality of the stormwater runoff flowing from the site. The Water Quality Volume calculated for the proposed development site is 3,620 cubic feet per a calculation in conformance with the 2004 Connecticut Stormwater Quality Manual section 7.4.1. Refer to Appendix C. The two proposed stormwater infiltration systems provide for a cumulative 3,625 cubic feet prior to discharging pass the overflow weir.

## Standard 2. Peak Flow Control

A. Stream channel protection is not required for this project, because the property does not discharge directly into a water body or watercourse. Regardless, the project demonstrates compliance. The post development 24 hour 2-year storm event peak flow rate of runoff of 2.28 cfs is less than the pre-development 1 -year peak flow rate of 3.09 cfs .
B. The proposed stormwater system is designed to adequately pass flows up to the 25 -year design storm event as required in Section 3 of the drainage manual. Refer to the HydroCAD model found in Appendix B, and the Conveyance calculations in Appendix D.
C. The post-development peak flow rates from the 1 -year, 2 -year, 5 -year, 10 -year, 25 -year and 50- year, 24 -hour storm events are controlled to the corresponding pre-development peak discharge rates. Reference is made to the HydroCAD report found in Appendix B.
D. The infiltration galleries are designed with separate metering manhole downstream. They are equipped with high overflow weir walls to pass the larger 50 and 100-year storm events.
E. A downstream hydraulic grade line analysis has been prepared for the project. Refer to Appendices E and F herein.

## Standard 3. Construction Erosion and Sediment Control

A. A detailed Erosion and Sediment Control Plan (sheet C-4) is designed to minimize erosion and contain and properly dispose of any accumulated sediment during construction. The erosion control measures proposed are to be installed and maintained in accordance with the Connecticut Guidelines for Soil Erosion and Sediment Control. Temporary sediment and erosion controls include an anti-tracking pad, silt fence, and tree protection. The proper use of the sediment and erosion control measures minimizes potential negative impacts during
construction. Additionally, the proposed catch basins and trench drain will have two-foot (2') sumps and bell trap/pvc elbow to remove sediment and/or floatables.

## Standard 4. Operation and Maintenance

A. A Standard City of Stamford Drainage Maintenance Agreement will be executed with the Environmental Protection Board at the completion of construction. A draft maintenance agreement has been prepared and is included in Appendix I.
B. The Low Impact Development Plan on sheet C-9 includes notes describing the long-term maintenance requirements for the project's drainage system. This includes routine and nonroute inspection and maintenance tasks to be undertaken after construction is completed as well as the schedule for implementing these tasks.

## Standard 5. Stormwater Management Report

A. This document and its enclosed appendices serve as the required Stormwater Management.
B. Our certification is provided herein.

## Summary

Based on the above information, the proposed improvements are designed in accordance with the City of Stamford Stormwater Drainage Manual and will not adversely impact adjacent or downstream properties or City-owned drainage facilities."


Louis DiMarzo, P
CT Registration \# 26847 II!"!!!!
February 3, 2022

## APPENDIX - A

## National Flood Hazard Layer FIRMette



## Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT
SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

| SPECIAL FLOOD |
| :--- | :--- |
| HAZARD AREAS |


| Without Base Flood Elevation (BFE) |
| :--- | :--- |
| Zone A, $V$, A99 |
| With BFE or Depth Zone AE, AO, AH, VE, AR |

Regulatory Floodway

B- 20.2 Cross Sections with 1\% Annual Chance
17.5 Water Surface Elevation
(8)- - - Coastal Transec
mu $\sin _{13}$. Base Flood Elevation Line (BFE)
Limit of Study

- Jurisdiction Boundary
--- --- Coastal Transect Baseline
OTHER FEATURES $\qquad$ Profile Baseline
$\qquad$

MAP PANELS

## $\therefore$ Digital Data Available

No Digital Data Available Unmapped


The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards
The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 11/9/2020 at 9:50 AM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images fo unmapped and unmodernized areas cannot be used for regulatory purposes.


## MAP LEGEND

| Area of Interest (AOI) |  | $C$$C / D$ |
| :---: | :---: | :---: |
| Area of Interest (AOI) |  |  |
| Soils $\square$ |  |  |
| Soil Rating Polygons |  |  |
| $\square \mathrm{A}$ | $\square$ | Not rated or not available |
| A/D | Water Fe | ures |
|  | $\sim$ | Streams and Canals |
| B |  |  |
|  | Transpo | tion |
| B/D | H+ | Rails |
| C | $\sim$ | Interstate Highways |
| C/D | - | US Routes |
| D | $\approx$ | Major Roads |
| Not rated or not available | 12) | Local Roads |
| Soil Rating Lines | Backgro |  |
| $\cdots \mathrm{A}$ |  | Aerial Photography |
| $\cdots$ A/D |  |  |
| $\cdots B$ |  |  |
| $\cdots$ B/D |  |  |
| $\cdots \mathrm{C}$ |  |  |
| $\cdots$ C/D |  |  |
| $\cdots$ D |  |  |
| * Not rated or not available |  |  |
| Soil Rating Points |  |  |
| $\square \quad \mathrm{A}$ |  |  |
| $\square \quad \mathrm{A} / \mathrm{D}$ |  |  |
| $\square \quad \mathrm{B}$ |  |  |
| $\square \mathrm{B} / \mathrm{D}$ |  |  |

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:12,000.

Warning: Soil Map may not be valid at this scale.
Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale

Please rely on the bar scale on each map sheet for map measurements.
Source of Map: Natural Resources Conservation Service Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)
Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.
This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.
Soil Survey Area: State of Connecticut
Survey Area Data: Version 21, Sep 7, 2021
Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Oct 8, 2020—Oct 14 2020

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

## Hydrologic Soil Group

| Map unit symbol | Map unit name | Rating | Acres in AOI | Percent of AOI |
| :--- | :--- | :--- | ---: | ---: |
| 307 | Urban land | D | 1.3 | $100.0 \%$ |
| Totals for Area of Interest | $\mathbf{1 . 3}$ | $\mathbf{1 0 0 . 0 \%}$ |  |  |

## Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

## Rating Options

Aggregation Method: Dominant Condition
Component Percent Cutoff: None Specified

Tie-break Rule: Higher

NOAA Atlas 14, Volume 10, Version 3
Location name: Stamford, Connecticut, USA* Latitude: $\mathbf{4 1 . 0 5 5 1}^{\circ}$, Longitude: $\mathbf{- 7 3 . 5 2 7 1}{ }^{\circ}$

Elevation: $18.08 \mathrm{ft}^{* *}$

* source: ESRI Maps
** source: USGS


## POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite
NOAA, National Weather Service, Silver Spring, Maryland
PF tabular | PF_graphical | Maps \& aerials

## PF tabular

| PDS-based point precipitation frequency estimates with 90\% confidence intervals (in inches) ${ }^{1}$ |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Duration | Average recurrence interval (years) |  |  |  |  |  |  |  |  |  |
|  | 1 | 2 | 5 | 10 | 25 | 50 | 100 | 200 | 500 | 1000 |
| 5-min | $\begin{gathered} 0.365 \\ (0.282-0.463) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{0 . 4 2 5} \\ (0.328-0.540) \end{gathered}$ | $\begin{gathered} 0.523 \\ (0.403-0.667) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{0 . 6 0 5} \\ (0.463-0.775) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{0 . 7 1 7} \\ (0.532-0.951) \\ \hline \end{gathered}$ | $\begin{gathered} 0.802 \\ (0.583-1.08) \\ \hline \end{gathered}$ | $\begin{gathered} 0.890 \\ (0.628-1.24) \end{gathered}$ | $\begin{array}{\|c\|} \hline 0.985 \\ (0.662-1.40) \\ \hline \end{array}$ | $\begin{array}{\|c\|} \hline 1.12 \\ (0.724-1.64) \\ \hline \end{array}$ | $\begin{gathered} 1.23 \\ (0.775-1.83) \end{gathered}$ |
| 10-min | $\begin{gathered} 0.517 \\ (0.400-0.657) \end{gathered}$ | $\begin{gathered} \mathbf{0 . 6 0 2} \\ (0.465-0.765) \\ \hline \end{gathered}$ | $\begin{gathered} 0.741 \\ (0.571-0.945) \end{gathered}$ | $\begin{array}{\|c\|} \hline 0.856 \\ (0.656-1.10) \\ \hline \end{array}$ | $\begin{gathered} 1.01 \\ (0.753-1.35) \\ \hline \end{gathered}$ | $\begin{gathered} 1.14 \\ (0.825-1.53) \\ \hline \end{gathered}$ | $\begin{gathered} 1.26 \\ (0.889-1.75) \end{gathered}$ | $\begin{gathered} 1.40 \\ (0.939-1.99) \end{gathered}$ | $\begin{array}{\|c\|} \hline 1.59 \\ (1.03-2.32) \\ \hline \end{array}$ | $\begin{gathered} \hline 1.74 \\ (1.10-2.59) \end{gathered}$ |
| 15-min | $\begin{gathered} \mathbf{0 . 6 0 8} \\ (0.470-0.772) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{0 . 7 0 8} \\ (0.547-0.901) \\ \hline \end{gathered}$ | $\begin{array}{c\|} \hline 0.872 \\ (0.671-1.11) \\ \hline \end{array}$ | $\begin{array}{\|c\|} \hline 1.01 \\ (0.772-1.29) \\ \hline \end{array}$ | $\begin{gathered} 1.20 \\ (0.886-1.59) \end{gathered}$ | $\begin{array}{\|c\|} \hline 1.34 \\ (0.971-1.81) \\ \hline \end{array}$ | $\begin{gathered} 1.48 \\ (1.05-2.06) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 1.64 \\ (1.11-2.34) \\ \hline \end{gathered}$ | $\begin{array}{\|c\|} \hline 1.86 \\ (1.21-2.73) \\ \hline \end{array}$ | $\begin{gathered} \mathbf{2 . 0 4} \\ (1.29-3.05) \\ \hline \end{gathered}$ |
| 30-min | $\begin{gathered} \hline 0.850 \\ (0.658-1.08) \\ \hline \end{gathered}$ | $\begin{gathered} 0.991 \\ (0.766-1.26) \\ \hline \end{gathered}$ | $\begin{gathered} 1.22 \\ (0.941-1.56) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 1.41 \\ (1.08-1.81) \\ \hline \end{gathered}$ | $\begin{gathered} 1.68 \\ (1.24-2.22) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 1.88 \\ (1.36-2.53) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 2.08 \\ (1.47-2.89) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 2.30 \\ (1.55-3.27) \\ \hline \end{gathered}$ | $\begin{array}{\|c\|} \hline \mathbf{2 . 6 0} \\ (1.69-3.81) \\ \hline \end{array}$ | $\begin{gathered} 2.84 \\ (1.80-4.23) \\ \hline \end{gathered}$ |
| 60-min | $\begin{gathered} \hline \hline 1.09 \\ (0.845-1.39) \end{gathered}$ | $\begin{gathered} 1.27 \\ (0.985-1.62) \end{gathered}$ | $\begin{gathered} \hline 1.57 \\ (1.21-2.00) \end{gathered}$ | $\begin{gathered} \hline 1.82 \\ (1.39-2.33) \end{gathered}$ | $\begin{gathered} \hline 2.16 \\ (1.60-2.86) \end{gathered}$ | $\begin{gathered} 2.42 \\ (1.75-3.26) \end{gathered}$ | $\begin{gathered} \hline 2.68 \\ (1.89-3.72) \end{gathered}$ | $\begin{gathered} \hline 2.96 \\ (1.99-4.21) \end{gathered}$ | $\begin{gathered} 3.34 \\ (2.16-4.89) \end{gathered}$ | $\begin{gathered} \hline \mathbf{3 . 6 3} \\ (2.30-5.42) \end{gathered}$ |
| 2-hr | $\begin{gathered} 1.41 \\ (1.10-1.78) \end{gathered}$ | $\begin{gathered} \hline 1.66 \\ (1.29-2.10) \end{gathered}$ | $\begin{gathered} \hline 2.07 \\ (1.61-2.63) \end{gathered}$ | $\begin{gathered} \hline 2.41 \\ (1.86-3.08) \end{gathered}$ | $\begin{gathered} \hline 2.88 \\ (2.15-3.80) \end{gathered}$ | $\begin{gathered} \hline 3.24 \\ (2.37-4.35) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 3.61 \\ (2.56-4.99) \end{gathered}$ | $\begin{gathered} 4.01 \\ (2.71-5.66) \end{gathered}$ | $\begin{array}{\|c\|} \hline 4.57 \\ (2.97-6.65) \\ \hline \end{array}$ | $\begin{gathered} \hline 5.01 \\ (3.18-7.44) \\ \hline \end{gathered}$ |
| 3-hr | $\begin{gathered} \hline 1.63 \\ (1.27-2.05) \\ \hline \end{gathered}$ | $\begin{gathered} 1.93 \\ (1.50-2.42) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 2.42 \\ (1.88-3.05) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 2.82 \\ (2.18-3.58) \\ \hline \end{gathered}$ | $\begin{gathered} 3.38 \\ (2.53-4.45) \\ \hline \end{gathered}$ | $\begin{array}{c\|} \hline 3.80 \\ (2.79-5.09) \\ \hline \end{array}$ | $\begin{gathered} \hline 4.24 \\ (3.02-5.86) \\ \hline \end{gathered}$ | $\begin{array}{c\|} \hline 4.73 \\ (3.20-6.66) \\ \hline \end{array}$ | $\begin{gathered} \hline 5.42 \\ (3.53-7.86) \\ \hline \end{gathered}$ | $\begin{gathered} 5.98 \\ (3.80-8.83) \\ \hline \end{gathered}$ |
| 6-hr | $\begin{gathered} 2.05 \\ (1.61-2.56) \end{gathered}$ | $\begin{gathered} \mathbf{2 . 4 4} \\ (1.92-3.05) \end{gathered}$ | $\begin{gathered} \hline 3.08 \\ (2.41-3.87) \end{gathered}$ | $\begin{gathered} \hline 3.61 \\ (2.81-4.55) \end{gathered}$ | $\begin{gathered} \hline 4.35 \\ (3.28-5.69) \end{gathered}$ | $\begin{gathered} \hline 4.89 \\ (3.62-6.52) \end{gathered}$ | $\begin{gathered} 5.47 \\ (3.93-7.54) \end{gathered}$ | $\begin{gathered} 6.13 \\ (4.16-8.57) \end{gathered}$ | $\begin{gathered} 7.07 \\ (4.62-10.2) \end{gathered}$ | $\begin{gathered} 7.85 \\ (5.00-11.5) \end{gathered}$ |
| 12-hr | $\begin{gathered} \hline 2.53 \\ (2.00-3.14) \end{gathered}$ | $\begin{gathered} \hline 3.03 \\ (2.39-3.76) \end{gathered}$ | $\begin{gathered} 3.84 \\ (3.03-4.78) \end{gathered}$ | $\begin{gathered} \hline 4.51 \\ (3.54-5.65) \end{gathered}$ | $\begin{gathered} 5.44 \\ (4.13-7.08) \end{gathered}$ | $\begin{gathered} \mathbf{6 . 1 4} \\ (4.56-8.14) \end{gathered}$ | $\begin{gathered} 6.87 \\ (4.97-9.42) \end{gathered}$ | $\begin{gathered} 7.72 \\ (5.26-10.7) \end{gathered}$ | $\begin{gathered} 8.95 \\ (5.87-12.8) \end{gathered}$ | $\begin{gathered} 9.98 \\ (6.39-14.5) \end{gathered}$ |
| 24-hr | $\begin{gathered} \mathbf{2 . 9 6} \\ (2.36-3.65) \end{gathered}$ | $\begin{gathered} 3.58 \\ (2.85-4.42) \end{gathered}$ | $\begin{gathered} 4.60 \\ (3.65-5.70) \end{gathered}$ | $\begin{gathered} 5.45 \\ (4.29-6.77) \end{gathered}$ | $\begin{gathered} \hline 6.61 \\ (5.05-8.56) \end{gathered}$ | $\begin{array}{c\|} \hline 7.47 \\ (5.59-9.87) \end{array}$ | $\begin{gathered} 8.40 \\ (6.11-11.5) \end{gathered}$ | $\begin{gathered} \hline 9.49 \\ (6.49-13.1) \end{gathered}$ | $\begin{gathered} 11.1 \\ (7.30-15.8) \end{gathered}$ | $\begin{gathered} \hline 12.5 \\ (8.00-18.0) \end{gathered}$ |
| 2-day | $\begin{gathered} 3.31 \\ (2.65-4.05) \end{gathered}$ | $\begin{gathered} 4.07 \\ (3.26-4.99) \end{gathered}$ | $\begin{gathered} 5.32 \\ (4.25-6.54) \end{gathered}$ | $\begin{gathered} 6.35 \\ (5.04-7.85) \end{gathered}$ | $\begin{gathered} 7.78 \\ (5.98-10.0) \end{gathered}$ | $\begin{gathered} 8.83 \\ (6.66-11.6) \end{gathered}$ | $\begin{gathered} 9.97 \\ (7.32-13.6) \end{gathered}$ | $\begin{array}{c\|} \hline 11.3 \\ (7.79-15.6) \\ \hline \end{array}$ | $\begin{gathered} 13.4 \\ (8.86-19.0) \end{gathered}$ | $\begin{gathered} 15.2 \\ (9.80-21.9) \\ \hline \end{gathered}$ |
| 3-day | $\begin{gathered} 3.57 \\ (2.87-4.35) \end{gathered}$ | $\begin{gathered} 4.40 \\ (3.54-5.38) \\ \hline \end{gathered}$ | $\begin{gathered} 5.77 \\ (4.63-7.07) \end{gathered}$ | $\begin{gathered} 6.91 \\ (5.50-8.50) \end{gathered}$ | $\begin{gathered} 8.47 \\ (6.54-10.9) \\ \hline \end{gathered}$ | $\begin{array}{c\|} \hline 9.62 \\ (7.28-12.6) \\ \hline \end{array}$ | $\begin{array}{\|c\|} \hline 10.9 \\ (8.01-14.8) \\ \hline \end{array}$ | $\begin{array}{c\|} \hline 12.4 \\ (8.52-16.9) \\ \hline \end{array}$ | $\begin{gathered} 14.7 \\ (9.71-20.7) \end{gathered}$ | $\begin{gathered} 16.7 \\ (10.7-23.9) \end{gathered}$ |
| 4-day | $\begin{gathered} 3.82 \\ (3.09-4.65) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 4.70 \\ (3.79-5.73) \\ \hline \end{gathered}$ | $\begin{gathered} 6.14 \\ (4.94-7.50) \\ \hline \end{gathered}$ | $\begin{gathered} 7.34 \\ (5.86-9.00) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 8.98 \\ (6.95-11.5) \\ \hline \end{gathered}$ | $\begin{array}{c\|} \hline 10.2 \\ (7.73-13.3) \\ \hline \end{array}$ | $\begin{array}{c\|} \hline 11.5 \\ (8.50-15.6) \\ \hline \end{array}$ | $\begin{gathered} 13.1 \\ (9.03-17.9) \\ \hline \end{gathered}$ | $\begin{array}{\|c\|} \hline 15.5 \\ (10.3-21.8) \\ \hline \end{array}$ | $\begin{gathered} 17.6 \\ (11.4-25.1) \\ \hline \end{gathered}$ |
| 7-day | $\begin{gathered} \hline 4.55 \\ (3.70-5.51) \end{gathered}$ | $\begin{gathered} 5.51 \\ (4.47-6.67) \end{gathered}$ | $\begin{gathered} 7.07 \\ (5.72-8.59) \end{gathered}$ | $\begin{gathered} \hline 8.37 \\ (6.72-10.2) \end{gathered}$ | $\begin{gathered} 10.2 \\ (7.89-12.9) \end{gathered}$ | $\begin{gathered} \hline 11.5 \\ (8.73-14.9) \end{gathered}$ | $\begin{gathered} 12.9 \\ (9.53-17.3) \end{gathered}$ | $\begin{gathered} \hline 14.6 \\ (10.1-19.8) \end{gathered}$ | $\begin{gathered} 17.1 \\ (11.4-23.9) \end{gathered}$ | $\begin{gathered} \hline 19.2 \\ (12.5-27.3) \end{gathered}$ |
| 10-day | $\begin{gathered} \hline 5.27 \\ (4.30-6.36) \end{gathered}$ | $\begin{gathered} \hline 6.28 \\ (5.11-7.58) \end{gathered}$ | $\begin{gathered} \hline 7.92 \\ (6.43-9.59) \end{gathered}$ | $\begin{gathered} 9.29 \\ (7.49-11.3) \\ \hline \end{gathered}$ | $\begin{gathered} 11.2 \\ (8.69-14.1) \end{gathered}$ | $\begin{gathered} 12.6 \\ (9.57-16.2) \end{gathered}$ | $\begin{gathered} 14.1 \\ (10.4-18.7) \end{gathered}$ | $\begin{gathered} \hline 15.8 \\ (11.0-21.3) \end{gathered}$ | $\begin{gathered} 18.3 \\ (12.2-25.5) \end{gathered}$ | $\begin{gathered} \mathbf{2 0 . 4} \\ (13.2-28.9) \end{gathered}$ |
| 20-day | $\begin{gathered} \hline 7.44 \\ (6.10-8.91) \\ \hline \end{gathered}$ | $\begin{gathered} 8.57 \\ (7.02-10.3) \end{gathered}$ | $\begin{gathered} 10.4 \\ (8.50-12.5) \end{gathered}$ | $\begin{gathered} 12.0 \\ (9.69-14.4) \\ \hline \end{gathered}$ | $\begin{gathered} 14.1 \\ (11.0-17.6) \end{gathered}$ | $\begin{gathered} \hline 15.7 \\ (11.9-19.9) \\ \hline \end{gathered}$ | $\begin{array}{c\|} \hline 17.3 \\ (12.7-22.6) \\ \hline \end{array}$ | $\begin{array}{c\|} \hline 19.1 \\ (13.3-25.6) \\ \hline \end{array}$ | $\begin{gathered} \mathbf{2 1 . 5} \\ (14.4-29.7) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{2 3 . 5} \\ (15.3-33.0) \\ \hline \end{gathered}$ |
| 30-day | $\begin{gathered} 9.23 \\ (7.60-11.0) \end{gathered}$ | $\begin{gathered} \hline \mathbf{1 0 . 4} \\ (8.60-12.5) \end{gathered}$ | $\begin{gathered} \mathbf{1 2 . 4} \\ (10.2-14.9) \end{gathered}$ | $\begin{gathered} 14.1 \\ (11.5-17.0) \end{gathered}$ | $\begin{gathered} 16.4 \\ (12.8-20.3) \end{gathered}$ | $\begin{gathered} \mathbf{1 8 . 1} \\ (13.9-22.9) \end{gathered}$ | $\begin{gathered} 19.9 \\ (14.6-25.8) \end{gathered}$ | $\begin{gathered} \hline 21.7 \\ (15.2-28.9) \end{gathered}$ | $\begin{gathered} \mathbf{2 4 . 1} \\ (16.1-33.1) \end{gathered}$ | $\begin{gathered} \mathbf{2 5 . 9} \\ (16.9-36.2) \end{gathered}$ |
| 45-day | $\begin{gathered} 11.4 \\ (9.46-13.6) \end{gathered}$ | $\begin{gathered} \hline 12.8 \\ (10.5-15.2) \end{gathered}$ | $\begin{gathered} 14.9 \\ (12.3-17.8) \end{gathered}$ | $\begin{gathered} 16.7 \\ (13.7-20.0) \end{gathered}$ | $\begin{gathered} 19.2 \\ (15.1-23.7) \end{gathered}$ | $\begin{gathered} \mathbf{2 1 . 1} \\ (16.2-26.5) \end{gathered}$ | $\begin{gathered} \mathbf{2 3 . 0} \\ (16.9-29.5) \end{gathered}$ | $\begin{gathered} 24.9 \\ (17.5-33.0) \end{gathered}$ | $\begin{gathered} \mathbf{2 7 . 2} \\ (18.3-37.2) \end{gathered}$ | $\begin{gathered} 28.9 \\ (18.9-40.3) \end{gathered}$ |
| 60-day | $\begin{gathered} 13.3 \\ (11.0-15.7) \\ \hline \end{gathered}$ | $\begin{gathered} 14.7 \\ (12.2-17.4) \\ \hline \end{gathered}$ | $\begin{gathered} 17.0 \\ (14.0-20.2) \\ \hline \end{gathered}$ | $\begin{gathered} \hline 18.9 \\ (15.5-22.6) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{2 1 . 5} \\ (16.9-26.4) \\ \hline \end{gathered}$ | $\begin{array}{c\|} \hline \mathbf{2 3 . 6} \\ (18.1-29.4) \\ \hline \end{array}$ | $\begin{array}{c\|} \hline \mathbf{2 5 . 6} \\ (18.8-32.7) \\ \hline \end{array}$ | $\begin{array}{c\|} \hline \mathbf{2 7 . 5} \\ (19.3-36.3) \\ \hline \end{array}$ | $\begin{array}{\|c\|} \hline \mathbf{2 9 . 8} \\ (20.1-40.6) \\ \hline \end{array}$ | $\begin{gathered} \hline 31.5 \\ (20.6-43.7) \\ \hline \end{gathered}$ |

[^4]PDS-based depth-duration-frequency (DDF) curves Latitude: $41.0551^{\circ}$, Longitude: $-73.5271^{\circ}$


| Average recurrence <br> interval <br> (years) |
| :---: |
| -1 |
| -2 |
| -5 |
| -10 |
| -25 |
| -50 |
| — 100 |
| — 200 |
| — 500 |
| -1000 |



| Duration |  |
| :---: | :---: |
| — $5-\mathrm{min}$ — $10-\mathrm{min}$ $-15-\mathrm{min}$ $-30-\mathrm{min}$ $-60-\mathrm{min}$ $-2-\mathrm{hr}$ $-3-\mathrm{hr}$ -6 hr $-12-\mathrm{hr}$ $-24-\mathrm{hr}$ | - 2-day <br> - 3-day <br> - 4-day <br> - 7-day <br> - 10-day <br> - 20-day <br> - 30-day <br> - 45-day <br> - 60-day |

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Maps \& aerials

## Small scale terrain



Large scale aerial


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US Department of Commerce
National Oceanic and Atmospheric Administration
National Weather Service
National Water Center 1325 East West Highway
Silver Spring, MD 20910
Questions?: HDSC.Questions@noaa.gov
Disclaimer

## APPENDIX - B





Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

## Subcatchment 11: EX. SOUTH BASIN <br> Subcatchment 15: EX. EAST BASIN

Subcatchment 17: EX. NORTH BASIN

Subcatchment21: PR. SOUTH BYPASS

Subcatchment22: PR. SOUTH-1 BASIN

Subcatchment23: PR. SOUTH-2 BASIN

Subcatchment25: PR. EAST BASIN

## Subcatchment27: PR. NORTH BASIN

Pond 62: BMP-1-48" CONC GALS

Pond 63: BMP-2-48" CONC GALS

## Link 91: EX. SOUTH OUT

Link 92: PR. SOUTH OUT

## Link 95: EX, EAST OUT

Link 96: PR. EAST OUT

Link 97: EX, NORTH OUT

Link 98: PR. NORTH OUT

Runoff Area=48,082 sf $80.34 \%$ Impervious Runoff Depth $>2.43^{\prime \prime}$ $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=95.25$ Runoff=3.09 cfs $9,752 \mathrm{cf}$

Runoff Area $=1,354$ sf $0.00 \%$ Impervious Runoff Depth $>1.48$ " $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=84.00$ Runoff $=0.06$ cfs 167 cf

Runoff Area=801 sf $93.51 \%$ Impervious Runoff Depth>2.63" $\mathrm{Tc}=5.0 \mathrm{~min} \quad \mathrm{CN}=97.09$ Runoff $=0.05 \mathrm{cfs} 175 \mathrm{cf}$

Runoff Area $=27,749$ sf $96.14 \%$ Impervious Runoff Depth>2.67" $\mathrm{Tc}=5.0 \mathrm{~min} \quad \mathrm{CN}=97.46$ Runoff $=1.88 \mathrm{cfs} 6,167 \mathrm{cf}$

Runoff Area=11,610 sf $87.75 \%$ Impervious Runoff Depth $>2.54$ " $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=96.29$ Runoff $=0.77 \mathrm{cfs} 2,459 \mathrm{cf}$

Runoff Area=8,852 sf $94.84 \%$ Impervious Runoff Depth>2.65" Tc=5.0 min CN=97.28 Runoff=0.60 cfs $1,953 \mathrm{cf}$

Runoff Area $=1,251$ sf $0.00 \%$ Impervious Runoff Depth $>1.48$ " $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=84.00$ Runoff $=0.05 \mathrm{cfs} 154 \mathrm{cf}$

Runoff Area=775 sf $27.48 \%$ Impervious Runoff Depth $>1.77$ " $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=87.85$ Runoff $=0.04 \mathrm{cfs} 114 \mathrm{cf}$

Peak Elev=12.48' Storage=2,076 cf Inflow=0.77 cfs 2,802 cf Outflow=0.04 cfs 751 cf

Peak Elev=16.45' Storage=1,630 cf Inflow=0.60 cfs 1,953 cf Outflow=0.02 cfs 343 cf Inflow=3.09 cfs 9,752 cf Primary=3.09 cfs 9,752 cf

Inflow=1.88 cfs 6,918 cf Primary $=1.88$ cfs 6,918 cf

Inflow=0.06 cfs 167 cf Primary=0.06 cfs 167 cf

Inflow=0.05 cfs 154 cf Primary $=0.05$ cfs 154 cf

Inflow=0.05 cfs 175 cf Primary=0.05cfs 175 cf

Inflow=0.04 cfs 114 cf Primary=0.04 cfs 114 cf

Total Runoff Area $=100,474$ sf Runoff Volume $=20,942$ cf Average Runoff Depth $=2.50$ " $\mathbf{1 5 . 5 5 \%}$ Pervious $=\mathbf{1 5 , 6 2 1} \mathbf{s f} \mathbf{8 4 . 4 5 \%}$ Impervious $=84,853$ sf

## Summary for Subcatchment 11: EX. SOUTH BASIN

Runoff $=\quad 3.09 \mathrm{cfs} @ 12.07 \mathrm{hrs}$, Volume= $9,752 \mathrm{cf}$, Depth> 2.43"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 1-Yr Stamford Rainfall=2.96"

| Area (sf) | CN | Description |  |
| ---: | ---: | :--- | :--- |
| * | 12,667 | 98.00 | Roofs, HSG D |
| 25,964 | 98.00 | Paved Hardscapes, HSG D |  |
| 9,451 | 84.00 | $50-75 \%$ Grass cover, Fair, HSG D |  |
| 48,082 | 95.25 | Weighted Average |  |
| 9,451 |  | 19.66\% Pervious Area |  |
| 38,631 |  | $80.34 \%$ Impervious Area |  |
| Tc | Length | Slope | Velocity |
| (feet) | Capacity | Description |  |
| (ft/ft) | (ft/sec) | (cfs) |  |
| (min) | Direct Entry, |  |  |

## Subcatchment 11: EX. SOUTH BASIN



## Summary for Subcatchment 15: EX. EAST BASIN

Runoff $=\quad 0.06$ cfs @ 12.08 hrs, Volume= 167 cf, Depth> 1.48"

Routed to Link 95 : EX, EAST OUT
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 1-Yr Stamford Rainfall=2.96"

|  | rea (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 0 | 98.00 | Roofs, HSG D |  |  |
|  | 0 | 98.00 | Paved Hardscapes, HSG D |  |  |
|  | 1,354 | 84.00 | 50-75\% | Grass cove | r, Fair, HSG D |
|  | $\begin{aligned} & 1,354 \\ & 1,354 \end{aligned}$ | 84.00 | 100.00\% Pervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 5.0 |  |  |  |  | Direct Entry, |

Subcatchment 15: EX. EAST BASIN


## Summary for Subcatchment 17: EX. NORTH BASIN

Runoff $=0.05$ cfs @ 12.07 hrs, Volume= 175 cf, Depth> 2.63"

Routed to Link 97 : EX, NORTH OUT
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 1-Yr Stamford Rainfall=2.96"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 0 | 98.00 | Roofs, HSG D |
| $*$ | 749 | 98.00 |
| Paved Hardscapes, HSG D |  |  |
| 52 | 84.00 | $50-75 \%$ Grass cover, Fair, HSG D |
| 801 | 97.09 | Weighted Average |
| 52 |  | $6.49 \%$ Pervious Area |
| 749 |  | $93.51 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- |

## Subcatchment 17: EX. NORTH BASIN



## Summary for Subcatchment 21: PR. SOUTH BYPASS

Runoff $=\quad 1.88$ cfs @ 12.07 hrs, Volume= $6,167 \mathrm{cf}$, Depth> 2.67"
Routed to Link 92 : PR. SOUTH OUT
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 1-Yr Stamford Rainfall=2.96"

|  | Area (sf) | CN | Reocription |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 24,917 | 98.00 |  |  |  |
|  | 1,760 | 98.00 | Paved Hardscapes, HSG D |  |  |
|  | 1,072 | 84.00 | 50-75\% Grass cover, Fair, HSG D |  |  |
|  | 27,749 | 97.46 | Weighted Average |  |  |
|  | 1,072 |  | 3.86\% Pervious Area |  |  |
|  | 26,677 |  | 96.14\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 5.0 |  |  |  |  | Direct Entry, |

## Subcatchment 21: PR. SOUTH BYPASS



## Summary for Subcatchment 22: PR. SOUTH-1 BASIN

Runoff $=\quad 0.77$ cfs @ 12.07 hrs, Volume $=\quad 2,459 \mathrm{cf}$, Depth> 2.54"

Routed to Pond 62 : BMP-1-48" CONC GALS
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 1-Yr Stamford Rainfall=2.96"

|  | Area (sf) | CN | Reocription |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1,480 | 98.00 |  |  |  |
|  | 8,708 | 98.00 | Paved Hardscapes, HSG D |  |  |
|  | 1,422 | 84.00 | 50-75\% | Grass cove | r, Fair, HSG D |
|  | 11,610 | 96.29 | Weighted Average |  |  |
|  | 1,422 |  | 12.25\% Pervious Area |  |  |
|  | 10,188 |  | 87.75\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 5.0 |  |  |  |  | Direct Entry, |

## Subcatchment 22: PR. SOUTH-1 BASIN



## Summary for Subcatchment 23: PR. SOUTH-2 BASIN

Runoff $=\quad 0.60$ cfs @ 12.07 hrs, Volume= $1,953 \mathrm{cf}$, Depth> 2.65"
Routed to Pond 63 : BMP-2-48" CONC GALS
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 1-Yr Stamford Rainfall=2.96"

|  | rea (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 0 | 98.00 | Roofs, HSG D |  |  |
|  | 8,395 | 98.00 | Paved Hardscapes, HSG D |  |  |
|  | 457 | 84.00 | 50-75\% Grass cover, Fair, HSG D |  |  |
|  | 8,852 | 97.28 | Weighted Average |  |  |
|  | 457 |  | 5.16\% Pervious Area |  |  |
|  | 8,395 |  | 94.84\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 5.0 |  |  |  |  | Direct Entry, |

## Subcatchment 23: PR. SOUTH-2 BASIN


$\square$ Runoff

## Summary for Subcatchment 25: PR. EAST BASIN

Runoff $=\quad 0.05$ cfs @ 12.08 hrs, Volume= 154 cf, Depth> 1.48"
Routed to Link 96 : PR. EAST OUT
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 1-Yr Stamford Rainfall=2.96"

|  | rea (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 0 | 98.00 | Roofs, HSG D |  |  |
|  | 0 | 98.00 | Paved Hardscapes, HSG D |  |  |
|  | 1,251 | 84.00 | 50-75\% Grass cover, Fair, HSG D |  |  |
|  | $\begin{aligned} & 1,251 \\ & 1,251 \end{aligned}$ | 84.00 | Weighted Average 100.00\% Pervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 5.0 |  |  |  |  | Direct Entry, |

Subcatchment 25: PR. EAST BASIN


## Summary for Subcatchment 27: PR. NORTH BASIN

Runoff $=\quad 0.04$ cfs @ 12.07 hrs, Volume= 114 cf, Depth> 1.77"

Routed to Link 98 : PR. NORTH OUT
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 1-Yr Stamford Rainfall=2.96"

|  | ea (sf) | CN | Roofs, HSG D |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 0 | 98.00 |  |  |  |
| * | 213 | 98.00 | Roofs, HSG D <br> Paved Hardscapes, HSG D |  |  |
|  | 562 | 84.00 | 50-75\% Grass cover, Fair, HSG D |  |  |
|  | 775 | 87.85 | Weighted Average |  |  |
|  | 562 |  | 72.52\% Pervious Area |  |  |
|  | 213 |  | 27.48\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 5.0 |  |  |  |  | Direct Entry, |

## Subcatchment 27: PR. NORTH BASIN



## Summary for Pond 62: BMP-1-48" CONC GALS

[79] Warning: Submerged Pond 63 Primary device \# 3 OUTLET by 0.98'

| Inflow Area = | 20,462 | 90.82\% Impervious | Depth > 1.64" for 1-Yr Stamford event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.77 cfs @ | 12.07 hrs , Volume= | 2,802 cf |
| Outflow | 0.04 cfs @ | 15.74 hrs, Volume= | 751 cf, Atten $=95 \%$, Lag $=220.0$ min |
| Primary | 0.04 cfs @ | 15.74 hrs, Volume= | 751 cf |

Routed to Link 92 : PR. SOUTH OUT
Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.01 \mathrm{hrs} / 4$
Peak Elev= 12.48' @ 15.74 hrs Surf.Area= 901 sf Storage= 2,076 cf
Plug-Flow detention time= 502.1 min calculated for 751 cf ( $27 \%$ of inflow)
Center-of-Mass det. time $=302.1 \mathrm{~min}(1,115.5-813.3)$

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | :---: | :---: | :---: |
| \#1 | 9.00' | 526 cf | 24.50 'W x 34.00'L x 4.50'H Prismatoid |
|  |  |  | 3,749 cf Overall - 2,433 cf Embedded $=1,316$ cf $\times 40.0 \%$ Voids |
| \#2 | $9.50 '$ | 1,871 cf | Concrete Galley $4 \times 8 \times 4 \times 20$ Inside \#1 |
|  |  |  | Inside $=42.0 \mathrm{~W}$ W 43.0"H $=>12.47 \mathrm{sf} \times 7.50 \mathrm{~L}=93.6 \mathrm{cf}$ |
|  |  |  | Outside= 52.8"W x 48.0"H => $15.20 \mathrm{sf} \times 8.00 \mathrm{~L}=121.6 \mathrm{cf}$ |
|  |  |  | 20 Chambers in 5 Rows |
| \#3 | $11.60{ }^{\prime}$ | 41 cf | $3.00^{\prime} \mathrm{W} \times 4.00^{\prime} \mathrm{L} \times 3.40{ }^{\prime} \mathrm{H}$ CB\#2 |
| \#4 | 11.75' | 27 cf | $3.00^{\prime} \mathrm{W} \times 4.00^{\prime} \mathrm{L} \times 2.25^{\prime} \mathrm{H}$ CB\#1 |
| \#5 | 11.50' | 67 cf | 12.0" Round Pipe Storage |
|  |  |  | L= 85.0' S= 0.0350 '/' |
| \#6 | 11.00' | 27 cf | 10.0" Round Pipe Storage |
|  |  |  | L= 50.0' S= 0.0200 '/' |
| \#7 | 11.50 | 14 cf | 10.0" Round Pipe Storage |
|  |  |  | L=25.0' S= 0.0200 '/' |
| \#8 | 11.00' | 16 cf | 10.0" Round Pipe Storage |
|  |  |  | L=30.0' S=0.0200 '/' |
|  | $2,589 \mathrm{cf}$ Total Available Storage |  |  |
| Device | Routing | Invert Outl | t Devices |
| \#1 | Device 3 | 14.00' 4.0' | 4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s) |
| \#2 | Device 3 | 12.40' 6.0' | Vert. Orifice X 2.00 C= 0.600 |
|  |  |  | ed to weir flow at low heads |
| \#3 | Primary | 10.80' 12.0 | ' Round Culvert |
|  |  |  | 0.0' CPP, square edge headwall, $\mathrm{Ke}=0.500$ |
|  |  |  | / Outlet Invert= 10.80' / 10.00' S= 0.0200 '/' Cc= 0.900 |
|  |  |  | 010 PVC, smooth interior, Flow Area= 0.79 sf |
| Primary OutFlow Max=0.04 cfs @ 15.74 hrs HW=12.48' TW=9.65' (Fixed TW Elev= 9.65') — 3=Culvert (Passes 0.04 cfs of 4.11 cfs potential flow) |  |  |  |
|  |  |  |  |  |  |
| -1=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs ) |  |  |  |
| -2=Orifice (Orifice Controls 0.04 cfs @ 0.95 fps ) |  |  |  |

## Pond 62: BMP-1-48" CONC GALS



## $\square$ Inflow

 $\square$ PrimaryStage-Area-Storage for Pond 62: BMP-1-48" CONC GALS

| Elevation (feet) | Storage (cubic-feet) | Elevation (feet) | Storage (cubic-feet) | Elevation (feet) | Storage (cubic-feet) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 9.00 | 0 | 11.65 | 1,501 | 14.30 | 2,566 |
| 9.05 | 17 | 11.70 | 1,535 | 14.35 | 2,568 |
| 9.10 | 33 | 11.75 | 1,569 | 14.40 | 2,569 |
| 9.15 | 50 | 11.80 | 1,604 | 14.45 | 2,571 |
| 9.20 | 67 | 11.85 | 1,639 | 14.50 | 2,573 |
| 9.25 | 83 | 11.90 | 1,674 | 14.55 | 2,574 |
| 9.30 | 100 | 11.95 | 1,709 | 14.60 | 2,576 |
| 9.35 | 117 | 12.00 | 1,744 | 14.65 | 2,578 |
| 9.40 | 133 | 12.05 | 1,779 | 14.70 | 2,579 |
| 9.45 | 150 | 12.10 | 1,815 | 14.75 | 2,581 |
| 9.50 | 167 | 12.15 | 1,850 | 14.80 | 2,582 |
| 9.55 | 196 | 12.20 | 1,885 | 14.85 | 2,584 |
| 9.60 | 226 | 12.25 | 1,919 | 14.90 | 2,585 |
| 9.65 | 256 | 12.30 | 1,954 | 14.95 | 2,586 |
| 9.70 | 286 | 12.35 | 1,989 | 15.00 | 2,587 |
| 9.75 | 317 | 12.40 | 2,023 | 15.05 | 2,588 |
| 9.80 | 347 | 12.45 | 2,057 | 15.10 | 2,588 |
| 9.85 | 378 | 12.50 | 2,091 | 15.15 | 2,589 |
| 9.90 | 409 | 12.55 | 2,124 | 15.20 | 2,589 |
| 9.95 | 440 | 12.60 | 2,158 | 15.25 | 2,589 |
| 10.00 | 471 | 12.65 | 2,191 | 15.30 | 2,589 |
| 10.05 | 502 | 12.70 | 2,224 | 15.35 | 2,589 |
| 10.10 | 533 | 12.75 | 2,257 | 15.40 | 2,589 |
| 10.15 | 564 | 12.80 | 2,289 | 15.45 | 2,589 |
| 10.20 | 595 | 12.85 | 2,322 |  |  |
| 10.25 | 626 | 12.90 | 2,355 |  |  |
| 10.30 | 657 | 12.95 | 2,387 |  |  |
| 10.35 | 688 | 13.00 | 2,420 |  |  |
| 10.40 | 719 | 13.05 | 2,452 |  |  |
| 10.45 | 750 | 13.10 | 2,475 |  |  |
| 10.50 | 781 | 13.15 | 2,482 |  |  |
| 10.55 | 811 | 13.20 | 2,489 |  |  |
| 10.60 | 842 | 13.25 | 2,496 |  |  |
| 10.65 | 873 | 13.30 | 2,504 |  |  |
| 10.70 | 904 | 13.35 | 2,511 |  |  |
| 10.75 | 935 | 13.40 | 2,518 |  |  |
| 10.80 | 965 | 13.45 | 2,525 |  |  |
| 10.85 | 996 | 13.50 | 2,532 |  |  |
| 10.90 | 1,027 | 13.55 | 2,535 |  |  |
| 10.95 | 1,058 | 13.60 | 2,537 |  |  |
| 11.00 | 1,088 | 13.65 | 2,539 |  |  |
| 11.05 | 1,119 | 13.70 | 2,542 |  |  |
| 11.10 | 1,150 | 13.75 | 2,544 |  |  |
| 11.15 | 1,181 | 13.80 | 2,546 |  |  |
| 11.20 | 1,212 | 13.85 | 2,549 |  |  |
| 11.25 | 1,243 | 13.90 | 2,551 |  |  |
| 11.30 | 1,275 | 13.95 | 2,553 |  |  |
| 11.35 | 1,307 | 14.00 | 2,556 |  |  |
| 11.40 | 1,338 | 14.05 | 2,557 |  |  |
| 11.45 | 1,370 | 14.10 | 2,559 |  |  |
| 11.50 | 1,403 | 14.15 | 2,561 |  |  |
| 11.55 | 1,435 | 14.20 | 2,562 |  |  |
| 11.60 | 1,468 | 14.25 | 2,564 |  |  |

## Summary for Pond 63: BMP-2 - 48" CONC GALS



Routed to Pond 62 : BMP-1-48" CONC GALS
Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.01 \mathrm{hrs} / 4$
Peak Elev= 16.45' @ 15.61 hrs Surf.Area= 713 sf Storage= 1,630 cf
Plug-Flow detention time $=582.7$ min calculated for 343 cf ( $18 \%$ of inflow)
Center-of-Mass det. time $=341.0 \mathrm{~min}(1,105.0-764.1)$

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | :---: | :---: | :---: |
| \#1 | 13.00' | 446 cf | $\begin{aligned} & \text { 20.00'W x 34.00'L x 4.50'H Prismatoid } \\ & 3,060 \mathrm{cf} \text { Overall }-1,946 \mathrm{cf} \text { Embedded }=1,114 \mathrm{cf} \times 40.0 \% \text { Voids } \end{aligned}$ |
| \#2 | 13.50 | 1,497 cf | Concrete Galley $\mathbf{4 x 8 x 4} \times 16$ Inside \#1 Inside= 42.0"W x 43.0"H => $12.47 \mathrm{sf} \times 7.50^{\prime} \mathrm{L}=93.6 \mathrm{cf}$ Outside $=52.8^{\prime \prime} \mathrm{W} \times 48.0^{\prime \prime} \mathrm{H}=>15.20 \mathrm{sf} \times 8.00^{\prime} \mathrm{L}=121.6 \mathrm{cf}$ 16 Chambers in 4 Rows |
| \#3 | 16.10' | 40 cf | 3.00 'W x 4.00'L x 3.30'H CB\#4 |
| \#4 | 15.90' | 27 cf | 3.00 'W x 4.00'L x 2.25 'H CB\#3 |
| \#5 | 15.20' | 20 cf | 10.0" Round Pipe Storage $L=36.0^{\prime} \quad S=0.0200 ' / 1$ |
|  |  | 2,029 cf | Total Available Storage |
| Device | Routing | Invert Outl | t Devices |
| \#1 | Device 3 | 18.00' 4.0' | ong Sharp-Crested Rectangular Weir 2 End Contraction(s) |
| \#2 | Device 3 | $\text { 16.40' } \begin{gathered} \text { 6.0" } \\ \text { Limi } \end{gathered}$ | Vert. Orifice/Grate X $2.00 \quad \mathrm{C}=0.600$ ed to weir flow at low heads |
| \#3 | Primary | 14.50' 12.0 | Round Culvert |
|  |  | $\begin{aligned} & \mathrm{L}=8 \\ & \text { Inlet } \end{aligned}$ | 5.0' CPP, square edge headwall, $\mathrm{Ke}=0.500$ / Outlet Invert= 14.50' $/ 11.50$ ' S=0.0353 '/' Cc= 0.900 |
|  |  |  | 010 PVC, smooth interior, Flow Area= 0.79 sf |
| Primary OutFlow Max=0.02 cfs @ 15.61 hrs HW=16.45' (Free Discharge) $\leftarrow_{3=C u l v e r t ~(P a s s e s ~} 0.02$ cfs of 4.56 cfs potential flow) |  |  |  |
|  |  |  |  |
| -1=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs ) |  |  |  |

## Pond 63: BMP-2-48" CONC GALS



Stage-Area-Storage for Pond 63: BMP-2-48" CONC GALS

| Elevation (feet) | Storage (cubic-feet) | Elevation (feet) | Storage (cubic-feet) | Elevation (feet) | Storage (cubic-feet) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 13.00 | 0 | 15.65 | 1,207 | 18.30 | 2,016 |
| 13.05 | 14 | 15.70 | 1,232 | 18.35 | 2,016 |
| 13.10 | 27 | 15.75 | 1,258 | 18.40 | 2,017 |
| 13.15 | 41 | 15.80 | 1,284 | 18.45 | 2,017 |
| 13.20 | 54 | 15.85 | 1,310 | 18.50 | 2,018 |
| 13.25 | 68 | 15.90 | 1,335 | 18.55 | 2,019 |
| 13.30 | 82 | 15.95 | 1,362 | 18.60 | 2,019 |
| 13.35 | 95 | 16.00 | 1,389 | 18.65 | 2,020 |
| 13.40 | 109 | 16.05 | 1,415 | 18.70 | 2,020 |
| 13.45 | 122 | 16.10 | 1,442 | 18.75 | 2,021 |
| 13.50 | 136 | 16.15 | 1,468 | 18.80 | 2,022 |
| 13.55 | 160 | 16.20 | 1,495 | 18.85 | 2,022 |
| 13.60 | 184 | 16.25 | 1,522 | 18.90 | 2,023 |
| 13.65 | 208 | 16.30 | 1,549 | 18.95 | 2,023 |
| 13.70 | 233 | 16.35 | 1,575 | 19.00 | 2,024 |
| 13.75 | 257 | 16.40 | 1,602 | 19.05 | 2,025 |
| 13.80 | 282 | 16.45 | 1,628 | 19.10 | 2,025 |
| 13.85 | 307 | 16.50 | 1,654 | 19.15 | 2,026 |
| 13.90 | 332 | 16.55 | 1,680 | 19.20 | 2,026 |
| 13.95 | 357 | 16.60 | 1,706 | 19.25 | 2,027 |
| 14.00 | 383 | 16.65 | 1,732 | 19.30 | 2,028 |
| 14.05 | 408 | 16.70 | 1,758 | 19.35 | 2,028 |
| 14.10 | 433 | 16.75 | 1,784 | 19.40 | 2,029 |
| 14.15 | 458 | 16.80 | 1,809 |  |  |
| 14.20 | 483 | 16.85 | 1,835 |  |  |
| 14.25 | 508 | 16.90 | 1,861 |  |  |
| 14.30 | 533 | 16.95 | 1,886 |  |  |
| 14.35 | 558 | 17.00 | 1,912 |  |  |
| 14.40 | 583 | 17.05 | 1,938 |  |  |
| 14.45 | 608 | 17.10 | 1,955 |  |  |
| 14.50 | 633 | 17.15 | 1,961 |  |  |
| 14.55 | 658 | 17.20 | 1,966 |  |  |
| 14.60 | 682 | 17.25 | 1,971 |  |  |
| 14.65 | 707 | 17.30 | 1,977 |  |  |
| 14.70 | 732 | 17.35 | 1,982 |  |  |
| 14.75 | 757 | 17.40 | 1,987 |  |  |
| 14.80 | 782 | 17.45 | 1,993 |  |  |
| 14.85 | 807 | 17.50 | 1,998 |  |  |
| 14.90 | 832 | 17.55 | 1,999 |  |  |
| 14.95 | 857 | 17.60 | 2,001 |  |  |
| 15.00 | 882 | 17.65 | 2,002 |  |  |
| 15.05 | 907 | 17.70 | 2,003 |  |  |
| 15.10 | 931 | 17.75 | 2,004 |  |  |
| 15.15 | 956 | 17.80 | 2,005 |  |  |
| 15.20 | 981 | 17.85 | 2,007 |  |  |
| 15.25 | 1,006 | 17.90 | 2,008 |  |  |
| 15.30 | 1,031 | 17.95 | 2,009 |  |  |
| 15.35 | 1,056 | 18.00 | 2,010 |  |  |
| 15.40 | 1,081 | 18.05 | 2,011 |  |  |
| 15.45 | 1,106 | 18.10 | 2,013 |  |  |
| 15.50 | 1,131 | 18.15 | 2,014 |  |  |
| 15.55 | 1,156 | 18.20 | 2,014 |  |  |
| 15.60 | 1,181 | 18.25 | 2,015 |  |  |

## Summary for Link 91: EX. SOUTH OUT



Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

## Link 91: EX. SOUTH OUT



## Summary for Link 92: PR. SOUTH OUT

Inflow Area $=\quad 48,211$ sf, $93.88 \%$ Impervious, Inflow Depth > 1.72" for 1-Yr Stamford event Inflow $=1.88$ cfs @ 12.07 hrs , Volume= 6,918 cf Primary $=1.88 \mathrm{cfs}$ @ 12.07 hrs , Volume $=\quad 6,918 \mathrm{cf}$, Atten= $0 \%$, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

## Link 92: PR. SOUTH OUT



## Summary for Link 95: EX, EAST OUT

Inflow Area = Inflow = Primary =


1,354 sf, $0.00 \%$ Impervious, Inflow Depth > 1.48" for 1-Yr Stamford event

Primary outflow $=$ Inflow, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$

## Link 95: EX, EAST OUT

Hydrograph


## Summary for Link 96: PR. EAST OUT

Inflow Area = Inflow = Primary =

1,251 sf, $\quad 0.00 \%$ Impervious, Inflow Depth > 1.48" for 1-Yr Stamford event 0.05 cfs @ 12.08 hrs , Volume=

154 cf
154 cf, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$

Primary outflow $=$ Inflow, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Link 96: PR. EAST OUT


## Summary for Link 97: EX, NORTH OUT

Inflow Area = Inflow =

801 sf, $93.51 \%$ Impervious, Inflow Depth > 2.63" for 1-Yr Stamford event Primary = 0.05 cfs @ 12.07 hrs, Volume=

175 cf
0.05 cfs @ 12.07 hrs , Volume=

175 cf, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$
Primary outflow $=$ Inflow, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$

## Link 97: EX, NORTH OUT



## Summary for Link 98: PR. NORTH OUT

Inflow Area =
Inflow =
Primary =

775 sf, $27.48 \%$ Impervious, Inflow Depth > 1.77" for 1-Yr Stamford event

Primary outflow $=$ Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

## Link 98: PR. NORTH OUT



Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

## Subcatchment 11: EX. SOUTH BASIN

Subcatchment 15: EX. EAST BASIN

Subcatchment 17: EX. NORTH BASIN

Subcatchment21: PR. SOUTH BYPASS

Subcatchment22: PR. SOUTH-1 BASIN

Subcatchment23: PR. SOUTH-2 BASIN

Subcatchment25: PR. EAST BASIN

## Subcatchment27: PR. NORTH BASIN

Pond 62: BMP-1-48" CONC GALS

Pond 63: BMP-2-48" CONC GALS

## Link 91: EX. SOUTH OUT

Link 92: PR. SOUTH OUT

## Link 95: EX, EAST OUT

Link 96: PR. EAST OUT

Link 97: EX, NORTH OUT

Link 98: PR. NORTH OUT

Runoff Area=48,082 sf $80.34 \%$ Impervious Runoff Depth $>3.04$ " $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=95.25$ Runoff $=3.82 \mathrm{cfs} 12,188 \mathrm{cf}$

Runoff Area $=1,354$ sf $0.00 \%$ Impervious Runoff Depth $>2.00$ " $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=84.00$ Runoff $=0.08 \mathrm{cfs} 226 \mathrm{cf}$

Runoff Area=801 sf $93.51 \%$ Impervious Runoff Depth>3.24" $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=97.09$ Runoff $=0.07 \mathrm{cfs} 216 \mathrm{cf}$

Runoff Area=27,749 sf 96.14\% Impervious Runoff Depth>3.28" $\mathrm{Tc}=5.0 \mathrm{~min} \quad \mathrm{CN}=97.46$ Runoff $=2.28 \mathrm{cfs} 7,592 \mathrm{cf}$

Runoff Area=11,610 sf $87.75 \%$ Impervious Runoff Depth $>3.15^{\prime \prime}$ $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=96.29$ Runoff $=0.94 \mathrm{cfs} 3,051 \mathrm{cf}$

Runoff Area=8,852 sf $94.84 \%$ Impervious Runoff Depth $>3.26$ " $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=97.28$ Runoff $=0.73 \mathrm{cfs} 2,407 \mathrm{cf}$

Runoff Area $=1,251$ sf $0.00 \%$ Impervious Runoff Depth $>2.00$ " $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=84.00$ Runoff $=0.07 \mathrm{cfs} 209 \mathrm{cf}$

Runoff Area=775 sf $27.48 \%$ Impervious Runoff Depth $>2.33$ " $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=87.85$ Runoff $=0.05 \mathrm{cfs} 150 \mathrm{cf}$

Peak Elev=12.55' Storage=2,127 cf Inflow=0.94 cfs $3,846 \mathrm{cf}$ Outflow=0.14 cfs 1,794 cf

Peak Elev=16.50' Storage=1,654 cf Inflow=0.73 cfs 2,407 cf Outflow=0.07 cfs 795 cf

Inflow=3.82 cfs 12,188 cf Primary $=3.82$ cfs 12,188 cf

Inflow=2.28 cfs 9,385 cf Primary $=2.28$ cfs 9,385 cf

Inflow=0.08 cfs 226 cf Primary $=0.08$ cfs 226 cf

Inflow=0.07 cfs 209 cf Primary $=0.07$ cfs 209 cf

Inflow=0.07 cfs 216 cf Primary $=0.07$ cfs 216 cf

Inflow=0.05 cfs 150 cf Primary $=0.05$ cfs 150 cf

Total Runoff Area $=100,474 \mathbf{s f}$ Runoff Volume $=26,040$ cf Average Runoff Depth $=3.11^{\prime \prime}$ $\mathbf{1 5 . 5 5 \%}$ Pervious $=\mathbf{1 5 , 6 2 1} \mathbf{s f} \mathbf{8 4 . 4 5 \%}$ Impervious $=84,853$ sf

Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

## Subcatchment 11: EX. SOUTH BASIN

Subcatchment 15: EX. EAST BASIN

Subcatchment 17: EX. NORTH BASIN

Subcatchment 21: PR. SOUTH BYPASS

Subcatchment22: PR. SOUTH-1 BASIN

Subcatchment23: PR. SOUTH-2 BASIN

Subcatchment25: PR. EAST BASIN

## Subcatchment27: PR. NORTH BASIN

Pond 62: BMP-1-48" CONC GALS

Pond 63: BMP-2-48" CONC GALS

## Link 91: EX. SOUTH OUT

Link 92: PR. SOUTH OUT

## Link 95: EX, EAST OUT

Link 96: PR. EAST OUT

Link 97: EX, NORTH OUT

Link 98: PR. NORTH OUT

Runoff Area=48,082 sf $80.34 \%$ Impervious Runoff Depth $>4.05{ }^{\prime \prime}$ $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=95.25$ Runoff $=5.00 \mathrm{cfs} 16,222 \mathrm{cf}$

Runoff Area $=1,354$ sf $0.00 \%$ Impervious Runoff Depth $>2.90^{\prime \prime}$ $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=84.00$ Runoff $=0.11 \mathrm{cfs} 328 \mathrm{cf}$

Runoff Area=801 sf $93.51 \%$ Impervious Runoff Depth>4.26" $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=97.09$ Runoff $=0.09 \mathrm{cfs} 284 \mathrm{cf}$

Runoff Area $=27,749$ sf $96.14 \%$ Impervious Runoff Depth>4.30" $\mathrm{Tc}=5.0 \mathrm{~min} \quad \mathrm{CN}=97.46$ Runoff $=2.95 \mathrm{cfs} 9,940 \mathrm{cf}$

Runoff Area=11,610 sf $87.75 \%$ Impervious Runoff Depth $>4.17^{\prime \prime}$ $\mathrm{Tc}=5.0 \mathrm{~min} \quad \mathrm{CN}=96.29$ Runoff $=1.22 \mathrm{cfs} 4,030 \mathrm{cf}$

Runoff Area=8,852 sf $94.84 \%$ Impervious Runoff Depth>4.28" Tc=5.0 min CN=97.28 Runoff=0.94 cfs $3,156 \mathrm{cf}$

Runoff Area $=1,251$ sf $0.00 \%$ Impervious Runoff Depth $>2.90$ " $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=84.00$ Runoff $=0.10 \mathrm{cfs} 303 \mathrm{cf}$

Runoff Area=775 sf $27.48 \%$ Impervious Runoff Depth $>3.27$ " $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=87.85$ Runoff $=0.07 \mathrm{cfs} 211 \mathrm{cf}$

Peak Elev=12.82' Storage=2,304 cf Inflow=1.22 cfs 5,571 cf Outflow=0.78 cfs 3,514 cf

Peak Elev=16.67' Storage=1,742 cf Inflow=0.94 cfs 3,156 cf Outflow=0.38 cfs 1,541 cf

Inflow=5.00 cfs 16,222 cf Primary=5.00 cfs 16,222 cf

Inflow=2.95 cfs 13,454 cf Primary=2.95 cfs 13,454 cf

Inflow=0.11 cfs 328 cf Primary $=0.11$ cfs 328 cf

Inflow=0.10 cfs 303 cf Primary $=0.10$ cfs 303 cf

Inflow=0.09 cfs 284 cf Primary=0.09 cfs 284 cf

Inflow=0.07 cfs 211 cf Primary=0.07 cfs 211 cf

Total Runoff Area $=100,474 \mathrm{sf}$ Runoff Volume $=34,473$ cf Average Runoff Depth $=4.12$ " $\mathbf{1 5 . 5 5 \%}$ Pervious $=\mathbf{1 5 , 6 2 1} \mathbf{~ s f} \mathbf{8 4 . 4 5 \%}$ Impervious $=84,853$ sf

Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

## Subcatchment 11: EX. SOUTH BASIN

Subcatchment 15: EX. EAST BASIN

Subcatchment 17: EX. NORTH BASIN

Subcatchment21: PR. SOUTH BYPASS

Subcatchment22: PR. SOUTH-1 BASIN

Subcatchment23: PR. SOUTH-2 BASIN

Subcatchment25: PR. EAST BASIN

## Subcatchment27: PR. NORTH BASIN

Pond 62: BMP-1-48" CONC GALS

Pond 63: BMP-2-48" CONC GALS

## Link 91: EX. SOUTH OUT

Link 92: PR. SOUTH OUT

## Link 95: EX, EAST OUT

Link 96: PR. EAST OUT

Link 97: EX, NORTH OUT

Link 98: PR. NORTH OUT

Runoff Area $=48,082$ sf $80.34 \%$ Impervious Runoff Depth>4.89" Tc=5.0 min CN=95.25 Runoff=5.97 cfs 19,596 cf

Runoff Area $=1,354$ sf $\quad 0.00 \%$ Impervious Runoff Depth $>3.68$ " Tc=5.0 min $\mathrm{CN}=84.00$ Runoff=0.14 cfs 415 cf

Runoff Area=801 sf 93.51\% Impervious Runoff Depth>5.10" Tc=5.0 min CN=97.09 Runoff=0.10 cfs 341 cf

Runoff Area=27,749 sf 96.14\% Impervious Runoff Depth>5.15" $\mathrm{Tc}=5.0 \mathrm{~min} \quad \mathrm{CN}=97.46$ Runoff=3.51 cfs $11,899 \mathrm{cf}$

Runoff Area $=11,610$ sf $87.75 \%$ Impervious Runoff Depth $>5.01$ " $\mathrm{Tc}=5.0 \mathrm{~min} \quad \mathrm{CN}=96.29$ Runoff=1.46 cfs $4,847 \mathrm{cf}$

Runoff Area=8,852 sf 94.84\% Impervious Runoff Depth>5.12" $\mathrm{Tc}=5.0 \mathrm{~min} \quad \mathrm{CN}=97.28$ Runoff=1.12 cfs $3,780 \mathrm{cf}$

Runoff Area=1,251 sf $0.00 \%$ Impervious Runoff Depth>3.68" $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=84.00$ Runoff= 0.13 cfs 384 cf

Runoff Area=775 sf 27.48\% Impervious Runoff Depth $>4.08$ " $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=87.85$ Runoff=0.09 cfs 263 cf

Peak Elev=13.30' Storage=2,504 cf Inflow=2.02 cfs 7,011 cf Outflow=1.53 cfs 4,951 cf

Peak Elev=16.83' Storage=1,823 cf Inflow=1.12 cfs 3,780 cf Outflow $=0.79$ cfs 2,164 cf

Inflow=5.97 cfs 19,596 cf Primary $=5.97$ cfs 19,596 cf

Inflow $=4.19$ cfs 16,851 cf Primary $=4.19 \mathrm{cfs} 16,851 \mathrm{cf}$

Inflow=0.14 cfs 415 cf Primary $=0.14$ cfs 415 cf

Inflow=0.13 cfs 384 cf Primary= 0.13 cfs 384 cf

Inflow $=0.10$ cfs 341 cf Primary $=0.10$ cfs 341 cf

Inflow=0.09 cfs 263 cf Primary $=0.09$ cfs 263 cf

Total Runoff Area $=100,474 \mathrm{sf}$ Runoff Volume $=41,527$ cf Average Runoff Depth $=4.96$ " $\mathbf{1 5 . 5 5 \%}$ Pervious $=\mathbf{1 5 , 6 2 1}$ sf $\mathbf{8 4 . 4 5 \%}$ Impervious $=84,853$ sf

Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

## Subcatchment 11: EX. SOUTH BASIN

Subcatchment 15: EX. EAST BASIN

Subcatchment 17: EX. NORTH BASIN

Subcatchment21: PR. SOUTH BYPASS

Subcatchment22: PR. SOUTH-1 BASIN

Subcatchment 23: PR. SOUTH-2 BASIN

Subcatchment 25: PR. EAST BASIN

## Subcatchment27: PR. NORTH BASIN

Pond 62: BMP-1-48" CONC GALS

Pond 63: BMP-2-48" CONC GALS

## Link 91: EX. SOUTH OUT

Link 92: PR. SOUTH OUT

## Link 95: EX, EAST OUT

Link 96: PR. EAST OUT

Link 97: EX, NORTH OUT

Link 98: PR. NORTH OUT

Runoff Area=48,082 sf $80.34 \%$ Impervious Runoff Depth $>6.04$ " $\mathrm{Tc}=5.0 \mathrm{~min} \quad \mathrm{CN}=95.25$ Runoff $=7.30 \mathrm{cfs} 24,213 \mathrm{cf}$

Runoff Area $=1,354$ sf $0.00 \%$ Impervious Runoff Depth $>4.77^{\prime \prime}$ $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=84.00$ Runoff $=0.18 \mathrm{cfs} 538 \mathrm{cf}$

Runoff Area=801 sf $93.51 \%$ Impervious Runoff Depth>6.26" $\mathrm{Tc}=5.0 \mathrm{~min} \quad \mathrm{CN}=97.09$ Runoff $=0.12 \mathrm{cfs} 418 \mathrm{cf}$

Runoff Area $=27,749$ sf $96.14 \%$ Impervious Runoff Depth $>6.30^{\prime \prime}$ $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=97.46$ Runoff $=4.27 \mathrm{cfs} 14,575 \mathrm{cf}$

Runoff Area=11,610 sf $87.75 \%$ Impervious Runoff Depth $>6.17^{\prime \prime}$ $\mathrm{Tc}=5.0 \mathrm{~min} \quad \mathrm{CN}=96.29$ Runoff $=1.78 \mathrm{cfs} 5,965 \mathrm{cf}$

Runoff Area=8,852 sf $94.84 \%$ Impervious Runoff Depth>6.28" $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=97.28$ Runoff=1.36 cfs $4,634 \mathrm{cf}$

Runoff Area $=1,251$ sf $0.00 \%$ Impervious Runoff Depth $>4.77^{\prime \prime}$ $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=84.00$ Runoff $=0.16 \mathrm{cfs} 497 \mathrm{cf}$

Runoff Area=775 sf $27.48 \%$ Impervious Runoff Depth $>5.19$ " $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=87.85$ Runoff $=0.11 \mathrm{cfs} 335 \mathrm{cf}$

Peak Elev=14.12' Storage=2,559 cf Inflow=2.80 cfs 8,978 cf Outflow=2.80 cfs 6,915 cf

Peak Elev=17.00' Storage=1,912 cf Inflow=1.36 cfs 4,634 cf Outflow=1.12 cfs 3,014 cf

Inflow=7.30 cfs 24,213 cf Primary $=7.30$ cfs $24,213 \mathrm{cf}$

Inflow=7.12 cfs 21,490 cf Primary=7.12 cfs 21,490 cf

Inflow=0.18 cfs 538 cf Primary $=0.18$ cfs 538 cf

Inflow=0.16 cfs 497 cf Primary $=0.16$ cfs 497 cf

Inflow=0.12 cfs 418 cf Primary=0.12 cfs 418 cf

Inflow=0.11 cfs 335 cf Primary=0.11 cfs 335 cf

Total Runoff Area $=100,474$ sf Runoff Volume $=51,175$ cf Average Runoff Depth $=6.11$ " $\mathbf{1 5 . 5 5 \%}$ Pervious $=\mathbf{1 5 , 6 2 1} \mathbf{s f} \mathbf{8 4 . 4 5 \%}$ Impervious $=84,853$ sf

Time span $=0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}, 2401$ points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

## Subcatchment 11: EX. SOUTH BASIN

Subcatchment 15: EX. EAST BASIN

Subcatchment 17: EX. NORTH BASIN

Subcatchment21: PR. SOUTH BYPASS

Subcatchment22: PR. SOUTH-1 BASIN

Subcatchment23: PR. SOUTH-2 BASIN

Subcatchment25: PR. EAST BASIN

## Subcatchment27: PR. NORTH BASIN

Pond 62: BMP-1-48" CONC GALS

Pond 63: BMP-2-48" CONC GALS

## Link 91: EX. SOUTH OUT

Link 92: PR. SOUTH OUT

## Link 95: EX, EAST OUT

Link 96: PR. EAST OUT

Link 97: EX, NORTH OUT

Link 98: PR. NORTH OUT

Runoff Area=48,082 sf $80.34 \%$ Impervious Runoff Depth>6.90" Tc=5.0 min CN=95.25 Runoff=8.28 cfs 27,641 cf

Runoff Area $=1,354$ sf $\quad 0.00 \%$ Impervious Runoff Depth $>5.58$ " Tc=5.0 min $C N=84.00$ Runoff=0.21 cfs 630 cf

Runoff Area=801 sf 93.51\% Impervious Runoff Depth>7.12" $\mathrm{Tc}=5.0 \mathrm{~min} \quad \mathrm{CN}=97.09$ Runoff=0.14 cfs 475 cf

Runoff Area=27,749 sf 96.14\% Impervious Runoff Depth>7.16" $\mathrm{Tc}=5.0 \mathrm{~min} \quad \mathrm{CN}=97.46$ Runoff=4.83 cfs $16,560 \mathrm{cf}$

Runoff Area=11,610 sf $87.75 \%$ Impervious Runoff Depth $>7.02$ " $\mathrm{Tc}=5.0 \mathrm{~min} \quad \mathrm{CN}=96.29$ Runoff=2.01 cfs 6,794 cf

Runoff Area=8,852 sf 94.84\% Impervious Runoff Depth>7.14" Tc=5.0 min CN=97.28 Runoff=1.54 cfs 5,267 cf

Runoff Area=1,251 sf $0.00 \%$ Impervious Runoff Depth $>5.58$ " $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=84.00$ Runoff= 0.19 cfs 582 cf

Runoff Area=775 sf 27.48\% Impervious Runoff Depth>6.03" $\mathrm{Tc}=5.0 \mathrm{~min} \mathrm{CN}=87.85$ Runoff= 0.12 cfs 389 cf

Peak Elev=14.16' Storage=2,561 cf Inflow=3.17 cfs 10,438 cf Outflow=3.16 cfs 8,373 cf

Peak Elev=17.11' Storage=1,957 cf Inflow=1.54 cfs 5,267 cf Outflow=1.29 cfs 3,645 cf

Inflow=8.28 cfs 27,641 cf Primary $=8.28 \mathrm{cfs} 27,641 \mathrm{cf}$

Inflow $=7.97$ cfs 24,933 cf Primary=7.97cfs 24,933 cf

Inflow=0.21 cfs 630 cf Primary $=0.21$ cfs 630 cf

Inflow=0.19 cfs 582 cf Primary $=0.19$ cfs 582 cf

Inflow=0.14 cfs 475 cf Primary $=0.14$ cfs 475 cf

Inflow=0.12 cfs 389 cf Primary $=0.12$ cfs 389 cf

Total Runoff Area $=100,474 \mathbf{s f}$ Runoff Volume $=58,339$ cf Average Runoff Depth $=6.97$ " $\mathbf{1 5 . 5 5 \%}$ Pervious $=\mathbf{1 5 , 6 2 1}$ sf $\mathbf{8 4 . 4 5 \%}$ Impervious $=84,853$ sf
APPENDIX - C

Water Quality Volume Calculation

| Project: 819 East Main St, Stamford, CT | Date: 02/03/2022 |
| :--- | :--- |

$$
\mathrm{WQV}=\mathrm{Water} \text { Quality Volume }=\left(1.0^{*} \mathrm{R}^{*} \mathrm{~A}\right) / 12
$$

|  | Area $=$ | 1.1533 acres |
| :---: | ---: | ---: |
| Proposed | Impervious Area $=$ | 1.0439 acres |
| Conditions of | $\mathrm{I}=$ | $90.5 \%{ }^{\mathrm{a}}$ |
| Whole Site | $\mathrm{R}=$ | $0.865{ }^{\mathrm{b}}$ |
|  | $\mathrm{WQV}=$ | $0.0831 \mathrm{ac}. \mathrm{ft.}^{\mathrm{c}}$ |
|  | $\mathrm{WQV}=$ | $3,620 \mathrm{ft.}^{3}$ |
|  |  |  |

*Required $\mathrm{WQV}=\mid \quad 3,620 \mathrm{ft}^{3}$

* Retainage of full WQV is required per
the City of Stamford Stormwater
Drainage Manual Standard 1 of section
2.4 Stormwater Management Standards
${ }^{a}$ I=Percent Impervious Coverage
${ }^{\mathrm{b}}$ R=0.05+0.009(I); Volumetric runoff Coefficient, Equation taken from 2004 Connecticut Stormwater Quality Manual section 7.4.1
c WQV=(1.0"xRxA)/12; Water Quality Volume, Equation ttaken from 2004 Connecticut Stormwater Quality Manual section 7.4.1



## APPENDIX - D



Conveyance Calculations
Rational Method - Peak Rate of Runoff - 25 Year Storm Event

| ROOF to EX.MH | Basin Description |  |  |  | Drainage Path |  |  |  | $\mathrm{Q}=\mathrm{ACI}$ (cfs) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Acres | C | Description | AC | Length | $\Delta \mathrm{H}$ | Slope \% | Description | Time (min) | Intensity (in/hr) | $\mathrm{Q}_{25}$ |
|  | 0.572 | 0.95 | Immpervivews | 0.543 |  |  |  |  |  |  |  |
|  | 0.000 | 0.30 | Pervious | 0.000 |  |  |  |  |  |  |  |
|  | 0.572 |  | Total | 0.543 |  |  |  |  | 5 | 7.6 | 4.13 |
|  | $\mathrm{Q}_{25}$ (cfs) | Pipe Size <br> (inches) | Pipe Length (feet) | Roughness Coefficient <br> (n) | Material | Slope <br> (ft/ft) | $\mathrm{Q}_{\text {full }}(\mathrm{cfs})$ | $\mathrm{Q}_{25} / \mathrm{Q}_{\text {full }}(\%)$ | Pipe Flow <br> Man | Capacity ings Equa |  |
|  | 4.13 | 12 | 60 | 0.011 | PVC | 0.0150 | 5.17 | 79.9\% | $Q_{\text {fill }}=$ | n) ${ }^{*}$ |  |


| MH\#1 to <br> EX.MH | Basin Description |  |  |  | Drainage Path |  |  |  | Q = ACI (cfs) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Acres | C | Description | AC | Length | $\Delta \mathrm{H}$ | Slope \% | Description | Time (min) | Intensity (in/hr) | $\mathrm{Q}_{25}$ |
|  | 0.000 | 0.95 | Impervious | 0.000 |  |  |  |  |  |  |  |
|  | 0.000 | 0.30 | Pervious | 0.000 |  |  |  |  |  |  |  |
|  | 0.000 |  | Total | 0.000 |  |  |  |  | HydroCA | Report | 2.80 |
|  | $\mathrm{Q}_{25}$ (cfs) | Pipe Size <br> (inches) | Pipe Length (feet) | Roughness Coefficient <br> (n) | Material | Slope <br> (ft/ft) | $\mathrm{Q}_{\text {full }}(\mathrm{cfs})$ | $\mathrm{Q}_{25} / \mathrm{Q}_{\text {full }}(\%)$ | Pipe Fl <br> Ma | Capacit <br> ings Equ |  |
|  | 2.80 | 12 | 180 | 0.011 | PVC | 0.0100 | 4.22 | 66"3\% | $Q_{\text {full }}=$ | ) ${ }^{\text {A }}$ S |  |


| $\begin{gathered} \text { MH\#2to } \\ \text { MH\#1 } \end{gathered}$ | Basin Description |  |  |  | Drainage Path |  |  |  | Q = ACI (cfs) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Acres | C | Description | AC | Length | $\Delta \mathrm{H}$ | Slope \% | Description | Time (min) | Intensity (in/hr) | $\mathrm{Q}_{25}$ |
|  | 0.000 | 0.95 | Imperswowisw | 0.000 |  |  |  |  |  |  |  |
|  | 0.000 | 0.30 | Pervious | 0.000 |  |  |  |  |  |  |  |
|  | 0.000 |  | Total | 0.000 |  |  |  |  | HydroCA | Report | 2.80 |
|  | $\mathrm{Q}_{25}(\mathrm{cfs})$ | Pipe Size <br> (inches) | Pipe Length (feet) | Roughness Coefficient <br> (n) | Material | Slope <br> (ft/ft) | $\mathrm{Q}_{\text {full }}(\mathrm{cfs})$ | $\mathrm{Q}_{25} / \mathrm{Q}_{\text {full }}(\%)$ | Pipe Flo <br> Man | Capacity ings Equa |  |
|  | 2.80 | 12 | 40 | 0.011 | PVC | 0.020 | 5.97 | 46.9\%"* | $\mathrm{Q}_{\text {full }}=($ | n)*A* |  |




## APPENDIX - E




FlexTable: Conduit Table

| Label | Start Node | Stop Node | Elevation Ground (Start) (ft) | Hydraulic Grade Line (In) (ft) | Invert (Start) <br> (ft) | $\begin{array}{\|l\|} \hline \text { Elevation Ground } \\ \text { (Stop) } \\ \text { (ft) } \\ \hline \end{array}$ | Hydraulic Grade Line (Out) <br> (ft) | Invert (Stop) <br> (ft) | Length (User Defined) (ft) | $\begin{gathered} \text { Slope } \\ \left(\begin{array}{c} \text { Calculated) } \\ (\mathrm{ft} / \mathrm{ft}) \end{array}\right. \end{gathered}$ | Diameter <br> (in) | Material | $\begin{aligned} & \text { Flow } \\ & \text { (cfs) } \end{aligned}$ | Capacity (Full Flow) (cfs) | Velocity ( $\mathrm{ft} / \mathrm{s}$ ) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P-1A | CB-1A | OUT-1 | 17.18 | 14.03 | 13.30 | 17.48 | 13.44 | 12.88 | 19.0 | 0.022 | 12.0 | Concrete | 2.94 | 5.30 | 6.92 |
| P-1B | CB-1B | OUT-1 | 17.30 | 13.89 | 13.20 | 17.48 | 13.39 | 12.90 | 6.5 | 0.046 | 12.0 | Concrete | 2.59 | 7.65 | 8.80 |
| P-2 | MH-2 | OUT-1 | 16.64 | 23.46 | 6.84 | 17.48 | 7.40 | 5.40 | 134.0 | 0.011 | 24.0 | Concrete | 78.31 | 23.45 | 24.93 |
| P-3 | MH-3 | MH-2 | 15.50 | 21.29 | 7.50 | 16.64 | 16.64 | 6.90 | 164.0 | 0.004 | 18.0 | Concrete | 17.69 | 6.35 | 10.01 |
| P-3A | CB-3A | MH-3 | 15.38 | 15.52 | 11.60 | 15.50 | 15.50 | 11.40 | 5.0 | 0.040 | 12.0 | Concrete | 2.18 | 7.13 | 2.78 |
| P-3B | СB-3B | MH-3 | 14.90 | 15.50 | 11.80 | 15.50 | 15.50 | 11.40 | 20.0 | 0.020 | 12.0 | Concrete | 0.27 | 5.04 | 0.35 |
| P-4 | MH-4 | MH-3 | 13.70 | 23.55 | 8.90 | 15.50 | 15.50 | 7.50 | 355.0 | 0.004 | 18.0 | Concrete | 15.82 | 6.60 | 8.95 |
| P-4A | CB-4A | MH-4 | 13.18 | 13.73 | 9.10 | 13.70 | 13.70 | 8.90 | 8.0 | 0.025 | 15.0 | Concrete | 3.94 | 10.21 | 3.21 |
| P-4B | СB-4B | MH-4 | 13.25 | 13.77 | 9.20 | 13.70 | 13.70 | 8.90 | 15.0 | 0.020 | 15.0 | Concrete | 4.32 | 9.14 | 3.52 |
| P-5 | MH-5 | MH-4 | 14.41 | 14.12 | 9.20 | 13.70 | 13.70 | 8.95 | 98.0 | 0.003 | 15.0 | Concrete | 4.23 | 3.26 | 3.44 |
| P-6 | MH-6 | MH-5 | 15.30 | 14.41 | 10.00 | 14.41 | 14.12 | 9.30 | 132.0 | 0.005 | 15.0 | Concrete | 3.03 | 4.70 | 2.47 |
| P-6A | CB-6A | MH-6 | 14.78 | 14.43 | 10.30 | 15.30 | 14.41 | 10.10 | 10.0 | 0.020 | 12.0 | Concrete | 1.79 | 5.04 | 2.28 |
| P-6B | CB-6B | MH-6 | 14.83 | 14.44 | 10.40 | 15.30 | 14.41 | 10.10 | 15.0 | 0.020 | 12.0 | Concrete | 1.55 | 5.04 | 1.97 |
| P-7 | MH-7 | MH-2 | 11.75 | 30.45 | 7.65 | 16.64 | 16.64 | 7.05 | 172.0 | 0.003 | 24.0 | Concrete | 64.11 | 13.36 | 20.41 |
| P-7A | CB-7A | MH-7 | 11.78 | 11.76 | 8.60 | 11.75 | 11.75 | 8.25 | 39.0 | 0.009 | 15.0 | Concrete | 0.78 | 6.12 | 0.63 |
| P-7B | СВ-7B | MH-7 | 11.60 | 11.79 | 7.90 | 11.75 | 11.75 | 7.75 | 20.0 | 0.008 | 15.0 | Concrete | 2.84 | 5.59 | 2.31 |
| P-7C | СВ-7C | CB-7B | 11.45 | 11.61 | 8.15 | 11.60 | 11.60 | 7.90 | 8.0 | 0.031 | 15.0 | Concrete | 2.37 | 11.42 | 1.93 |
| P-7D | CB-7D | MH-7 | 11.50 | 11.86 | 8.50 | 11.75 | 11.75 | 8.05 | 5.0 | 0.090 | 15.0 | Concrete | 9.67 | 19.38 | 7.88 |
| P-8 | MH-8 | MH-7 | 12.39 | 11.95 | 8.50 | 11.75 | 11.75 | 8.10 | 39.0 | 0.010 | 12.0 | PVC | 3.30 | 4.69 | 4.20 |
| P-9 | MH-9 | MH-7 | 11.35 | 24.43 | 7.95 | 11.75 | 11.75 | 7.70 | 58.0 | 0.004 | 18.0 | Concrete | 49.11 | 6.90 | 27.79 |
| P-9A | CB-9A | MH-9 | 10.80 | 11.38 | 8.20 | 11.35 | 11.35 | 8.10 | 4.0 | 0.025 | 12.0 | Concrete | 2.87 | 5.63 | 3.65 |
| P-98 | Св-9B | MH-9 | 10.84 | 11.88 | 8.35 | 11.35 | 11.35 | 8.05 | 16.0 | 0.019 | 12.0 | PVC | 8.42 | 6.34 | 10.72 |
| P-10 | MH-10 | MH-9 | 11.46 | 24.00 | 8.30 | 11.35 | 11.35 | 8.00 | 96.0 | 0.003 | 18.0 | Concrete | 38.13 | 5.87 | 21.58 |
| P-11 | MH-11 | MH-10 | 14.20 | 11.69 | 9.00 | 11.46 | 11.46 | 8.60 | 43.0 | 0.009 | 12.0 | Concrete | 2.58 | 3.44 | 3.29 |
| P-11A | CB-11A | MH-11 | 14.66 | 11.72 | 10.70 | 14.20 | 11.69 | 10.50 | 6.0 | 0.033 | 12.0 | Concrete | 2.58 | 6.50 | 3.29 |
| P-12 | MH-12 | MH-10 | 13.43 | 31.72 | 9.10 | 11.46 | 11.46 | 8.40 | 170.0 | 0.004 | 18.0 | Concrete | 36.26 | 6.74 | 20.52 |
| P-12A | CB-12A | MH-12 | 13.32 | 13.97 | 9.70 | 13.43 | 13.43 | 9.30 | 31.0 | 0.013 | 15.0 | Concrete | 8.50 | 7.34 | 6.93 |
| P-12B | CB-12B | MH-12 | 15.00 | 13.64 | 10.90 | 13.43 | 13.43 | 9.50 | 59.0 | 0.024 | 15.0 | Concrete | 3.90 | 9.95 | 3.18 |
| P-13 | MH-13 | MH-12 | 14.80 | 19.44 | 10.80 | 13.43 | 13.43 | 9.20 | 114.0 | 0.014 | 18.0 | Concrete | 24.13 | 12.44 | 13.65 |
| P-14 | MH-14 | MH-13 | 17.83 | 20.98 | 13.50 | 14.80 | 14.80 | 10.90 | 116.0 | 0.022 | 18.0 | Concrete | 24.24 | 15.73 | 13.72 |
| P-15 | MH-15 | MH-14 | 19.23 | 20.18 | 14.10 | 17.83 | 17.83 | 13.60 | 44.0 | 0.011 | 18.0 | Concrete | 24.28 | 11.20 | 13.74 |
| P-15A | CB-15A | MH-15 | 19.80 | 20.47 | 16.00 | 19.23 | 19.23 | 15.10 | 30.0 | 0.030 | 15.0 | Concrete | 13.12 | 11.19 | 10.69 |
| P-15B | CB-15B | MH-15 | 20.43 | 19.34 | 17.20 | 19.23 | 19.23 | 14.80 | 51.0 | 0.047 | 15.0 | Concrete | 3.06 | 14.01 | 2.49 |
| P-15C | CB-15C | MH-15 | 41.10 | 20.67 | 20.00 | 19.23 | 19.23 | 14.30 | 53.0 | 0.108 | 15.0 | Concrete | 2.75 | 21.18 | 11.90 |
| P-16 | MH-16 | MH-15 | 20.20 | 19.96 | 15.00 | 19.23 | 19.23 | 14.20 | 79.0 | 0.010 | 15.0 | Concrete | 6.21 | 6.50 | 5.06 |
| P-16A | CB-16A | MH-16 | 19.00 | 20.88 | 15.90 | 20.20 | 19.96 | 15.10 | 78.0 | 0.010 | 12.0 | Concrete | 3.87 | 3.61 | 4.93 |
| P-16B | CB-16B | CB-16A | 19.20 | 19.13 | 16.40 | 19.00 | 19.00 | 16.00 | 27.0 | 0.015 | 12.0 | Concrete | 2.51 | 4.34 | 3.20 |

FlexTable: Catch Basin Table

| Label | Elevation (Ground) (ft) | Hydraulic Grade Line (Out) (ft) | Is Overflowing? | $\begin{gathered} \text { Flow } \\ \text { (Captured) } \\ \text { (cfs) } \end{gathered}$ | $\begin{aligned} & \hline \text { Flow (Additional } \\ & \text { Carryover) } \\ & \text { (cfs) } \end{aligned}$ | $\begin{aligned} & \text { Flow (Total } \\ & \text { Out) } \\ & \text { (cfs) } \end{aligned}$ | Elevation (Invert) (ft) | Is Surcharged? | Inlet C | $\begin{gathered} \hline \text { Local CA } \\ \left(\mathrm{ft}^{2}\right) \end{gathered}$ | Structure Type | $\begin{aligned} & \text { Length } \\ & (\mathrm{ft}) \end{aligned}$ | Width (ft) | Inlet Location | $\begin{aligned} & \text { Longitudinal } \\ & \text { Slope (Inlet) } \end{aligned}$ (ft/ft) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| CB-1A | 17.18 | 14.03 | False | 2.94 | 0.00 | 2.94 | 11.30 | False | 0.690 | 20,807.0 | Box Structure | 5.40 | 4.40 | On Grade | 0.011 |
| CB-1B | 17.30 | 13.89 | False | 2.59 | 0.00 | 2.59 | 11.20 | False | 0.755 | 18,380.5 | Box Structure | 5.40 | 4.40 | On Grade | 0.011 |
| CB-3A | 15.38 | 15.38 | True | 2.18 | 0.00 | 2.18 | 9.40 | True | 0.625 | 15,456.3 | Box Structure | 5.40 | 4.40 | On Grade | 0.004 |
| СВ-3B | 14.90 | 14.90 | True | 0.27 | 0.00 | 0.27 | 9.80 | True | 0.950 | 1,543.7 | Box Structure | 5.40 | 4.40 | On Grade | 0.004 |
| CB-4A | 13.18 | 13.18 | True | 3.94 | 0.00 | 3.94 | 7.10 | True | 0.495 | 27,909.1 | Box Structure | 5.40 | 4.40 | On Grade | 0.004 |
| CB-4B | 13.25 | 13.25 | True | 4.32 | 0.00 | 4.32 | 7.20 | True | 0.950 | 24,538.5 | Box Structure | 5.40 | 4.40 | On Grade | 0.004 |
| CB-6A | 14.78 | 14.43 | False | 1.79 | 0.00 | 1.79 | 8.30 | True | 0.528 | 12,703.3 | Box Structure | 5.40 | 4.40 | On Grade | 0.005 |
| CB-6B | 14.83 | 14.44 | False | 1.55 | 0.00 | 1.55 | 8.40 | True | 0.950 | 8,787.5 | Box Structure | 5.40 | 4.40 | On Grade | 0.005 |
| CB-7A | 11.78 | 11.76 | False | 0.78 | 0.00 | 0.78 | 6.65 | True | 0.495 | 4,416.4 | Box Structure | 5.40 | 4.40 | On Grade | 0.009 |
| CB-7B | 11.60 | 11.60 | True | 0.48 | 0.00 | 2.84 | 5.90 | True | 0.950 | 2,745.5 | Box Structure | 5.40 | 4.40 | In Sag |  |
| CB-7C | 11.45 | 11.45 | True | 2.37 | 0.00 | 2.37 | 6.15 | True | 0.950 | 13,448.2 | Box Structure | 5.40 | 4.40 | In Sag |  |
| CB-7D | 11.50 | 11.50 | True | 9.67 | 7.30 | 9.67 | 6.50 | True | 0.885 | 16,776.9 | Box Structure | 5.40 | 4.40 | In Sag |  |
| CB-9A | 10.80 | 10.80 | True | 2.87 | 0.00 | 2.87 | 6.20 | True | 0.495 | 20,313.8 | Box Structure | 5.40 | 4.40 | In Sag |  |
| Св-9B | 10.84 | 10.84 | True | 8.42 | 0.00 | 8.42 | 6.35 | True | 0.788 | 59,663.4 | Box Structure | 5.40 | 4.40 | In Sag |  |
| CB-11A | 14.66 | 11.72 | False | 2.58 | 0.00 | 2.58 | 8.70 | True | 0.885 | 14,693.7 | Box Structure | 5.40 | 4.40 | On Grade | 0.010 |
| CB-12A | 13.32 | 13.32 | True | 8.50 | 0.00 | 8.50 | 7.70 | True | 0.690 | 60,211.5 | Box Structure | 5.40 | 4.40 | On Grade | 0.010 |
| CB-12B | 15.00 | 13.64 | False | 3.90 | 0.00 | 3.90 | 8.90 | True | 0.820 | 27,611.0 | Box Structure | 5.40 | 4.40 | On Grade | 0.010 |
| CB-15A | 19.80 | 19.80 | True | 13.12 | 0.00 | 13.12 | 14.00 | True | 0.788 | 92,914.8 | Box Structure | 10.80 | 4.40 | On Grade | 0.012 |
| CB-15B | 20.43 | 19.34 | False | 3.06 | 0.00 | 3.06 | 15.20 | True | 0.820 | 17,393.8 | Box Structure | 5.40 | 4.40 | On Grade | 0.012 |
| CB-15C | 41.10 | 20.67 | False | 2.75 | 0.00 | 2.75 | 20.00 | False | 0.820 | 15,608.7 | Box Structure | 10.80 | 4.40 | On Grade | 0.060 |
| CB-16A | 19.00 | 19.00 | True | 1.38 | 0.00 | 3.87 | 13.90 | True | 0.755 | 7,865.6 | Box Structure | 5.40 | 4.40 | In Sag |  |
| CB-16B | 19.20 | 19.13 | False | 2.51 | 0.00 | 2.51 | 14.40 | True | 0.625 | 14,271.3 | Box Structure | 5.40 | 4.40 | In Sag |  |

FlexTable: Manhole Table

| Label | Elevation (Ground) (ft) | Hydraulic Grade Line (Out) <br> (ft) | Is Overflowing? | $\begin{aligned} & \hline \text { Flow (Known) } \\ & \text { (cfs) } \end{aligned}$ | $\begin{aligned} & \hline \text { Flow (Total Out) } \\ & \text { (cfs) } \end{aligned}$ | Elevation (Invert) (ft) | Is Surcharged? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| MH-2 | 16.64 | 16.64 | True | 0.00 | 78.31 | 6.84 | True |
| MH-3 | 15.50 | 15.50 | True | 0.00 | 17.69 | 6.90 | True |
| MH-4 | 13.70 | 13.70 | True | 0.00 | 15.82 | 8.90 | True |
| MH-5 | 14.41 | 14.12 | False | 0.00 | 4.23 | 9.20 | True |
| MH-6 | 15.30 | 14.41 | False | 0.00 | 3.03 | 10.00 | True |
| MH-7 | 11.75 | 11.75 | True | 0.00 | 64.11 | 7.65 | True |
| MH-8 | 12.39 | 11.95 | False | 0.00 | 3.30 | 8.50 | True |
| MH-9 | 11.35 | 11.35 | True | 0.00 | 49.11 | 7.95 | True |
| MH-10 | 11.46 | 11.46 | True | 0.00 | 38.13 | 8.30 | True |
| MH-11 | 14.20 | 11.69 | False | 0.00 | 2.58 | 9.00 | True |
| MH-12 | 13.43 | 13.43 | True | 0.00 | 36.26 | 9.10 | True |
| MH-13 | 14.80 | 14.80 | True | 0.00 | 24.13 | 10.80 | True |
| MH-14 | 17.83 | 17.83 | True | 0.00 | 24.24 | 13.50 | True |
| MH-15 | 19.23 | 19.23 | True | 0.00 | 24.28 | 14.10 | True |
| MH-16 | 20.20 | 19.96 | False | 0.00 | 6.21 | 15.00 | True |

FlexTable: Outfall Table

| Label | Elevation <br> (Ground) <br> (ft) | Elevation <br> (Invert) <br> (ft) | Elevation (User <br> Defined <br> Tailwater) <br> (ft) | Boundary <br> Condition Type | Hydraulic Grade <br> (ft) | Flow (Total Out) <br> (cfs) | Notes |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| OUT-1 | 17.48 | 5.40 | 6.40 | User Defined <br> Tailwater | 13.39 | 83.61 |  |

FlexTable: Catchment Table

| Outflow Element | Area (User Defined) (ft ${ }^{2}$ ) | Runoff Coefficient (Rational) | $\underset{\left(\mathrm{ft}^{2}\right)}{\text { Catchment CA }}$ | Time of Concentration (min) | $\begin{aligned} & \hline \text { Flow (Total Out) } \\ & \text { (cfs) } \end{aligned}$ | Notes |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| MH-4 | 40,184.0 | 0.853 | 34,256.9 | 5.000 | 6.02 |  |
| MH-15 | 13,864.0 | 0.755 | 10,467.3 | 5.000 | 1.84 |  |
| MH-8 | 19,736.0 | 0.950 | 18,749.2 | 5.000 | 3.30 |  |
| MH-16 | 14,484.0 | 0.950 | 13,759.8 | 5.000 | 2.42 |  |
| MH-5 | 9,915.0 | 0.950 | 9,419.3 | 5.000 | 1.66 |  |
| CB-1A | 30,155.0 | 0.690 | 20,807.0 | 10.000 | 2.94 |  |
| CB-1B | 24,345.0 | 0.755 | 18,380.5 | 10.000 | 2.59 |  |
| CB-3B | 1,625.0 | 0.950 | 1,543.8 | 5.000 | 0.27 |  |
| CB-3A | 24,730.0 | 0.625 | 15,456.3 | 10.000 | 2.18 |  |
| CB-4B | 25,830.0 | 0.950 | 24,538.5 | 5.000 | 4.32 |  |
| CB-4A | 56,382.0 | 0.495 | 27,909.1 | 10.000 | 3.94 |  |
| CB-6A | 24,082.0 | 0.528 | 12,703.3 | 10.000 | 1.79 |  |
| CB-6B | 9,250.0 | 0.950 | 8,787.5 | 5.000 | 1.55 |  |
| CB-7B | 2,890.0 | 0.950 | 2,745.5 | 5.000 | 0.48 |  |
| CB-7C | 14,156.0 | 0.950 | 13,448.2 | 5.000 | 2.37 |  |
| CB-7A | 8,922.0 | 0.495 | 4,416.4 | 5.000 | 0.78 |  |
| CB-7D | 18,957.0 | 0.885 | 16,776.9 | 10.000 | 2.37 |  |
| CB-11A | 16,603.0 | 0.885 | 14,693.7 | 5.000 | 2.58 |  |
| CB-12B | 33,672.0 | 0.820 | 27,611.0 | 10.000 | 3.90 |  |
| CB-12A | 87,263.0 | 0.690 | 60,211.5 | 10.000 | 8.50 |  |
| CB-15A | 117,987.0 | 0.788 | 92,914.8 | 10.000 | 13.12 |  |
| CB-15B | 21,212.0 | 0.820 | 17,393.8 | 5.000 | 3.06 |  |
| CB-16B | 22,834.0 | 0.625 | 14,271.3 | 5.000 | 2.51 |  |
| CB-16A | 10,418.0 | 0.755 | 7,865.6 | 5.000 | 1.38 |  |
| CB-9A | 41,038.0 | 0.495 | 20,313.8 | 10.000 | 2.87 |  |
| CB-9B | 75,763.0 | 0.788 | 59,663.4 | 10.000 | 8.42 |  |
| CB-15C | 19,035.0 | 0.820 | 15,608.7 | 5.000 | 2.75 |  |

APPENDIX - F



FlexTable: Conduit Table

| Label | Start Node | Stop Node | Elevation Ground (Start) (ft) | Hydraulic Grade Line (In) (ft) | Invert (Start) <br> (ft) | $\begin{array}{\|l\|} \hline \text { Elevation Ground } \\ \text { (Stop) } \\ \text { (ft) } \\ \hline \end{array}$ | Hydraulic Grade Line (Out) <br> (ft) | Invert (Stop) <br> (ft) | Length (User Defined) (ft) | $\begin{gathered} \text { Slope } \\ \left(\begin{array}{c} \text { Calculated) } \\ (\mathrm{ft} / \mathrm{ft}) \end{array}\right. \end{gathered}$ | Diameter <br> (in) | Material | $\begin{aligned} & \text { Flow } \\ & \text { (cfs) } \end{aligned}$ | Capacity (Full Flow) (cfs) | Velocity ( $\mathrm{ft} / \mathrm{s}$ ) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P-1A | CB-1A | OUT-1 | 17.18 | 14.03 | 13.30 | 17.48 | 13.44 | 12.88 | 19.0 | 0.022 | 12.0 | Concrete | 2.94 | 5.30 | 6.92 |
| P-1B | CB-1B | OUT-1 | 17.30 | 13.89 | 13.20 | 17.48 | 13.39 | 12.90 | 6.5 | 0.046 | 12.0 | Concrete | 2.59 | 7.65 | 8.80 |
| P-2 | MH-2 | OUT-1 | 16.64 | 23.38 | 6.84 | 17.48 | 7.40 | 5.40 | 134.0 | 0.011 | 24.0 | Concrete | 78.12 | 23.45 | 24.87 |
| P-3 | MH-3 | MH-2 | 15.50 | 21.29 | 7.50 | 16.64 | 16.64 | 6.90 | 164.0 | 0.004 | 18.0 | Concrete | 17.68 | 6.35 | 10.01 |
| P-3A | CB-3A | MH-3 | 15.38 | 15.52 | 11.60 | 15.50 | 15.50 | 11.40 | 5.0 | 0.040 | 12.0 | Concrete | 2.18 | 7.13 | 2.78 |
| P-3B | СB-3B | MH-3 | 14.90 | 15.50 | 11.80 | 15.50 | 15.50 | 11.40 | 20.0 | 0.020 | 12.0 | Concrete | 0.27 | 5.04 | 0.35 |
| P-4 | MH-4 | MH-3 | 13.70 | 23.54 | 8.90 | 15.50 | 15.50 | 7.50 | 355.0 | 0.004 | 18.0 | Concrete | 15.81 | 6.60 | 8.94 |
| P-4A | CB-4A | MH-4 | 13.18 | 13.73 | 9.10 | 13.70 | 13.70 | 8.90 | 8.0 | 0.025 | 15.0 | Concrete | 3.94 | 10.21 | 3.21 |
| P-4B | СB-4B | MH-4 | 13.25 | 13.77 | 9.20 | 13.70 | 13.70 | 8.90 | 15.0 | 0.020 | 15.0 | Concrete | 4.32 | 9.14 | 3.52 |
| P-5 | MH-5 | MH-4 | 14.41 | 14.12 | 9.20 | 13.70 | 13.70 | 8.95 | 98.0 | 0.003 | 15.0 | Concrete | 4.23 | 3.26 | 3.44 |
| P-6 | MH-6 | MH-5 | 15.30 | 14.41 | 10.00 | 14.41 | 14.12 | 9.30 | 132.0 | 0.005 | 15.0 | Concrete | 3.03 | 4.70 | 2.47 |
| P-6A | CB-6A | MH-6 | 14.78 | 14.43 | 10.30 | 15.30 | 14.41 | 10.10 | 10.0 | 0.020 | 12.0 | Concrete | 1.79 | 5.04 | 2.28 |
| P-6B | CB-6B | MH-6 | 14.83 | 14.44 | 10.40 | 15.30 | 14.41 | 10.10 | 15.0 | 0.020 | 12.0 | Concrete | 1.55 | 5.04 | 1.97 |
| P-7 | MH-7 | MH-2 | 11.75 | 30.38 | 7.65 | 16.64 | 16.64 | 7.05 | 172.0 | 0.003 | 24.0 | Concrete | 63.93 | 13.36 | 20.35 |
| P-7A | CB-7A | MH-7 | 11.78 | 11.76 | 8.60 | 11.75 | 11.75 | 8.25 | 39.0 | 0.009 | 15.0 | Concrete | 0.78 | 6.12 | 0.63 |
| P-7B | СВ-7B | MH-7 | 11.60 | 11.79 | 7.90 | 11.75 | 11.75 | 7.75 | 20.0 | 0.008 | 15.0 | Concrete | 2.84 | 5.59 | 2.31 |
| P-7C | СВ-7C | CB-7B | 11.45 | 11.61 | 8.15 | 11.60 | 11.60 | 7.90 | 8.0 | 0.031 | 15.0 | Concrete | 2.37 | 11.42 | 1.93 |
| P-7D | CB-7D | MH-7 | 11.50 | 11.76 | 8.50 | 11.75 | 11.75 | 8.05 | 5.0 | 0.090 | 15.0 | Concrete | 2.37 | 19.38 | 1.93 |
| P-8 | MH-8 | MH-7 | 12.39 | 11.95 | 8.50 | 11.75 | 11.75 | 8.10 | 39.0 | 0.010 | 12.0 | PVC | 3.30 | 4.69 | 4.20 |
| P-9 | MH-9 | MH-7 | 11.35 | 24.43 | 7.95 | 11.75 | 11.75 | 7.70 | 58.0 | 0.004 | 18.0 | Concrete | 49.11 | 6.90 | 27.79 |
| P-9A | CB-9A | MH-9 | 10.80 | 11.38 | 8.20 | 11.35 | 11.35 | 8.10 | 4.0 | 0.025 | 12.0 | Concrete | 2.87 | 5.63 | 3.65 |
| P-98 | Св-9B | MH-9 | 10.84 | 11.88 | 8.35 | 11.35 | 11.35 | 8.05 | 16.0 | 0.019 | 12.0 | PVC | 8.42 | 6.34 | 10.72 |
| P-10 | MH-10 | MH-9 | 11.46 | 24.00 | 8.30 | 11.35 | 11.35 | 8.00 | 96.0 | 0.003 | 18.0 | Concrete | 38.13 | 5.87 | 21.58 |
| P-11 | MH-11 | MH-10 | 14.20 | 11.69 | 9.00 | 11.46 | 11.46 | 8.60 | 43.0 | 0.009 | 12.0 | Concrete | 2.58 | 3.44 | 3.29 |
| P-11A | CB-11A | MH-11 | 14.66 | 11.72 | 10.70 | 14.20 | 11.69 | 10.50 | 6.0 | 0.033 | 12.0 | Concrete | 2.58 | 6.50 | 3.29 |
| P-12 | MH-12 | MH-10 | 13.43 | 31.72 | 9.10 | 11.46 | 11.46 | 8.40 | 170.0 | 0.004 | 18.0 | Concrete | 36.26 | 6.74 | 20.52 |
| P-12A | CB-12A | MH-12 | 13.32 | 13.97 | 9.70 | 13.43 | 13.43 | 9.30 | 31.0 | 0.013 | 15.0 | Concrete | 8.50 | 7.34 | 6.93 |
| P-12B | CB-12B | MH-12 | 15.00 | 13.64 | 10.90 | 13.43 | 13.43 | 9.50 | 59.0 | 0.024 | 15.0 | Concrete | 3.90 | 9.95 | 3.18 |
| P-13 | MH-13 | MH-12 | 14.80 | 19.44 | 10.80 | 13.43 | 13.43 | 9.20 | 114.0 | 0.014 | 18.0 | Concrete | 24.13 | 12.44 | 13.65 |
| P-14 | MH-14 | MH-13 | 17.83 | 20.98 | 13.50 | 14.80 | 14.80 | 10.90 | 116.0 | 0.022 | 18.0 | Concrete | 24.24 | 15.73 | 13.72 |
| P-15 | MH-15 | MH-14 | 19.23 | 20.18 | 14.10 | 17.83 | 17.83 | 13.60 | 44.0 | 0.011 | 18.0 | Concrete | 24.28 | 11.20 | 13.74 |
| P-15A | CB-15A | MH-15 | 19.80 | 20.47 | 16.00 | 19.23 | 19.23 | 15.10 | 30.0 | 0.030 | 15.0 | Concrete | 13.12 | 11.19 | 10.69 |
| P-15B | CB-15B | MH-15 | 20.43 | 19.34 | 17.20 | 19.23 | 19.23 | 14.80 | 51.0 | 0.047 | 15.0 | Concrete | 3.06 | 14.01 | 2.49 |
| P-15C | CB-15C | MH-15 | 41.10 | 20.67 | 20.00 | 19.23 | 19.23 | 14.30 | 53.0 | 0.108 | 15.0 | Concrete | 2.75 | 21.18 | 11.90 |
| P-16 | MH-16 | MH-15 | 20.20 | 19.96 | 15.00 | 19.23 | 19.23 | 14.20 | 79.0 | 0.010 | 15.0 | Concrete | 6.21 | 6.50 | 5.06 |
| P-16A | CB-16A | MH-16 | 19.00 | 20.88 | 15.90 | 20.20 | 19.96 | 15.10 | 78.0 | 0.010 | 12.0 | Concrete | 3.87 | 3.61 | 4.93 |
| P-16B | CB-16B | CB-16A | 19.20 | 19.13 | 16.40 | 19.00 | 19.00 | 16.00 | 27.0 | 0.015 | 12.0 | Concrete | 2.51 | 4.34 | 3.20 |

FlexTable: Catch Basin Table

| Label | Elevation (Ground) (ft) | Hydraulic Grade Line (ft) | Is Overflowing? | $\begin{gathered} \text { Flow } \\ \text { (Captured) } \\ \text { (cfs) } \end{gathered}$ | $\begin{aligned} & \text { Flow (Additional } \\ & \text { Carryover) } \\ & \text { (ffs) } \end{aligned}$ | $\begin{aligned} & \text { Flow (Total } \\ & \text { Out) } \\ & \text { (ffs) } \end{aligned}$ | Elevation (Invert) (ft) | Is Surcharged? | Inlet C | $\begin{gathered} \hline \text { Local CA } \\ \left(\mathrm{ft}^{2}\right) \end{gathered}$ | Structure Type | Length (ft) | Width (ft) | Inlet Location | Longitudinal Slope (Inlet) (ft/ft) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| CB-1A | 17.18 | 14.03 | False | 2.94 | 0.00 | 2.94 | 11.30 | False | 0.690 | 20,807.0 | Box Structure | 5.40 | 4.40 | On Grade | 0.011 |
| CB-1B | 17.30 | 13.89 | False | 2.59 | 0.00 | 2.59 | 11.20 | False | 0.755 | 18,380.5 | Box Structure | 5.40 | 4.40 | On Grade | 0.011 |
| CB-3A | 15.38 | 15.38 | True | 2.18 | 0.00 | 2.18 | 9.40 | True | 0.625 | 15,456.3 | Box Structure | 5.40 | 4.40 | On Grade | 0.004 |
| CB-3B | 14.90 | 14.90 | True | 0.27 | 0.00 | 0.27 | 9.80 | True | 0.950 | 1,543.7 | Box Structure | 5.40 | 4.40 | On Grade | 0.004 |
| CB-4A | 13.18 | 13.18 | True | 3.94 | 0.00 | 3.94 | 7.10 | True | 0.495 | 27,909.1 | Box Structure | 5.40 | 4.40 | On Grade | 0.004 |
| CB-4B | 13.25 | 13.25 | True | 4.32 | 0.00 | 4.32 | 7.20 | True | 0.950 | 24,538.5 | Box Structure | 5.40 | 4.40 | On Grade | 0.004 |
| CB-6A | 14.78 | 14.43 | False | 1.79 | 0.00 | 1.79 | 8.30 | True | 0.528 | 12,703.3 | Box Structure | 5.40 | 4.40 | On Grade | 0.005 |
| CB-6B | 14.83 | 14.44 | False | 1.55 | 0.00 | 1.55 | 8.40 | True | 0.950 | 8,787.5 | Box Structure | 5.40 | 4.40 | On Grade | 0.005 |
| CB-7A | 11.78 | 11.76 | False | 0.78 | 0.00 | 0.78 | 6.65 | True | 0.495 | 4,416.4 | Box Structure | 5.40 | 4.40 | On Grade | 0.009 |
| СВ-7B | 11.60 | 11.60 | True | 0.48 | 0.00 | 2.84 | 5.90 | True | 0.950 | 2,745.5 | Box Structure | 5.40 | 4.40 | In Sag |  |
| CB-7C | 11.45 | 11.45 | True | 2.37 | 0.00 | 2.37 | 6.15 | True | 0.950 | 13,448.2 | Box Structure | 5.40 | 4.40 | In Sag |  |
| CB-7D | 11.50 | 11.50 | True | 2.37 | 0.00 | 2.37 | 6.50 | True | 0.885 | 16,776.9 | Box Structure | 5.40 | 4.40 | In Sag |  |
| CB-9A | 10.80 | 10.80 | True | 2.87 | 0.00 | 2.87 | 6.20 | True | 0.495 | 20,313.8 | Box Structure | 5.40 | 4.40 | In Sag |  |
| CB-9B | 10.84 | 10.84 | True | 8.42 | 0.00 | 8.42 | 6.35 | True | 0.788 | 59,663.4 | Box Structure | 5.40 | 4.40 | In Sag |  |
| CB-11A | 14.66 | 11.72 | False | 2.58 | 0.00 | 2.58 | 8.70 | True | 0.885 | 14,693.7 | Box Structure | 5.40 | 4.40 | On Grade | 0.010 |
| CB-12A | 13.32 | 13.32 | True | 8.50 | 0.00 | 8.50 | 7.70 | True | 0.690 | 60,211.5 | Box Structure | 5.40 | 4.40 | On Grade | 0.010 |
| CB-12B | 15.00 | 13.64 | False | 3.90 | 0.00 | 3.90 | 8.90 | True | 0.820 | 27,611.0 | Box Structure | 5.40 | 4.40 | On Grade | 0.010 |
| CB-15A | 19.80 | 19.80 | True | 13.12 | 0.00 | 13.12 | 14.00 | True | 0.788 | 92,914.8 | Box Structure | 10.80 | 4.40 | On Grade | 0.012 |
| CB-15B | 20.43 | 19.34 | False | 3.06 | 0.00 | 3.06 | 15.20 | True | 0.820 | 17,393.8 | Box Structure | 5.40 | 4.40 | On Grade | 0.012 |
| CB-15C | 41.10 | 20.67 | False | 2.75 | 0.00 | 2.75 | 20.00 | False | 0.820 | 15,608.7 | Box Structure | 10.80 | 4.40 | On Grade | 0.060 |
| CB-16A | 19.00 | 19.00 | True | 1.38 | 0.00 | 3.87 | 13.90 | True | 0.755 | 7,865.6 | Box Structure | 5.40 | 4.40 | In Sag |  |
| CB-16B | 19.20 | 19.13 | False | 2.51 | 0.00 | 2.51 | 14.40 | True | 0.625 | 14,271.3 | Box Structure | 5.40 | 4.40 | In Sag |  |

FlexTable: Manhole Table

| Label | Elevation (Ground) (ft) | Hydraulic Grade Line (Out) <br> (ft) | Is Overflowing? | $\begin{aligned} & \hline \text { Flow (Known) } \\ & \text { (cfs) } \end{aligned}$ | $\begin{aligned} & \hline \text { Flow (Total Out) } \\ & \text { (cfs) } \end{aligned}$ | Elevation (Invert) (ft) | Is Surcharged? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| MH-2 | 16.64 | 16.64 | True | 0.00 | 78.12 | 6.84 | True |
| MH-3 | 15.50 | 15.50 | True | 0.00 | 17.68 | 6.90 | True |
| MH-4 | 13.70 | 13.70 | True | 0.00 | 15.81 | 8.90 | True |
| MH-5 | 14.41 | 14.12 | False | 0.00 | 4.23 | 9.20 | True |
| MH-6 | 15.30 | 14.41 | False | 0.00 | 3.03 | 10.00 | True |
| MH-7 | 11.75 | 11.75 | True | 7.12 | 63.93 | 7.65 | True |
| MH-8 | 12.39 | 11.95 | False | 0.00 | 3.30 | 8.50 | True |
| MH-9 | 11.35 | 11.35 | True | 0.00 | 49.11 | 7.95 | True |
| MH-10 | 11.46 | 11.46 | True | 0.00 | 38.13 | 8.30 | True |
| MH-11 | 14.20 | 11.69 | False | 0.00 | 2.58 | 9.00 | True |
| MH-12 | 13.43 | 13.43 | True | 0.00 | 36.26 | 9.10 | True |
| MH-13 | 14.80 | 14.80 | True | 0.00 | 24.13 | 10.80 | True |
| MH-14 | 17.83 | 17.83 | True | 0.00 | 24.24 | 13.50 | True |
| MH-15 | 19.23 | 19.23 | True | 0.00 | 24.28 | 14.10 | True |
| MH-16 | 20.20 | 19.96 | False | 0.00 | 6.21 | 15.00 | True |

FlexTable: Outfall Table

| Label | Elevation <br> (Ground) <br> (ft) | Elevation <br> (Invert) <br> (ft) | Elevation (User <br> Defined <br> Tailwater) <br> (ft) | Boundary <br> Condition Type | Hydraulic Grade <br> (ft) | Flow (Total Out) <br> (cfs) | Notes |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| OUT-1 | 17.48 | 5.40 | 6.40 | User Defined <br> Tailwater | 13.39 | 83.42 |  |

FlexTable: Catchment Table

| Outflow Element | Area (User Defined) ( $\mathrm{ft}^{2}$ ) | Runoff Coefficient (Rational) | $\underset{\left(\mathrm{ft}^{2}\right)}{\text { Catcht CA }}$ | Time of Concentration $(\mathrm{min})$ | $\begin{aligned} & \text { Flow (Total Out) } \\ & \text { (cfs) } \end{aligned}$ | Notes |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| MH-4 | 40,081.0 | 0.853 | 34,169.1 | 5.000 | 6.01 |  |
| MH-15 | 13,864.0 | 0.755 | 10,467.3 | 5.000 | 1.84 |  |
| MH-8 | 19,736.0 | 0.950 | 18,749.2 | 5.000 | 3.30 |  |
| MH-16 | 14,484.0 | 0.950 | 13,759.8 | 5.000 | 2.42 |  |
| MH-5 | 9,915.0 | 0.950 | 9,419.3 | 5.000 | 1.66 |  |
| CB-1A | 30,155.0 | 0.690 | 20,807.0 | 10.000 | 2.94 |  |
| CB-1B | 24,345.0 | 0.755 | 18,380.5 | 10.000 | 2.59 |  |
| CB-3B | 1,625.0 | 0.950 | 1,543.8 | 5.000 | 0.27 |  |
| CB-3A | 24,730.0 | 0.625 | 15,456.3 | 10.000 | 2.18 |  |
| CB-4B | 25,830.0 | 0.950 | 24,538.5 | 5.000 | 4.32 |  |
| CB-4A | 56,382.0 | 0.495 | 27,909.1 | 10.000 | 3.94 |  |
| CB-6A | 24,082.0 | 0.528 | 12,703.3 | 10.000 | 1.79 |  |
| CB-6B | 9,250.0 | 0.950 | 8,787.5 | 5.000 | 1.55 |  |
| CB-7B | 2,890.0 | 0.950 | 2,745.5 | 5.000 | 0.48 |  |
| CB-7C | 14,156.0 | 0.950 | 13,448.2 | 5.000 | 2.37 |  |
| CB-7A | 8,922.0 | 0.495 | 4,416.4 | 5.000 | 0.78 |  |
| CB-7D | 18,957.0 | 0.885 | 16,776.9 | 10.000 | 2.37 |  |
| CB-11A | 16,603.0 | 0.885 | 14,693.7 | 5.000 | 2.58 |  |
| CB-12B | 33,672.0 | 0.820 | 27,611.0 | 10.000 | 3.90 |  |
| CB-12A | 87,263.0 | 0.690 | 60,211.5 | 10.000 | 8.50 |  |
| CB-15A | 117,987.0 | 0.788 | 92,914.8 | 10.000 | 13.12 |  |
| CB-15B | 21,212.0 | 0.820 | 17,393.8 | 5.000 | 3.06 |  |
| CB-16B | 22,834.0 | 0.625 | 14,271.3 | 5.000 | 2.51 |  |
| CB-16A | 10,418.0 | 0.755 | 7,865.6 | 5.000 | 1.38 |  |
| CB-9A | 41,038.0 | 0.495 | 20,313.8 | 10.000 | 2.87 |  |
| CB-9B | 75,763.0 | 0.788 | 59,663.4 | 10.000 | 8.42 |  |
| CB-15C | 19,035.0 | 0.820 | 15,608.7 | 5.000 | 2.75 |  |

APPENDIX - G

Directly Connected impervious Area Tracking Worksheet City of Stamford Drainage Manual

## Note to user: complete all cells of this color only as indicated by section headings

| Project Name <br> Project Address <br> Project Applicant <br> Title of Plan <br> Revision Date of Plan <br> Tax Account Number | Part 1: General Information (All Projects) |
| :---: | :---: |
|  | 819 EAST MAIN STREET |
|  | 821, 825,827 \& 831 EAST MAIN STREET + 15, 27 \& 29 LAFAYETTE STREET |
|  | 819 EAST MAIN STREET, LLC |
|  | Site Plan depicting 821, 825, 827 \& 831 East Main St, 15, 27, \& 29 Lafayette St, Stamford, CT' |
|  | 2/03/2022 |
|  | 001-7666, 002-5499, 000-4640, 000-4639,001-1420, 001-7662, 001-7663 |

## Part 2: Project Details (All Projects)

1. What type of development is this? (choose from dropdown)
2. What is the total area of the project site?
3. What is the total area of land disturbance for this project?
4. Does project site drain to High Quality Waters, a Direct Waterfront, or within 500 ft . of Tidal Wetlands? (Yes/No)

Does Standard 1 apply based on information above?

| Development |
| :---: |
| 50,237 |
| 50,237 |
| NO |
| YES |
| $\mathrm{ft}^{2}$ |

## Part 3: Water Quality Target Total (Only for Standard 1 Projects)

| 5. What is the current (pre-development) DCIA for the site? <br> 6. Will the proposed development increase DCIA (without consideration of <br> proposed stormwater management)? (Yes/No) | 39,380 |
| :--- | ---: |
| 7. What is the proposed-development total impervious area for the site? | YES |
| Water Quality Volume (WQV) 45,473 <br> Standard 1 requirement  <br> Required treatment/retention volume <br> Provided treatment/retention volume for proposed development 3,620 | RETAIN FULL W.Q.V. |


| Part 4: Proposed DCIA Tracking (Only for Standard 1 Projects) |  |
| :--- | :---: |
| Pre-development total impervious area | 39,380 |
| Current DCIA | 39,380 |
| Proposed-development total impervious area | 45,473 |
| Proposed-development DCIA (after stormwater management) | 31,672 |
| Net change in DCIA from current to proposed-development | $-7,708$ |

Part 5: Post-Development (As-Built Certified) DCIA Tracking (Only for Standard 1 Projects) Post-development (per as-built) total impervious area Post-development (per as-built) DCIA (after stormwater management) Net change in DCIA from current to post-development

## Certification Statement

I hereby certify that the information contained in this worksheet is true and correct.

Engineer's Signature Date 2/03/2022

APPENDIX - H

## CHECKLISTS

Project Name:
819 EAST MAIN STREET

Project Address
821, 825, 827 \& 831 EAST MAIN STREET + 15, 27 \& 29 LAFAYETTE STREET
819 EAST MAIN STREET, LLC
Property Owner(s) $\qquad$
Tax Account Number(s)
(s) 001-7666, 002-5499, 000-4640,000-4639, 001-1420,001-7662, 001-7663

All checklists must be completed and submitted. Provide a brief explanation for any items not provided. Check boxes as completed or N/A as not applicable.

| $\boldsymbol{J}$ | Existing Conditions Plan |
| :--- | :--- |
| $\boldsymbol{J}$ | Stormwater Management Report |
| $\boldsymbol{J}$ | Stormwater Management Plan / Construction Plan |
|  | Certificate of Occupancy |

## Checklist for Existing Conditions Plan

## I. General Information

| $\checkmark$ | Site address |
| :---: | :---: |
| $\checkmark$ | Orientation, block, zone, City, street name |
| $\checkmark$ | Applicant name and legal address |
| $\checkmark$ | Surveyor name, address, contact information |
| $\checkmark$ | North arrow, bar scale, horizontal and vertical datum |
| $\checkmark$ | $24^{\prime \prime} \times 36$ " sheet size unless otherwise approved |
| $\sqrt{ }$ | Existing conditions survey shall be prepared in accordance with the Minimum Standards for Surveys and Maps in the State of Connecticut. The class of survey shall be A-2 and T-2 and shall be represented as such on the map. The base map shall be sealed and signed by a Professional Land Surveyor licensed in the State of Connecticut. |
| $\checkmark$ | Drawing scale shall be set at $1^{\prime \prime}=20^{\prime}$ or $1^{\prime \prime}=40^{\prime}$ when possible |

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## II. Existing Conditions Plan Elements

| $\checkmark$ | Show and label all property boundaries with linear bearing / distances and curve information |
| :---: | :---: |
| $\checkmark$ | Required zoning setbacks |
| $\checkmark$ | Show and label monument information |
| $\checkmark$ | Show and label at least one permanent benchmark on the parcel with northing, easting and elevation |
| $\checkmark$ | Label adjacent property ownership information |
| $\checkmark$ | Existing contours based on NAVD 88 (no exceptions) at 2 foot contour interval or 1 foot contour interval when slope is flatter than 2 percent at a minimum of 20 ft . beyond the property boundaries of the subject parcel |
| $\checkmark$ | Show spot elevations at low points, high points, and where topography is flatter than 2 percent |
| $\checkmark$ | All buildings and structures (label current use and finished floor elevations) |
| $\checkmark$ | All pavement, parking, driveways, property access points |
| $\checkmark$ | All roadways, streets, and rights-of-way. Label streets as public or private with street name |
| $\checkmark$ | All patios, decks, walkways, sidewalks, curb ramps (both adjacent to and opposite and existing roadways or intersections) |
| $\checkmark$ | Show and label (size, material, inverts) all existing utilities (overhead and underground) within the right-of-way and the project site (label ownership) including but not limited to water, gas and electrical services, wells, storm sewers, sanitary sewers and subsurface sewerage disposal systems. |
| $\checkmark$ | Show and label existing conveyance systems (swales, ditches, storm drains) including dimensions, elevations, sizes, slopes, and direction of flow |
| $\checkmark$ | Show and label boundaries of all easements, both public and private, with type, owner, and width |
| $\checkmark$ | Show and label all other existing features and improvements (e.g. light poles, mature trees of $8^{\prime \prime}$ (dbh) diameter or greater, vegetation, walls with top and bottom elevations, fences, pavement markings) |

## III. Resource Areas

| N/A |  | Show and label limits of inland wetlands, tidal wetlands and any associated setbacks. |
| :---: | :---: | :---: |
|  | $\checkmark$ | Show and label existing natural site features including tree canopy, outcroppings, permanent and intermittent watercourses, waterbodies, streams |
| N/A |  | Show and label limits of floodplain and floodway along with FIRM references (Community Number, Panel, Suffix, and Date) including any effective Letters of Map Revision/Amendment, zone designation and elevation. |
| N/A |  | Show and label any Conservation Easement Areas |
| N/A |  | Show and label Connecticut Coastal Jurisdiction Line (CJL) |
| N/A |  | Show and label existing steep slopes (25\% and greater) |

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## Checklist for Stormwater Management Report

## I. Project Report

A. Applicant / Site Information

| $\boldsymbol{\checkmark}$ | Applicant name, legal address, contact information (email \& phone) |
| :--- | :--- |
| $\boldsymbol{\nearrow}$ | Engineers name, legal address, contact information (email \& phone) |
| $\boldsymbol{\checkmark}$ | Site address and legal description |
| $\boldsymbol{\checkmark}$ | Current / proposed zoning and land use |
| $\boldsymbol{V}$ | Site vicinity map $\left(8.5^{\prime \prime} \times 11^{\prime \prime}\right)$ |

B. Project Description and Purpose
$\checkmark$
Project description including proposed project elements and anticipated construction schedule
C. Existing Conditions Description

| $\boldsymbol{V}$ | Site area, ground cover, vegetation, features (roads, buildings, utilities, etc.) |
| :--- | :--- | :--- |
| $\boldsymbol{V}$ | Site topography, slopes, drainage patterns, conveyances systems (swales, storm drains, etc.), stormwater <br> discharge locations |
| $\boldsymbol{\checkmark}$ | Receiving waterbody information including stormwater impairments and TMDL information (See the most recent <br> State of Connecticut Integrated Water Quality Report) |
| $\boldsymbol{V}$ | Site soils information including soil types, hydrologic soil group, bedrock / outcroppings, groundwater elevation, <br> significant geologic features |
| $\boldsymbol{V}$ | Provide NRCS Soils Mapping |
| N/A | Resource protection areas (wetlands, streams, lakes, etc.), buffers, floodplains, floodways |

D. Summary of Applicable General Design Criteria

| $\boldsymbol{V}$ | Methodology, design storm frequency |  |
| :--- | :--- | :--- |
| $\boldsymbol{V}$ | Hydrologic design criteria |  |
| $\boldsymbol{V}$ | Hydraulic design criteria |  |
| N/A | Flood hazard areas |  |
|  |  |  |


|  | $\frac{\text { Applying under "Lite" Stormwater Management: Skip to Section I }}{(R e f e r ~ t o ~ F l o w ~ C h a r t ~ o n ~ p a g e ~ v i i ~ o f ~ t h e ~ C i t y ~ o f ~ S t a m f o r d ~ S t o r m w a t e r ~ D r a i n a g e ~ M a n u a l) ~}$ |
| :--- | :--- |
| $\boldsymbol{V}$ | Area of disturbance, receiving waterbody classification (High Quality, Tidal Wetlands, Direct Waterfront) |
| $\boldsymbol{V}$ | Project type (development, redevelopment, linear development) |
| $\boldsymbol{V}$ | Pollutant reduction standard per flowchart Section 2.4 |

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## F. Summary of LID Site Constraints

N.A.

|  | Description of sensitive areas for protection |
| :--- | :--- |
|  | Mature tree inventory, which shall include 8-inch (dbh) diameter trees or greater |
|  | Steep slopes |
|  | Ledge and bedrock depth |
|  | Seasonal high groundwater elevation |
|  | Pollutant hotspots |
| $\boldsymbol{V}$ | Summary of infiltration rates |

G. Summary of Proposed Stormwater Treatment Practices

| $\checkmark$ | Proposed LID controls (i.e. minimize impervious, minimize DCIA, minimize disturbance, increase time of <br> concentrations, other LID controls and strategies) |
| :--- | :--- |
| $\checkmark$ | Location, size, types |
| $\checkmark$ | Design criteria and references |
| $\checkmark$ | Stormwater treatment practice, drainage area characteristics / details |

H. Summary of Compliance with Standards 1

| $\checkmark$ | Required pollutant reduction criteria |
| :--- | :--- |
| $\checkmark$ | Provided pollutant reduction (WQV) by stormwater treatment practice |
| $\checkmark$ | Summary of compliance with Standard 1 |

I. Summary of Compliance with Standards 2, 3, and 4

| $\checkmark$ | Description of proposed stormwater management system |
| :---: | :--- |
| $\checkmark$ | Pre-development site hydrology with delineation of each watershed area and sub-basin |
| $\checkmark$ | Post-development site hydrology with delineation of each watershed area and sub-basin |
| $\checkmark$ | Comparison table of pre- and post-development hydrology, peak flow, volume, and percent difference |
| $\checkmark$ | Summary table of watershed areas and sub-basin areas, time of concentration and runoff coefficients |
|  | Summary table demonstrating the 2-year, 24-hour post development peak flow rate is less than or equal to the <br> lowest of either: <br> -The pre-development 1-year, 24-hour storm peak flow rate <br> -50 percent of the pre-development 2-year, 24-hour storm peak flow rate |
| $\checkmark$ | Conveyance protection, emergency outlet sizing |
| $\checkmark$ | Hydraulic grade line summary and tail water elevation used in analysis |
| $\checkmark$ | Construction erosion and sediment control description, Standard 3 |
| $\checkmark$ | Operation and Maintenance, maintenance tasks and schedule on construction plans per Standard 4 |

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## J. Summary of Compliance with Applicable Drainage Facility Design Requirements



Description of applicable design requirements and compliance
Description of proposed drainage facilities and compliance
K. Stormwater Management Report

| $\boldsymbol{V}$ | Signed and stamped by professional engineer licensed in the State of Connecticut |
| :--- | :--- |
| $\boldsymbol{\checkmark}$ | Drainage impact statement in accordance with Standard 5B. |

II. Supporting Calculations (as appendix to Project Report)

## Applying under "Lite" Stormwater Management: Skip to Section N

L. Water Quality Volume / Water Quality Flow Calculations
$\sqrt{ }$ Calculations demonstrating the total Water Quality Volume generated by the post-development site and the $\checkmark$ required retention/treatment volume per Standard 1 in cubic feet.
$\sqrt{\boldsymbol{l}}$ Calculations demonstrating the total Water Quality Volume retained/treated by each stormwater treatment practice and the total Water Quality Volume generated by the post-development contributing drainage area to each stormwater treatment practice
M. Stormwater Treatment Practice Sizing Calculations

Calculations demonstrating how each stormwater treatment practice has been designed and sized in accordance with the Structural Stormwater BMP Design references in Appendix B. Calculations will vary by stormwater treatment practice, but a minimum, applicants shall provide calculations in accordance with design criteria from the Connecticut Stormwater Quality Manual.

| N.A. | N. Hydrologic and Hydraulic Design Calculations |  |
| :---: | :---: | :---: |
|  |  | Stream channel protection, Standard 2A |
|  | $\checkmark$ | Conveyance protection, Standard 2B |
|  | $\checkmark$ | Peak flow control (1-year, 2-year, 5-year, 10-year, 25-year, and 50-year storms), Standard 2C |
| N.A. <br> N.A. |  | Inlet analysis |
|  |  | Gutter flow (Site by site basis as requested by Engineering Bureau) |
|  | $\checkmark$ | Storm sewers and culverts (velocities, capacity, hydraulics) |
|  | $\checkmark$ | Hydraulic grade line required when pipe is flowing at full capacity <br> - Provide existing and proposed summary table <br> - Provide existing and proposed mapping, label structures |
|  | $\checkmark$ | Detention facilities (outlet structure, stage/storage, freeboard) |
|  | $\checkmark$ | Emergency outlet sizing, safely pass the 100 year storm, Standard 2D |
|  | $\checkmark$ | Outlet protection calculations, based on conveyance protection (i.e. riprap, energy dissipater) |

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O. Hydrologic and Hydraulic Model, Existing and Proposed

| $\boldsymbol{\checkmark}$ | Drainage routing diagram |
| :--- | :--- |
| $\boldsymbol{\checkmark}$ | Summary |
| $\boldsymbol{\checkmark}$ | Storage pond input |

P. Downstream analysis (Site by site basis as required by the Engineering Bureau)

| $\checkmark$ | Downstream analysis, Standard 2E |
| :--- | :--- |

III. Supporting Mapping (as appendix to Project Report)
N.A.
Q. Pre-Development Drainage Basin Area Mapping

| $\checkmark$ | $11^{\prime \prime} \times 17^{\prime \prime}$ or $8.5^{\prime \prime} \times 11^{\prime \prime}$ sheet size |  |
| :--- | :--- | :--- |
| $\checkmark$ | N.A. | Topography, drainage patterns, drainage area boundaries and sub basins, flow paths, times of concentration |
| $\checkmark$ | $\checkmark$ | Locations of existing stormwater discharges |
|  |  | Perennial and intermittent streams, wetlands, and floodplain / floodways |
| $\checkmark$ | NRCS soil types, locations, boring locations, infiltration testing locations |  |
| $\checkmark$ | Vegetation and groundcover |  |
| $\checkmark$ | Existing roads, buildings, driveways, parking areas, walks, patios, pools and other impervious surfaces, decks and <br> other structures |  |
| $\checkmark$ | Location, size, type of existing structural stormwater controls, facilities and conveyance systems |  |

R. Post-Development Drainage Basin Area Mapping

| $\checkmark$ | $11^{\prime \prime} \times 17^{\prime \prime}$ or $8.5^{\prime \prime} \times 11^{\prime \prime}$ sheet size |
| :---: | :--- |
| $\checkmark$ | Topography, drainage patterns, drainage area boundaries and sub basins, flow paths, times of concentration |
| $\checkmark$ | N.A. |
|  |  | Locations of proposed stormwater discharges $\quad$ Perenial and intermittent streams, wetlands, and floodplain / floodways.

IV. DCIA Tracking Worksheet (as appendix to Project Report)
$\square$

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## V. Proposed LID Review Map

## Applying under "Lite" Stormwater Management - Proposed LID Review Map NOT required.

| A. | General |
| :---: | :---: |
| $\checkmark$ | Site address |
| $\checkmark$ | Applicant name, legal address, contact information |
| $\checkmark$ | Engineers name, address, contact information |
| $\checkmark$ | North arrow, bar scale, horizontal and vertical datum |
| $\checkmark$ | Drawing scale shall be set at $1^{\prime \prime}=20^{\prime}$ or $1^{\prime \prime}=40^{\prime}$ when possible |
| $\checkmark$ | Signed and stamped by a Licensed Professional Engineer in the State of Connecticut |
| $\checkmark$ | $11^{\prime \prime} \times 17^{\prime \prime}$ or $24^{\prime \prime} \times 36^{\prime \prime}$ sheet size unless otherwise approved |
| $\checkmark$ | Existing and proposed contours based on NAVD 88 at 2 foot contour interval or 1 foot contour interval when slope is flatter than 2 percent |
| $\checkmark$ | Locations of existing stormwater discharges |
| $\checkmark$ | Roads, buildings, driveways, parking areas, walks, patios, pools and other impervious surfaces, and decks and other structures |
| $\checkmark$ | Location, size, ownership of stormwater conveyance systems (swales, pipes, etc.) |


|  | B. | LID Constraints: |
| :---: | :---: | :---: |
|  | $\checkmark$ | Boring / test pit locations |
|  | $\checkmark$ | Infiltration testing locations and results |
|  | $\checkmark$ | Vegetation and proposed limits of clearing / disturbance |
|  | $\checkmark$ | NRCS soils mapping |
|  | $\checkmark$ | Steep slopes |
| N.A. |  | Surface waters / Perennial and intermittent streams |
|  | $\checkmark$ | Resource protection areas and buffers, wetlands, floodplain / floodways |
|  | $\checkmark$ | Existing vegetation and mature trees, which shall include 8-inch (dbh) diameter trees or greater |
|  | $\checkmark$ | Poor soils (HSG C \& D) |
|  | $\checkmark$ | Shallow bedrock / ledge |
|  | $\checkmark$ | Seasonal high groundwater elevation |
| N.A. |  | Other site constraints (e.g. brownfield caps) |

C. Proposed Stormwater Treatment Measures:

| $\checkmark$ | Location, size, type, limits, and WQV provided by each proposed stormwater treatment practices |
| :---: | :--- |
| $\checkmark$ | Drainage area to each proposed stormwater treatment practice (total area, impervious area, WQV) |

D. Site Summary Table:

| $\checkmark$ | Total site area, disturbed area, pre- and post-development impervious areas |
| :---: | :--- |
| $\checkmark$ | Required pollutant reduction volume (retention or detention) |
| $\checkmark$ | Provided pollutant reduction volume (retention or detention) |

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## Checklist for Stormwater Management Plan / Construction Plans

A.

| $\checkmark$ | General |
| :---: | :--- |
| $\checkmark$ | Site orientation, address and legal description |
| $\checkmark$ | Applicant name, legal address, contact information |
| $\checkmark$ | Engineers name, address, contact information arrow, bar scale, horizontal and vertical datum |
| $\checkmark$ | Drawing scale shall be set at $1^{\prime \prime}=20^{\prime}$ or $1^{\prime \prime}=40^{\prime}$ when possible |
| $\checkmark$ | Stamped by a Licensed Professional Engineer in the State of Connecticut |
| $\checkmark$ | $24^{\prime \prime} \times 36^{\prime \prime}$ sheet size unless otherwise approved |

B. Site Development Plans

| $\checkmark$ | City of Stamford Standard Notes |
| :--- | :--- |
| $\checkmark$ | As required by the Drainage Maintenance Agreement, provide a written narrative describing the nature of the <br> proposed development activity and the program for operation and maintenance of drainage facilities and control <br> measures throughout the life of the project. |
| $\checkmark$ | Existing and proposed contours based on NAVD 88 at 2 foot contour interval or 1 foot contour interval when slope <br> is flatter than 2 percent |
| $\checkmark$ | All required spot elevations to clearly depict positive pitch |
| $\checkmark$ | Top and bottom elevation of all walls |
| $\checkmark$ | Roads, buildings, driveways, parking areas, walks, patios, pools and other impervious surfaces, and decks and <br> other structures |
| $\checkmark$ | All utilities and easements |
| $\checkmark$ | Location, size, maintenance access, type of proposed structural stormwater controls and facilities with elevations <br> and inverts |
| $\checkmark$ | Location, size, maintenance access, type of proposed non-structural stormwater controls and facilities with <br> elevations and inverts |
| $\checkmark$ | Location, size, type of proposed stormwater infrastructure, inlets, manholes, infiltration and detentions systems, <br> control structures with elevations and inverts |
| $\checkmark$ | Location, size, ownership of stormwater conveyance systems (swales, pipes, etc.) with elevations and inverts |
| $\checkmark$ | Identify roof leaders, curtain drains and foundation drains with elevations and inverts |
| $\checkmark$ | Proposed water quality treatment systems, size and model type |
| $\checkmark$ | Final stabilization measures which may include slope stabilization |

C. Erosion and Sedimentation Control Plan

| $\checkmark$ | Phasing and schedule |
| :---: | :--- |
| $\checkmark$ | Construction access and staging and stock pile areas |
| $\checkmark$ | Operation and maintenance of erosion and sedimentation controls |
| $\checkmark$ | Tree protection |
| $\checkmark$ | Downstream protection such as location of silt fencing |
| $\checkmark$ | Limit of disturbance |
| $\checkmark$ | Construction fencing |

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D. Construction Details

| $\checkmark$ | Standard City of Stamford details |
| :---: | :--- |
| $\checkmark$ | Infiltration system details |
| $\checkmark$ | Control structure details |
| $\checkmark$ | Water quality treatment details |
| $\checkmark$ | Infiltration testing results |

## Checklist for Certificate of Occupancy

|  | Final Improvement Location Survey |
| :--- | :--- |
|  | Stormwater Management Certification Form |
|  | Final DCIA Tracking Worksheet |
|  | Standard City of Stamford Drainage Maintenance Agreement (Agreement Covenant) |

Other Certifications at the discretion of the Engineering Bureau and/or EPB

|  | Wall Certification |
| :--- | :--- |
|  | Landscape Certification |
|  | Landscape Maintenance Agreement |
|  | Waiver Covering Storm Sewer Connection |
|  | Waiver Covering Granite Block, Depressed Curb, and Driveway Aprons |
|  | Flood Certification |

## APPENDIX - I

# CITY OF STAMFORD <br> OFFICE OF OPERATIONS <br> BUREAU OF ENGINEERING 

## WAIVER COVERING STORM SEWER CONNECTION

WHEREAS, the CITY OF STAMFORD has granted to the undersigned, the privilege to connect with the City-owned storm water sewer located on

Street and being in the vicinity of property owned by undersigned, at $\qquad$ Street, Stamford, Connecticut and which privilege of connecting with said storm water sewer and has been or shall be at the expense of the undersigned.

NOW, THEREFORE, I $\qquad$ of, the CITY OF STAMFORD, COUNTY OF FAIRFIELD AND STATE OF CONNECTICUT, the undersigned, owning property located at $\qquad$ Street, Stamford, Connecticut for and in consideration of the privilege heretofore granted to me to connect with the storm water system of the City of Stamford, do hereby expressly waive any and all claims for damages after such connection has been made arising from the backing up of any water from said storm sewer onto my property or for failure of said storm water sewer to absorb any water origination on my said land or for the stoppage, failure or faulty construction of said storm water sewer where I have connected with the same or any part of the entire system, or for any other reason.

AND FURTHERMORE, I do hereby agree to hold the said City of Stamford free and harmless from any liability as aforesaid or from any suit or claim arising under the circumstances above stated, or from any suit or claim presented by any person claiming by, under or through me.

In the Present of:
$\qquad$
DATE

## STATE OF CONNECTICUT )

) ss.Stamford
COUNTY OF FAIRFIELD )
Personally appeared signer and sealer of the
foregoing instrument and acknowledged the same to be
free act and deed, before me.

## Notary Public

Date
$\qquad$

## AGREEMENT COVENANT

AGREEMENT made this $\qquad$ by and between and the CITY OF STAMFORD, a municipal corporation lying within the county of Fairfield and State of Connecticut, acting herein by its duly authorized Mayor, David R. Martin (hereinafter referred to as the "City"), and the ENVIRONMENTAL PROTECTION BOARD OF THE CITY OF STAMFORD, acting herein by its duly authorized Chairman, Gary H. Stone (hereinafter referred to as the "EPB").

WITNESSETH:

WHEREAS, OWNER has commenced the planning and construction of on a parcel of land owned by them and as more particularly described on Schedule "A", attached hereto and made a part hereof (the "Property").

WHEREAS, certain drainage facilities ("Drainage Facilities"), including but not limited to $\qquad$ as more particularly described on Schedule "B" attached (the "Construction Plans") shall be installed in connection with the aforesaid construction and in accordance with the Construction Plans and issued therefore, (the "Permit") and;

WHEREAS, OWNER, the CITY and EPB share a joint concern that the Drainage Facilities be maintained in a functioning condition so as to avoid pollution of surface and groundwaters, flooding and/or improper drainage.

NOW, THEREFORE, in consideration of ten dollars and other good and valuable consideration receipt of which is hereby acknowledged by the OWNER, it is hereby agreed as follows:

1) OWNER shall clean the drainage facilities or cause such facilities to be cleaned by periodic removal of accumulated sediment and debris in a good and workman-like manner, at least two (2) times during every twelve (12) month period, which times shall be in the period between April and June and between October and December and more often as the City may determine to be necessary.
2) OWNER shall sweep, or cause to be swept, garage facilities, driveways and roadway surfaces located on the Property at least once per calendar quarter.
3) OWNER shall utilize only sand or calcium chloride in connection with the de-icing of areas within the Property meaning and intending that road salt (Sodium Chloride) shall not be used for said purpose.
4) OWNER shall repair or replace any defects or defective drainage facilities so as to maintain the drainage facilities, at all times, in a fully functional capacity.
5) OWNER shall file as-built drainage plans with the EPB immediately upon the completion of work. Said plans shall be prepared by a professional engineer/surveyor registered in the state of Connecticut.
6) OWNER grants the CITY and/or EPB, its agents, and employees, the right to enter the Property at all reasonable times upon twentyfour (24) hours notice to the OWNER for the purpose of inspecting the Property to determine if OWNER is complying with the requirements hereunder. A representative of the Owner shall have the right to accompany the City and/or EPB on their inspection of the Property.
7) If, after an inspection is made pursuant to Paragraph Six hereof, the CITY and/or EPB determines that the owner has failed to comply with the aforesaid undertakings, then the CITY and/or EPB shall give written notice of said determination to the then OWNER of the Property which notice shall also specify the said failure. Said notice shall be sent by registered or certified mail to the last known address of said Owner. If the Owner disputes the claim, he shall give written notice thereof to City and/or EPB within ten (10) days of receipt of said notice, and the EPB shall hold a hearing as promptly as possible to decide the merits of the disputed claim. If the claim is not disputed within said ten (10) days, the OWNER shall have thirty (30) days from the receipt of said notice to correct said failure, unless it is impossible to cure said defect within said time, in which case, the necessary repairs shall be immediately commenced and diligently pursued to completion within a reasonable time.
8) If the said failure is not remedied within the time frame herein stated, the CITY and/or EPB may proceed to cure the same and charge the actual cost thereof to the OWNER of the Property.
9) OWNER agrees to reimburse the CITY and/or EPB for reasonable legal fees and court costs if it becomes necessary for the CITY and/or EPB to sue for reimbursement of sums expended by the CITY and/or EPB in performance of OWNER'S obligation.
10) OWNER agrees and covenants to indemnify and save harmless the CITY and the EPB against any and all claims, suits, actions or judgments arising out of the delay in the performance of any of their obligations pursuant to this Agreement.
11) OWNER agrees that this covenant and restriction shall apply to and run with the land. It shall be binding on all future owners, administrators, executors, successors and assigns.
12) The OWNER hereby represents to the CITY and EPB that he/she is the owner, in fee simple, of all of the property described in "Schedule A" attached hereto and made a part hereof.
13) OWNER agrees that this Agreement and restrictive covenant upon execution of the same, shall be recorded on the land records at the OWNER'S expense at the time that a permit is issued for the Property herein and while the OWNER is in title.
14) OWNER agrees not to assert the invalidity of this document.
15) OWNER agrees that nothing herein shall be construed to be a limitation upon the right of the EPB to assert and enforce any rights it may have under federal, state or City statute, ordinance or regulation.
16) This agreement shall be governed by the laws of the State of Connecticut.

IN WITNESS WHEREOF, the said parties hereto have hereunto set their hands and seals, the day and year first above written. WITNESSED:

# THE CITY OF STAMFORD 

BY:
David R. Martin
Its duly authorized Mayor

THE ENVIRONMENTAL PROTECTION BOARD

BY: $\qquad$
Gary H. Stone
Its duly authorized Chairman

OWNER

BY: $\qquad$
(Owner's Name)
(Acknowledgement on the Following Page)

STATE OF CONNECTICUT\}
\} ss: STAMFORD Date:
COUNTY OF FAIRFIELD \}
Personally appeared David R. Martin, Mayor of the City of Stamford, signer and sealer of the foregoing Instrument, and acknowledged the same to be his free act and deed and the free act and deed of said City, before me.

Commissioner of the Superior
Court or Notary Public
STATE OF CONNECTICUT\}
\} SS: STAMFORD Date:
COUNTY OF FAIRFIELD \}
Personally appeared Gary H. Stone, Chairman of the Environmental Protection Board of the City of Stamford, signer and sealer of the foregoing Instrument, and acknowledged the same to be his free act and deed and the free act and deed of said Commission, before me.

Commissioner of the Superior Court or Notary Public

STATE OF CONNECTICUT\}
\} $S S$ : STAMFORD
Date:
COUNTY OF FAIRFIELD \}
Personally appeared_signer and sealer of the foregoing instrument, and acknowledged the same to be__ free act and deed, before me.

Commissioner of the Superior Court or Notary Public

## SCHEDULE "A"

## SCHEDULE "B"


[^0]:    DOES ANY PORTION OF THE PREMISES AFFECTED BY THIS APPLICATION LIE WITHIN 500 FEET OF THE BORDER LINE WITH GREENWICH, DARIEN OR NEW CANAAN? No (If yes, notification must be sent to Town Clerk of neighboring community by registered mail within 7 days of receipt of application - PA $87-307$.

    DOES THE PROJECT RESULT IN THE CREATION OF 10 OR MORE UNITS OR 10,000 SF OR MORE IN FLOOR AREA OR DISTURBANCE OF 20,000 SF OR MORE IN LAND AREA, THROUGH NEW DEVELOPMENT, RECONSTRUCTION,
    ENLARGEMENT OR SUBSTANTIAL ALTERATIONS? $\qquad$ Yes $\qquad$ (If yes, then complete the Stamford Sustainability Scorecard per Section 15.F).

[^1]:    ${ }^{1}$ The Applicants recognize that retail is a desirable use in this space; however, to ensure this is an active frontage, the Applicants reserve the right to utilize this space for other purposes permitted in the MX-D infill zone, including resident amenity space, in the event an appropriate retail tenant cannot be obtained.

[^2]:    Source: Connecticut Crash Data Repository from October 1, 2018 to September 30, 2021.

[^3]:    $\begin{array}{lll}\mathrm{EB}=\text { Eastbound } & \mathrm{SB}=\text { Southbound } & \text { WB }=\text { Westbound } \\ \mathrm{T}=\text { Through } & \mathrm{R}=\text { Right Tum } & \text { APP. }=\text { Approach }\end{array}$

[^4]:    ${ }^{1}$ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).
    Numbers in parenthesis are PF estimates at lower and upper bounds of the $90 \%$ confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5\%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.
    Please refer to NOAA Atlas 14 document for more information.

