# CITY OF STAMFORD

# MAYOR CAROLINE SIMMONS

DIRECTOR OF OPERATIONS

MATT QUINONES

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CITY ENGINEER

LOUIS CASOLO, JR., P.E.
Email: LCasolo@StamfordCT.gov

#### INTEROFFICE MEMOR & NDUM

December 5, 2023

To: Vineeta Mathur Principal Planner

From: Willetta Capelle P.E. - Coordinator of Site Plan Reviews and Inspections

**Subject:** 

1911 Summer Street - KCI Summer, LLC Zoning Application No. 223-39, 223-40

The Engineering Bureau received Zoning Application documents for a Text Change proposing amendments to Section 7.3 of the Zoning Regulations to facilitate the construction of residential uses located on corner lots and to grant the Zoning Board the authority to reduce setbacks applicable to on-site parking areas. Also received was an application proposing Critical Reconstruction of a historic house and construction of 4 new townhouses attached to the rear along with landscaping and on-site parking.

The following documents were reviewed:

- -"TP-1 Topographic Survey Prepared for Daniel Kolich 1911 Summer Street" by Ahneman Kirby dated 8/16/21
- -"SP-1 Proposed Site Plan Prepared for Daniel Kolich 1911 Summer Street" by Ahneman Kirby latest revision 9/21/23
- -"SP-2 Proposed Site Details Prepared for Daniel Kolich 1911 Summer Street" by Ahneman Kirby dated 3/28/22
- -"Stormwater Management Report Prepared for Daniel Kolich 1911 Summer Street" by Ahneman Kirby dated 7/24/23

The Engineer of Record Keith Werner, P.E. of Ahneman Kirby, LLC stated: "Based on the above information the proposed improvements are designed in accordance with the City of Stamford Stormwater Drainage Manual and will not adversely impact adjacent or downstream properties or City-owned drainage facilities."

The Engineering Bureau does not object to the Text Change since it does not affect the authority of this department. Regarding the reconstruction of the historic house and construction of 4 new townhouses, the Engineering Bureau previously reviewed Zoning Board of Appeals documents for this project and the following items numbered from that review are still outstanding and are to be addressed by a CT Professional Engineer PRIOR to Zoning Board approval:

2) Connections to the City storm system must be via manhole and a Storm Sewer Connection Waiver will be

required. Revised the plan to connect to the manhole on 5th St. Storm Sewer Waiver will be submitted

5) Protect the infiltration system from over compacting during construction. Add a note to the Site Plan.

Added #10 in drainage note on SP-2

- 6) Provide a construction sequence, which shall include the protection of the infiltration system. Added #10 in general construction staging on SP-2
- 7) An additional test pit and saturated hydraulic conductivity test are needed in the proposed infiltration area. Provide the results. Additional testing results will be submitted to engineering
- 8) Identify proposed pipe types, lengths, diameters, slopes and functions. Done
- 10) Add hay bales or silt fence on the eastern and southern property lines in the area of work. Done
- 11) Identify all proposed grading. The majority of the contours will stay as it and proposed spot elevations are shown
- 12) Provide a detail of the parking area. 4 garage parking and 2 outdoor parking spaces are proposed. Refer to architectural plan
- 13) Show inlet protection for all catch basins and inlets. Ultra-Urban Filter TSS added.
- 15) Provide survey references on the Site Plan. Done
- 16) Add a sump elevation for CB #1 callout. Done
- 17) Extend the length of the anti-tracking pad. Extended to be 30' in length
- 19) A Drainage Maintenance Agreement will be required. Drainage Maintenance Agreement will be submitted
- 20) Identify the location of the construction vehicle staging area. Done

In addition to the above comments, the Engineering Bureau determined that the following must also be addressed prior to Zoning Board approval:

- A) Provide additional plan sheet(s) to clearly show the utility and drainage information, erosion and sediment controls and other site information, as well as all 4 proposed townhouses. S&E plan and site plan have been separated. Refer to architectural drawing
- B) The Stormwater Management Report states that 3 catch basins will collect the parking lot drainage, however, only 1 catch basin is shown on the Site Plan. Clarify and clearly show proposed contours. There is one as shown on site plan
- C) Provide a permanent insert for CB #1 for Water Quality improvement. AbTech details added
- D) Eliminate the 90 degree bend in the pipe from the infiltration system to the outlet control structure. The maximum allowable bend is 45 degrees, unless there is a structure at the bend.

Replaced 90° bend with two 45° with a short segment of PVC

- E) Pervious pavers are not permitted in parking areas without Water Quality pretreatment. Replaced with asphalt
- F) Add inlet protection to the Legend. Done
- G) The symbols for tree protection and tree removal are too similar. It appears that the triangle on the east property line and the X on the south property line identify tree protection and tree removal. Clarify.

Yes. Triangle is protection and X is removal

- H) Verify with the Zoning and Traffic Departments if the planted buffer along the 5th Street frontage is permitted.

  Refer to landscape Plan. It is designed per requirement to Section 12.k.2.b. and 12.k.5 of Stamford Zoning regulations
- I) Add a catch basin inlet protection detail. Done

- J) Add a detail for the permanent insert for Water Quality improvement. Done
- K) Catch basins in parking lot areas must be H20 load rated. Add this to the detail. Added to the callout & detail
- L) Provide an infiltration test and test pit within the footprint of the proposed infiltration system. The infiltration test results provided have varied time intervals and the inches dropped is not accurate for all time periods. The percolation rate should be shown in inches per minute, not minutes per inches. Clarify the type of infiltration test to be performed, to confirm compliance with the Stamford Drainage Manual. For the first 6 reading, it is measured 5 minutes apart and for the last 3 reading, it is 10 minute intervals. The constant rate is 1" in 10 min or 6" per hour. 30" deep hole was excavated for percolation test. Presoaked for 2 hour then reading started. Another test results will be submitted

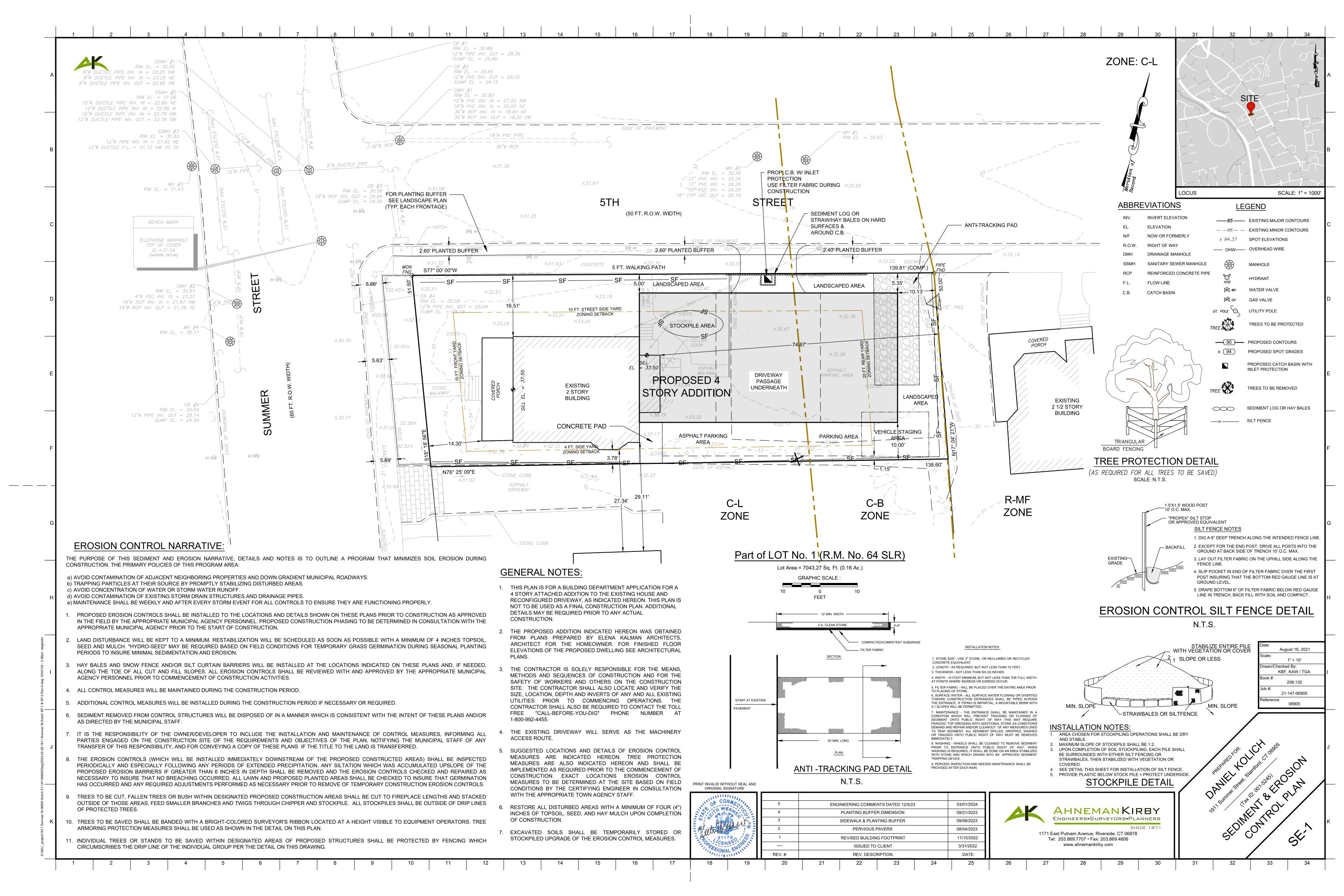
  M) Revise the drawdown calculations for the correct infiltration rate and test location.

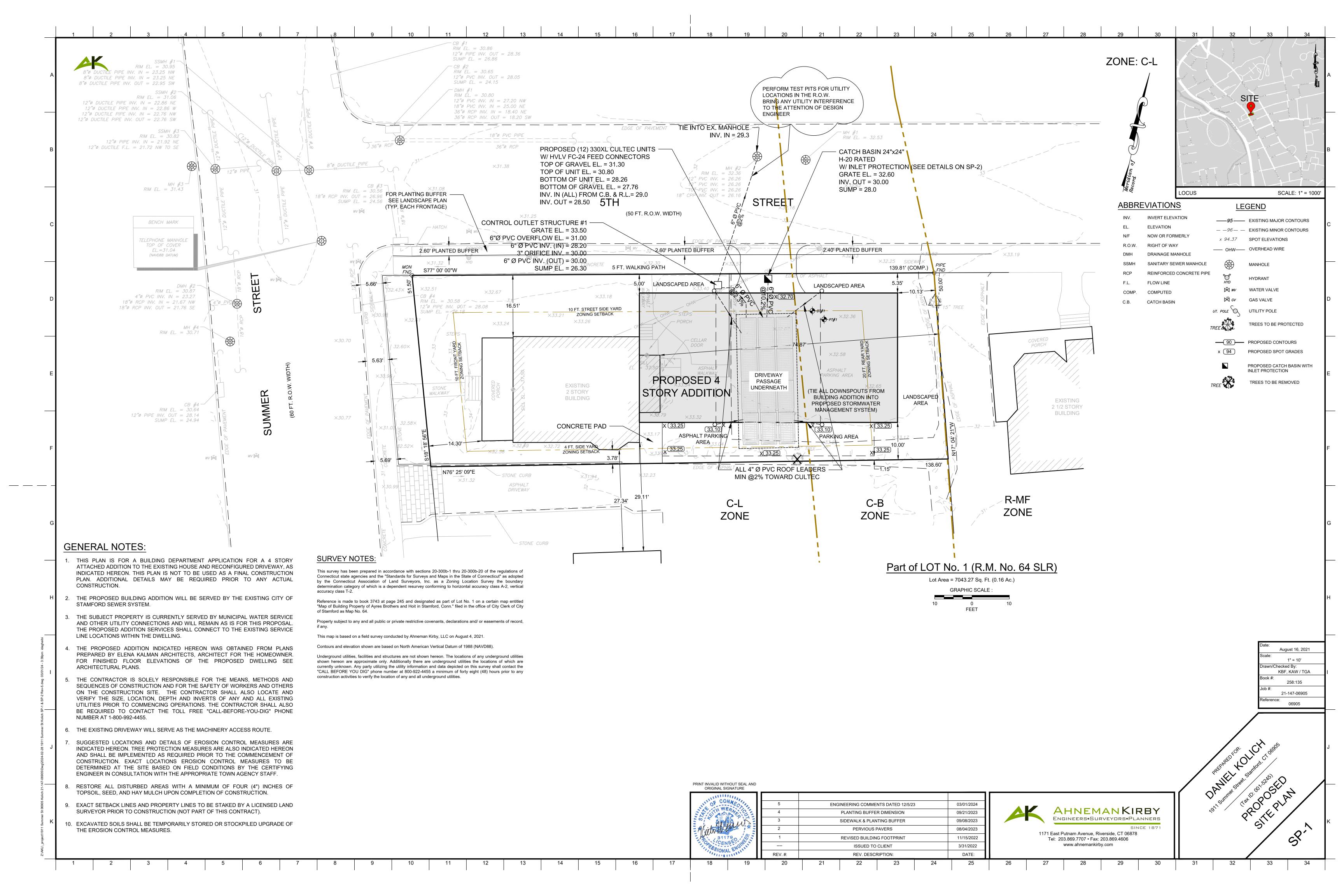
Revised percolation rate in table with inch per minute on SP-2. Revised the drawdown calculations

- N) Show all pipe connections to the infiltration system.
  - Roof leaders connections are shown
- O) Use a minimum Tc of 5 minutes for existing and proposed conditions in the drainage model. Done
- P) Provide a draft Drainage Maintenance Agreement. Done
- Q) An Excavation Permit will be required. Note is added as #1 in Standard City of Stamford Notes on SP-2
- R) The Engineering Bureau reserves the right to make additional comments.

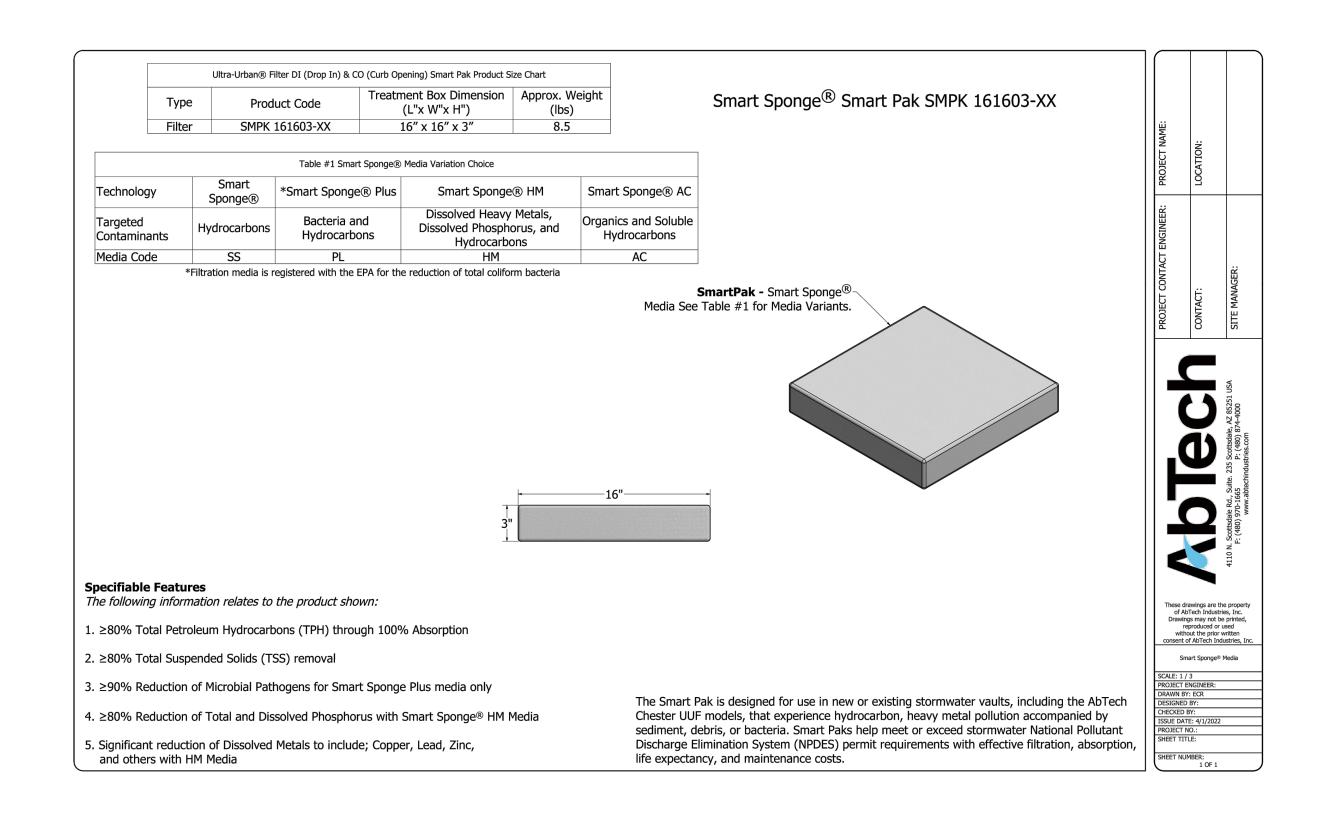
Please contact me at 203-977-4003 with any questions.

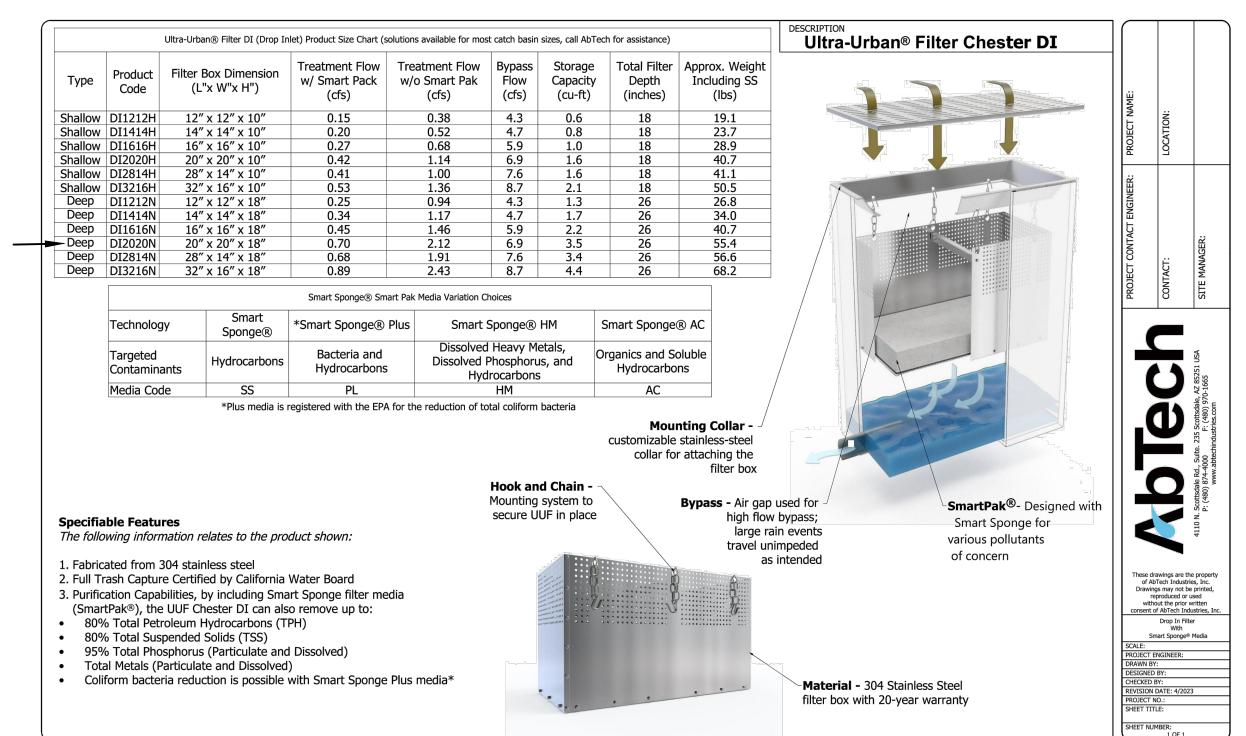
CC: Keith Werner Reg. No. 349











# PERMANENT INLET PROTECTION INSERT FOR C.B.

# GENERAL CONSTRUCTION STAGING:

- 1. INSTALL EROSION & SEDIMENT CONTROLS.
- 2. INSTALL TREE PROTECTION AS REQUIRED.
- 3. MARK AND CUT TREES TO BE REMOVED.
- 4. STRIP TOPSOIL AND STOCKPILE IT WITH APPROPRIATE SEDIMENTATION CONTROL MEASURES
- 5. ROUGH IN PROPOSED FOUNDATION AND CONSTRUCTION ACCESS.
- 6. EXCAVATE FOR PROPOSED FOUNDATION.
- 7. CONSTRUCT PROPOSED FOUNDATION.
- 8. BACKFILL AND ROUGH GRADE AROUND FOUNDATION.
- 9. INSTALL STORM DRAINAGE SYSTEM, AND UTILITIES AS REQUIRED.
- 10. USE INLET PROTECTION SUCH AS FILTER FABRIC ON CATCH BASIN AND HAYBALES AROUND IT DURING CONSTRUCTION.
- 11. PROTECT THE INFILTRATION SYSTEM FROM OVER COMPACTING DURING CONSTRUCTION (WORK WHEN SOIL IS DRY AND USE LIGHTWEIGHT MACHINES).
- 12. CONSTRUCT PROPOSED ADDITION.
- 13. CONSTRUCT STEPS AND WALKWAYS.
- 14. FINE GRADE AND STABILIZE ALL SLOPES.
- 15. LANDSCAPE AS REQUIRED.
- 16. INSERT PERMANENT INLET PROTECTION IN CATCH BASIN.
- 17. REMOVE EROSION AND SEDIMENT CONTROLS.

# Standard City of Stamford Notes:

- 1. A Street Opening Permit is required for all work within the City of Stamford Right-of-Way.
- 2. All work within the City of Stamford Right-of-Way shall be constructed to City of Stamford requirements, the State of Connecticut Basic Building Code and the Connecticut Guidelines for Soil Erosion and Sedimentation Control.
- 3. The Engineering Bureau of the City of Stamford shall be notified three days prior to any commencement of construction or work within the City of Stamford Right-
- 4. Trees within the City of Stamford Right-of-Way to be removed shall be posted in accordance with the Tree Ordinance.
- 5. Prior to any excavation the Contractor and/or Applicant/Owner, in accordance with Public Act 77-350, shall be required to contact "Call Before You Dig" at 1-800-922-4455 for mark out of underground utilities.
- 6. All retaining walls three (3) feet or higher measured from finished grade at the bottom of the wall to finished grade at the top of the wall and retaining walls supporting a surcharge or impounding Class I, II or III-A liquids are required to have a Building Permit. Retaining walls shall be designed and inspected during construction by a Professional Engineer licensed in the State of Connecticut. Prior to the issuance of a Certificate of Occupancy, retaining walls shall be certified by a Professional Engineer licensed in the State of Connecticut.
- 7. Certification will be required by a professional engineer licensed in the State of Connecticut that work has been completed in compliance with the approved
- 8. A Final Improvement Location Survey will be required by a professional land surveyor licensed in the State of Connecticut.
- 9. Connection to a city-owned storm sewer shall require the Waiver Covering Storm Sewer Connection to be filed with the City of Stamford Engineering Bureau.
- 10. Granite block or other decorative stone or brick, depressed curb, driveway apron and curbing within the City of Stamford Right-of-Way shall require the Waiver Covering Granite Block Depressed Curb and Driveway Aprons to be filed with the City of Stamford Engineering Bureau.
- 11. Sediment and erosion controls shall be maintained and repaired as necessary throughout construction until the site is stabilized.
- 12. To obtain a Certificate of Occupancy, submittal must include all items outlined in the Checklist for Certificate of Occupancy (Appendix D of the City of Stamford Drainage Manual).
- 13. Reference EPB Permit #, Zoning Permit #, Zoning Board of Appeals #, Subdivision #, if applicable.

# DRAINAGE NOTES:

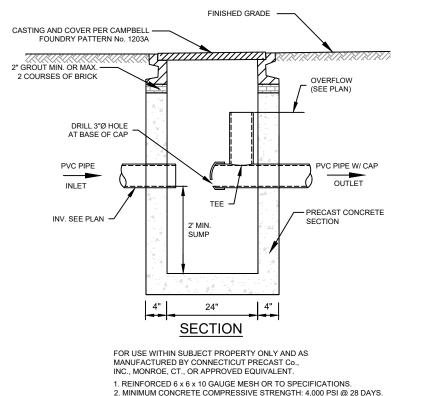
- 1. CONTRACTOR TO PROBE AND EXCAVATE WHERE POTENTIAL CONFLICTS MAY EXIST PRIOR TO DRAINAGE INSTALLATION. ALL KNOWN OR POTENTIAL CONFLICTS SHOULD BE BROUGHT TO THE ATTENTION OF THE DESIGN ENGINEER.
- 2. ALL NEW AND EXISTING AND FINISHED GRADES SHALL SLOPE TO DRAIN AWAY FROM THE PROPOSED AND EXISTING BUILDINGS.
- 3. ALL PVC PIPE TO BE SCHEDULE 40, OR EQUAL. MINIMUM PIPE PITCH SHALL BE 1-PERCENT.
- 4. ALL RETAINING WALLS GREATER THAN THREE FEET REQUIRE COMPUTATIONS SEALED AND SIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF CONNECTICUT TO BE SUBMITTED FOR ENGINEERING DIVISIONS RECORDS, PRIOR TO THE ISSUANCE OF A CERTIFICATE OF
- OCCUPANCY. THE DESIGN ENGINEER ASSUMES ALL RESPONSIBILITY OF THE RETAINING WALL DESIGN. 5. ALL DETENTION/RETENTION SYSTEMS SHALL BE INSTALLED PER MANUFACTURERS SPECIFICATIONS. ALL SYSTEMS SHALL USE A MANIFOLD SYSTEM

TO DISTRIBUTE RUNOFF EVENLY INTO EACH ROW OF INFILTRATORS. THE MANIFOLD SHALL BE INSTALLED ON THE INLET AND OVERFLOW SIDES

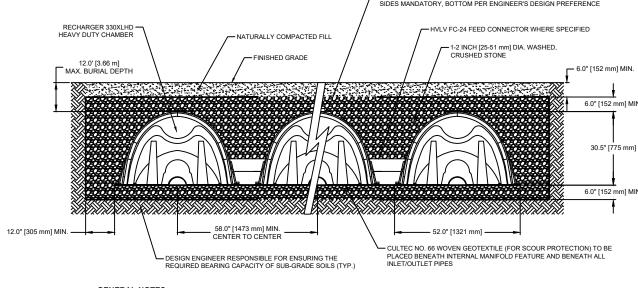
- 6. THERE MUST BE AT LEAST A 1-FOOT SEPARATION DISTANCE FROM THE BOTTOM OF STONE BELOW THE INFILTRATION STRUCTURE TO THE SEASONAL HIGH GROUNDWATER OR BEDROCK/LEDGE. PRIOR TO THE INSTALLATION OF THE INFILTRATORS THE ENGINEER SHALL VERIFY THE INFILTRATION STRUCTURE IS BEING INSTALLED IN THE APPROVED LOCATION AND IF THE LOCATION HAS BEEN CHANGED ADDITIONAL SOIL TESTING SHALL BE PERFORMED AND THE ENGINEER SHALL APPROVE THE REVISED LOCATION. A REVIEW BY THE APPROVING AUTHORITY WILL BE
- 7. EACH BMP TO BE INSTALLED SHALL HAVE THE SOILS BENEATH THE BMP SCARIFIED OR TILLED TO IMPROVE INFILTRATION.

WHEN NOT HANDLING THE FIRST FLUSH AND ONLY ON THE INLET SIDE WHEN A FIRST FLUSH SYSTEM IS BEING INSTALLED.

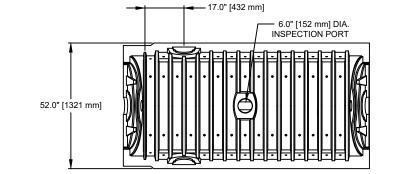
- 8. ALL AREAS THAT ARE USED BY CONSTRUCTION EQUIPMENT AND USED FOR CONTRACTOR PARKING MUST HAVE THE SOIL TILLED 12 TO 16 INCHES AND AMENDED WITH SMALL AMOUNTS OF ORGANIC MATERIAL IF NEEDED. THE AREA TO BE RESTORED SHALL BE DETERMINED BY THE SITE
- 9. A LETTER OF CERTIFICATION FROM A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF CONNECTICUT AND AN "IMPROVEMENT LOCATION SURVEY MAP" THAT DEPICTS ALL NEW "AS-BUILT" CONDITIONS FROM A SURVEYOR REGISTERED IN THE STATE OF CONNECTICUT SHALL BE SUBMITTED TO THE TOWN UPON COMPLETION OF WORK AND PRIOR TO THE ISSUANCE OF A CERTIFICATE OF OCCUPANCY BY THE BUILDING
- 10. PROTECT THE INFILTRATION SYSTEM FROM OVER COMPACTING DURING CONSTRUCTION.

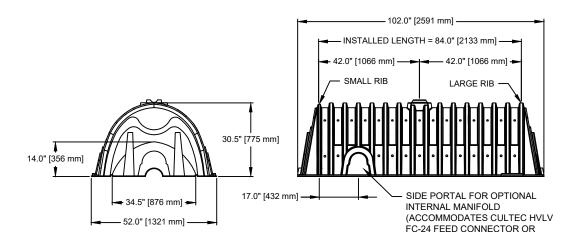


CONTROL OUTLET



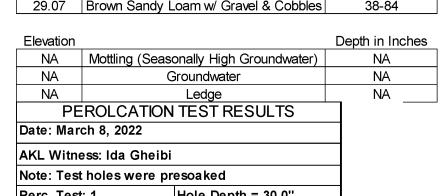
GENERAL NOTES
RECHARGER 330XL HD BY CULTEC, INC. OF BROOKFIELD, CT. ALL RECHARGER 330XL HD HEAVY DUTY UNITS ARE MARKED WITH A COLOR STRIPE FORMED INTO THE PART ALONG THE STORAGE PROVIDED = 11.32 CF/FT [1.05 m³/m] PER DESIGN UNIT. REFER TO CULTEC, INC.'S CURRENT RECOMMENDED INSTALLATION GUIDELINES. LENGTH OF THE CHAMBER ALL RECHARGER 330XL HD CHAMBERS MUST BE INSTALLED IN ACCORDANCE WITH ALL APPLICABLE LOCAL, STATE AND THE CHAMBER WILL BE DESIGNED TO WITHSTAND TRAFFIC LOADS WHEN INSTALLED ACCORDING TO CULTEC'S RECOMMENDED INSTALLATION INSTRUCTIONS





DEEP TESTS PERFORMED BY AKL 3/8/2022

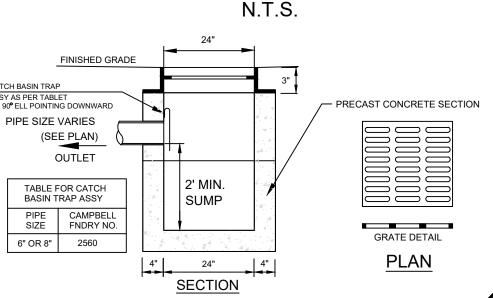
Test Pit or Soil Boring #: 1 Ground Elevation: 32.4		
	Soil Texture (Percent Sand, Silt and	Depth Range in
Elevation	Clay)	Inches
32.40	Asphalt	0-2
32.23	Dark Sandy Loam	2-38
29.07	Brown Sandy Loam w/ Gravel & Cobbles	38-84



I LINOLOATION ILOTTILOULIO				
Date: March 8, 2022				
AKL Witness: Ida Gheibi				
Note: Test holes were presoaked				
Perc. Test: 1		Hole Depth = 30.0"		
Time	Depth to Water (in)	Difference (in)	Perc. Rate (min/in)	
11:21 AM	7	-	-	
11:26 AM	8.5	1.5	0.3	
11:31 AM	9.5	1.0	0.2	
11:36 AM	10.5	1.0	0.2	
11:41 AM	11.5	1.0	0.2	
11:46 AM	12	0.5	0.1	
11:56 AM	13	1.0	0.1	
12:06 PM	14	1.0	0.1	
12:16 PM	15	1.0	0.1	
	Cor	nstant rate =	0.10	in/m
			6 00	in/

INSTALLED LENGTH ADJUSTMENT = 1.5' [0.46 m] SIDE PORTAL ACCEPTS CULTEC HVLV FC-24 FEED CONNECTOR **CULTEC RECHARGER 330XLHD** 

CULTEC RECHARGER 330XLHD CHAMBER STORAGE = 7.459 CF/FT [0.693 m³/m]



March 28, 2022 AS SHOWN KW / TGA 258:135 21-147-06905 06905

11.75 INCH [298 mm] MAX. O.D. PIPE

FOR USE WITHIN SUBJECT PROPERTY ONLY AND AS MANUFACTURED BY CONNECTICUT PRECAST Co., INC., MONROE, CT., OR APPROVED EQUIVALENT. 1. REINFORCED 6 x 6 x 10 GAUGE MESH OR TO SPECIFICATIONS. 2. MINIMUM CONCRETE COMPRESSIVE STRENGTH: 4,000 PSI @ 28 DAYS

PRECAST CONCRETE CATCH BASIN (CB) N.T.S.

AHNEMANKIRBY ENGINEERS SURVEYORS PLANNERS

1171 East Putnam Avenue, Riverside, CT 06878 Tel: 203.869.7707 • Fax: 203.869.4606 www.ahnemankirby.com

PRINT INVALID WITHOUT SEAL AND ORIGINAL SIGNATURE **ENGINEERING COMMENTS DATED 12/5/23** 03/01/2024 **REV. DESCRIPTION:** DATE:

REV. #:

# OPERATIONS AND MAINTENANCE PLAN REPORT

Prepared for:

# Daniel Kolich 1911 Summer St, Stamford, CT 06905

Prepared by:



Ahneman Kirby, LLC 1171 East Putnam Avenue Riverside, Connecticut 06878

March 1, 2024

# **Operations and Maintenance Plan**

1911 Summer St, Stamford, CT 06905 March 1, 2024

#### Scope:

The purpose of the Operations and Maintenance Plan is to ensure that the existing and proposed stormwater components installed at 1911 Summer St, Stamford, CT 06905 are maintained in operational condition throughout the life of the project. The service procedures associated with this plan shall be performed as required by the parties legally responsible for their maintenance.

#### Recommended Frequency of Service:

As further defined below, all stormwater components should be checked on a periodic basis and kept in full working order. Ultimately, the required frequency of inspection and service will depend on runoff quantities, pollutant loading, and clogging due to debris. At a minimum, we recommend that all stormwater components be inspected and serviced twice per year, once before winter begins and once during spring cleanup.

### **Qualified Inspector:**

The inspections must be completed by an individual experienced in the construction and maintenance of stormwater drainage systems. Once every five years the inspections must be completed by a professional engineer.

#### Service Procedures:

#### 1. Catch Basins & Drainage Inlets:

- a. Catch basins and drainage inlets shall be completely cleaned of accumulated debris and sediments at the completion of construction.
- b. For the first year, catch basins and drainage inlets shall be inspected on a quarterly basis.
- c. Any accumulated debris within the catch basins/inlets shall be removed and any repairs as required.
- d. From the second year onward, visual inspections shall occur twice per year, once in the spring and once in the fall, after fall cleanup of leaves has occurred.
- e. Accumulated debris within the catch basins/inlets shall be removed and repairs made as required.
- f. Accumulated sediments shall be removed at which time they are within 12 inches of the invert of the outlet pipe.
- g. Any additional maintenance required per the manufacturer's specifications shall also be completed.

#### 2. Storm Drainage Piping and Manholes/Junction Boxes:

- a. All storm drainage piping shall be completely flushed of debris and accumulated sediment at the completion of construction.
- b. Manholes/Junction Boxes shall be inspected and repaired on an annual basis.

- c. Unless system performance indicates degradation of piping, comprehensive video inspection of storm drainage piping shall occur once every ten years.
- d. Any additional maintenance required per the manufacturer's specifications shall also be completed.

#### 3. Stormwater Control Structures:

- a. All control structures (orifice, weir, etc.) shall be completely cleaned of accumulated debris and sediments at the completion of construction. Any repairs shall be performed.
- b. For the first year, control structures (orifice, weir, etc.) shall be inspected on a quarterly basis
- c. Any accumulated debris shall be removed and any repairs made to the control structures (orifice, weir, etc.) as required.
- d. From the second year onward, visual inspections shall occur twice per year, once in the spring and once in the fall, after fall cleanup of leaves has occurred.
- e. Accumulated debris shall be removed and repairs made as required.
- f. Any additional maintenance required per the manufacturer's specifications shall also be completed.

#### 7. <u>Drywells and Infiltration Systems:</u>

- a. All drywells/infiltrators shall be completely cleaned of accumulated debris and sediments upon the completion of construction.
- b. For the first year, the drywells/infiltrators shall be inspected on a quarterly basis.
- c. Any accumulated debris within the drywells/infiltrators shall be removed and any repairs made to the units as required.
- d. From the second year onward, visual inspection shall occur twice per year, once in the spring and once in the fall, after fall cleanup of leaves has occurred.
- e. Accumulated debris within the units shall be removed and repairs made as required.
- f. Any additional maintenance required per the manufacturer's specifications shall also be completed.

#### 11. Roof Gutters:

a. Remove accumulated debris and inspect for damage. Any damage should be repaired as required.

#### **Disposal of Debris and Sediment:**

All debris and sediment removed from the stormwater structures and bioretention/biofiltration basins shall be disposed of legally. There shall be no dumping of silt or debris into or in proximity to any inland or tidal wetlands.

#### Maintenance Records:

The Owners(s) must maintain all records (logs, invoices, reports, data, etc.) and have them readily available for inspection at all times.

# Operations and Maintenance Log (Page 1 of 5) 1911 Summer St, Stamford, CT 06905 March 1, 2024

Inspector's Name: Date of Inspection:  Affiliation: Phone #:  Catch Basins & Drainage Inlets:		
Catch Basins & Drainage Inlets:	П№П	
	⊓Мо⊓І	
	□ No □ I	N/A
Notes:		
Storm Drainage Piping and Manholes/Junction Boxes:		
<ul> <li>Do any manholes require additional repair? (identify below):</li> <li>□ Yes</li> <li>Is there any evidence of stormwater piping failure?</li> <li>□ Yes</li> </ul>	□ No □   □ No □   □ No □	N/A N/A
Notes:		
Stormwater Control Structures:		
<ul> <li>Are any repairs required? (identify below):</li> </ul>	□ No □ I □ No □ I □ No □ I	N/A
Notes:		

# Operations and Maintenance Log (Page 2 of 5) 1911 Summer St, Stamford, CT 06905 March 1, 2024

# Hydrodynamic Separators:

<ul> <li>Has accumulated debris been removed?</li> <li>Does unit require additional repair? (identify below):</li> <li>Has unit been cleaned of sediment?</li> </ul>	☐ Yes ☐ No ☐ N/A ☐ Yes ☐ No ☐ N/A ☐ Yes ☐ No ☐ N/A
Notes:	
Drainage Outfalls/Splash Pads/Scour Holes/Level Spreaders:	
<ul> <li>Have all drainage outlets been cleared of debris?</li> <li>Have all outlet protections been inspected/repaired?</li> <li>Have all erosion issues been repaired?</li> </ul>	☐ Yes ☐ No ☐ N/A ☐ Yes ☐ No ☐ N/A ☐ Yes ☐ No ☐ N/A
Notes:	
Bioretention/Biofiltration Basins/Rain Gardens:	
<ul> <li>Have basins been cleared of debris/sediments?</li> <li>Have draining times of basins been verified?</li> <li>Has vegetation been mowed (twice/year max.)?</li> <li>Has plantings and mulch been replaced (twice/year)?</li> </ul>	☐ Yes ☐ No ☐ N/A
Notes:	

# Operations and Maintenance Log (Page 3 of 5) 1911 Summer St, Stamford, CT 06905

March 1, 2024

# **Drywells and Infiltration Systems:**

<ul> <li>Have units been cleared of debris/sediments?</li> <li>Do units require additional repair? (identify below):</li> <li>Has draining times of system been verified?</li> </ul>	☐ Yes	□ No □ □ No □ □ No □	1 N/A
Notes:			
Porous Pavement:			
<ul> <li>Has pavement been vacuumed?</li> </ul>		□ No □	-
Has draining times been verified?	□ Yes	□ No □	] N/A
Notes:			
Gravel Pavement:			
<ul> <li>Has pavement been graded and additional gravel added?</li> </ul>		□ No □	-
<ul> <li>Has draining times been verified?</li> </ul>	□ Yes	□ No □	] N/A
Notes:			

Operations and Maintenance Log (Page 4 of 5)

1911 Summer St, Stamford, CT 06905

March 1, 2024

# Vegetative Roof:

<ul> <li>Have trays been cleared of debris/sediments?</li> <li>Have draining times of trays been verified?</li> <li>Has vegetation been weeded (bi-weekly)</li> <li>Have roof drains been inspected and cleared of debris</li> </ul>	☐ Yes ☐ Yes ☐ Yes ☐ Yes ☐ Yes	□ No □ □ No □ □ No □ □ No □	N/A N/A
Notes:			
Roof Gutters:			
<ul> <li>Has accumulated debris been removed from gutters?</li> <li>Do any gutters require additional repair? (identify below):</li> </ul>	□ Yes □ Yes	□ No □ □ No □	
Notes:			
Groundwater Pump System:			
<ul> <li>Has the electrical connections been inspected?</li> <li>Has the electrical connections for the generator been inspected?</li> <li>Has the generator been exercised?</li> <li>Has the sump been cleaned? (identify below):</li> </ul>	□ Yes □ Yes	□ No □ □ No □ □ No □ □ No □	N/A N/A
Notes:			

# **Operations and Maintenance Log (Page 5 of 5)**

1911 Summer St, Stamford, CT 06905 March 1, 2024

# Stormwater Pump System:

**Signature of Inspector:** 

<ul> <li>Has the electrical connections been inspected?</li> <li>Has the electrical connections for the generator been inspected?</li> <li>Has the generator been exercised?</li> <li>Has the sump been cleaned? (identify below):</li> </ul>	☐ Yes ☐ No ☐ N/A
Notes:	
Please make additional notes/observations and particular concerns b additional maintenance that has been performed:	elow. Also record any

Date



# Operation, Maintenance Manual Ultra Urban Filter Chester Drop In





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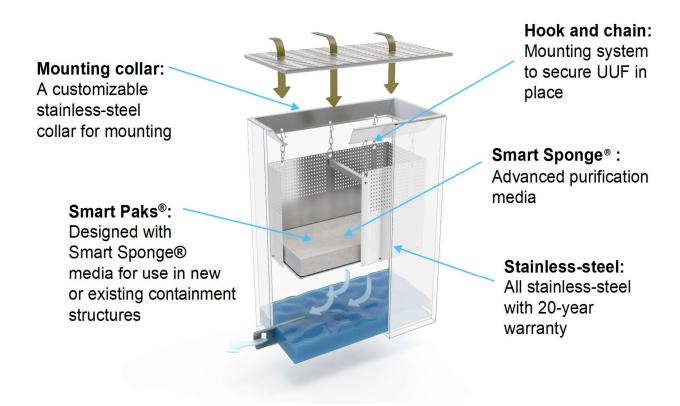
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# 1.0 Description

The Ultra-Urban® Filter (UUF) Chester series is a passive, flow-through, stainless-steel, stormwater filtration system with an optional interchangeable SmartPak® media insert with Smart Sponge® technology that targets additional contaminants of concern. The Chester UUFs with or without the SmartPak, are California Water Board Certified for Full Trash Capture. There are two types of models: the UUF "Drop-In" (UUF DI) and the UUF "Curb Opening" (UUF CO). Although the O&M guidelines are similar, this Guide is specific for the UUF Chester DI models.

The UUF Chester DI filter boxes are an all-stainless-steel design comprised of a collar, suspension chains, filter box, and optional SmartPak. The UUF is installed in catch basins just beneath the grate and utilizes gravity to operate without any mechanical parts or electricity. The unit has holes of 5mm in size to reduce trash and debris. With the optional SmartPak, will additionally reduce, TSS >80%, sediment, and both particulate and dissolved contaminants. Please see our media guide for more information on specific contaminant reduction.







# 2.0 Operation

The UUF filter box is positioned beneath the frame and inlet grate of stormwater catch basin structures. Untreated stormwater water can enter from any side of the inlet grate and funnels with the collar into the filter insert via gravity. This untreated stormwater continues through the filter screen at the bottom of the unit and will be filtered to less than 5mm in particle size. If a SmartPak is installed, it will provide the additional filtration for other pollutants of concern. The filtered water will flow out of the unit to be discharged. The filter is designed with an integrated bypass system consisting of an air gap to prevent backup in the case of a major storm event or the substantial accumulation of debris.

# 3.0 Inspection & Maintenance Overview

#### 3.1 General

Catch basin inspection, maintenance and historic documentation is an integral part of any comprehensive stormwater management plan. An inspection program that is regularly reviewed is necessary to ensure the UUF is operating as designed to provide the necessary pollutant removal. The frequency in which catch basins are cleaned is based on site-specific factors. It is important to closely monitor and document the first year of operation after initial installation to develop a long-term maintenance plan for the filter that is consistent with actual pollutant loadings.

# 3.2 Frequency and Timing of Inspections and Maintenance

In general, all treatment systems need to be inspected and maintained on a routine basis. The frequency and timing of the visits will vary based on the configuration of the filter, site-specific conditions such as rainfall, vegetation, surrounding ground type, and the geographic region. During the first year of operation, after initial installation, the UUF should be inspected more frequently to create a baseline of understanding of the operation of the filter and contaminant loading. Subsequent years of operation can have inspection frequency adjusted based on working experience, provided that no unusual events have occurred during the year. For most installations, AbTech recommends the following:

- First-Year Schedule 1) Quarterly inspections. 2) An inspection at the start and end of the rainfall season. 3) An inspection after each major rainfall event. Inspection visits may require maintenance.
- Subsequent Years Schedule Based on the first-year inspection findings, frequency of inspections may be adjusted. For example, if during the first year, the filter and location are determined to have high pollutant loadings, additional



inspections and maintenance may be necessary. Conversely, if pollutant loadings are minimal, frequency of visits may be reduced. Inspection visits may require maintenance.

### 3.3 Inspection & Maintenance Safety and Equipment Considerations

Safety is the most important consideration before inspecting and removing pollutants from the UUF Chester DI. Always employ proper traffic management and handling procedures for all inspections/maintenance where vehicles and pedestrians have access. Disposing of waste liquids and solids may be regulated and should be understood before removing waste products from the treatment system. Urban stormwater drainage structures are often installed along roadside curbs or in parking lots with limited space.

Examples of things to consider during inspection and maintenance:

- Personal Protective Equipment (PPE) reflective vests, glasses, steel-toed shoes, gloves.
- Allowing personnel space to remove and temporarily store surface grates.
- Maneuvering and parking maintenance vehicles.
- Equipment for directing traffic and pedestrians safety cones or barriers and use of appropriate signage.
- Tools to loosen consolidated sediment and debris covering the manway/grate.
- Equipment for removing the manway/grate (e.g. crowbar, manhole hook, jib crane).
- Storing and disposal of pollutants.
- Inspection Data Sheet.

In the event of accidental or chemical spill, contact emergency services and follow standard hazmat procedures.

# 4.0 Inspection Procedures

The UUF Chester DI can be typically inspected without entry into the catch basin. The inspection should begin by preparing all safety measures on site then followed by the inspection and documentation process. Specific procedures for the inspection are detailed below:

- 1. Wear all PPE and prepare documentation equipment.
- 2. Install all work zone safety equipment and conduct a brief safety meeting. Work zone safety equipment should protect the inspector(s) from vehicular traffic and should also isolate and protect pedestrians and vehicles from the work zone.



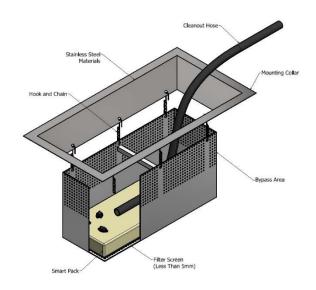
- 3. Remove the grate utilizing a puller/remover and safely set aside out of the way of the inspection operations and pedestrians or vehicles.
- 4. Inspect the grate and catch basin lip. The areas outside of the catch basin should be free from debris, obstructions, and standing water. The presence of any of these conditions outside of the catch basin are potential indicators of maintenance that may be necessary for the treatment system. If any of these maintenance indicators are encountered, they should be documented and, depending on severity, should be rectified through recommended maintenance.
- 5. Inspect the inside of the catch basin. A flashlight may be needed depending on lighting. The interior of the catch basin and pipe outlet(s) should be free from debris, obstructions, mosquitos, and in some cases standing water. The presence of any of these conditions in the interior of the catch basin are potential indicators of maintenance that may be necessary for the treatment system. The presence of standing water or mosquitos should be highlighted per vector control procedures. The local vector control agency should be notified if mosquitos are present in the catch basin or filter unit.
- 6. Inspect the collar and filter enclosure for physical or structural damage. The collar should be firmly mounted to the catch basin lip and there should be no loose or missing hardware. The filter should be supported by the suspension hooks and chains. Bent, broken, or otherwise damaged structural components should be documented and recommended for replacement.
- 7. Inspect the filter screen for pollutants. Pollutants such as trash and debris, and sediment are expected to be captured inside of the treatment system. The presence of such pollutants are indicators the filter is operating as intended. However, an unusually low quantity of such pollutants in the filter may be an indicator that the water is not being properly directed into the filter. The amount of pollutants should be documented and compared with the maximum capacities for the filter.
- 8. Inspect the SmartPak media filter (if included). When equipped with the optional SmartPak, the media filter fabric should be inspected for rips or tears. The Smart Sponge® media darkens in color as it collects pollutants, but with the media fabric around the Smart Pak®, it may be difficult to observe. If an oily film has accumulated on top of the media fabric, it may indicate a need to be replaced. See maintenance section for recommended change out schedule.
- 9. Finalize the Inspection Data Sheet. Photograph the conditions of interior and exterior of the catch basin and filter unit. Document the inspection event utilizing the Inspection Data Sheet included with this manual or similar.
- 10. Replace the grate and remove all work zone safety equipment.



### 5.0 Maintenance Procedures

The UUF Chester DI is typically maintained without entry into the catch basin and requires very little time. The maintenance should begin by preparing and installing all safety measures followed by the maintenance and documentation. Specific procedures for the maintenance are detailed below:

- 1. Wear all PPE and prepare documentation equipment.
- 2. Install all work zone safety equipment and conduct a brief safety meeting. Work zone safety equipment should protect the inspector(s) from vehicular traffic and should also isolate and protect pedestrians and vehicles from the work zone.
- 3. Remove the grate utilizing the puller/remover and safely set aside out of the way from inspection/maintenance operations and pedestrians or vehicles.
- 4. If during inspection it is determined the accumulated trash, debris, and sediment requires removal, an industrial vacuum can be utilized to remove the material. Using a reduced diameter suction hose, vacuum the trash, debris, and sediment from the catch basin filter. The suction hose may be inserted into the filter through catch basin opening as illustrated below. A pressure washing wand may be utilized to assist this process by freeing clogged material from the enclosure screen or media fabric. The suction hose should remain inside the filter while the filter is being washed down. It is also possible to remove sediment by hand by removing the filter enclosure from the catch basin.



- 5. If during inspection it is determined the media requires replacement, the following replacement procedures should be implemented. Replacement media should be preordered in advance of the maintenance visit. After removal of sediment, remove the SmartPak from the filter box. Additional cleaning may be needed under the SmartPak where sediment may have accumulated on the filter screen. Once cleaned, place a new SmartPak into the bottom of the box, is flat, and fits tightly.
- 6. Removed trash, debris, and sediment, and media should be disposed of following local, state, and federal guidelines. Material disposal is discussed in section 5.0.
- 7. The presence of standing water or mosquitos should be highlighted per vector control procedures. The local vector control agency should be notified if mosquitos are present in the catch basin or filter unit.



- 8. Finalize the Maintenance Report. Photograph the conditions of interior and exterior of the catch basin and filter unit. Document the maintenance event utilizing the Maintenance Report included with this manual or similar.
- 9. Replace the grate cover and remove all work zone safety equipment.

# 6.0 Smart Sponge Media Replacement & Disposal

## 6.1 Media Replacement

AbTech has determined through extensive testing and maintenance history, that replacing the SmartPak every 18 months to two years ensures regulatory compliance in most installations. However, media replacement can be determined by weight, contact info@abtechindustries.com for directions on this method.

## 6.2 Media Disposal

Generally, spent Smart Sponge filtration media used in typical stormwater applications saturated with hydrocarbons have been classified as a non-hazardous substance. Smart Sponge filtration media saturated with hydrocarbons, both in the lab and field settings, have been tested according to the EPA's Toxicity Characteristic Leaching Procedure (TCLP). The tests indicated that Smart Sponge is a non-leaching product and may be disposed of as follows:

- Subtitle D Landfills
- Waste-to-Energy Facilities
- Thermal Conversion Process Facilities
- Cement Kilns

#### Note:

In some cases, it is possible Smart Sponge and accumulated sediment found in a stormwater treatment system may contain measurable amounts of hazardous material. Areas with the greatest potential for hazardous pollutant loading include industrial areas and heavily traveled roads. These contaminants must be disposed of in accordance with all applicable waste disposal regulations. When scheduling maintenance, consideration must be made for the disposal of solid and liquid wastes and may require coordination with a local landfill for solid waste disposal. For liquid waste disposal, several options are available including a municipal vacuum truck decant facility, local wastewater treatment plant or on-site treatment and discharge.



# 7.0 Sample Reports

# 7.1 Inspection Report

Date	Personnel
Location	System Size

No.	Inspection Item	
1	Is settled trash, debris, and/or sediment on the surrounding area? Is the grate occluded?	Yes No
2	Is standing water surrounding the catch basin?	Yes No
3	Check the collar for structural integrity. Are there any abnormalities?	Yes No
4	Are any of the hooks or chains: bent, broken, or missing?	Yes No
5	Check the filter enclosure for structural integrity. Is there any damage to the enclosure?	Yes No
6	Is there any damage to the filter screen?	Yes No
7	Is there trash and debris, and sediment accumulated in the filter? If so, document the qty. If yes, then must be removed as part of the maintenance plan.	Yes No





8	Is there a black or oily sheen on top of the Smart Pak®? (if included). If yes, then media must be replaced as part of maintenance plan.	Yes No
9	Is there standing water inside the catch basin?	Yes No
10	Do you observe any mosquitos? If yes, contact vector control as per local guidelines.	Yes No



# 7.2 Maintenance Report

No.	Maintenance Activity	Date
1	Set up appropriate safety equipment and put on safety gear.	
2	Collect and remove trash, debris, etc. surrounding the catch basin and on the grate.	
3	Remove the trash, debris, and sediment from the filter using a vacuum hose, or by hand	
4	Repair or replace damaged or deteriorated structural components such as collar, chains, hooks, or enclosure.	
5	Remove obstructions from filter screen.	
6	Remove the spent media and replace with new media.	
7	Conduct O+M procedures as needed for any instrumentation, valves, and other devices. Repair or replace as needed.	
8	Notify agency or owner representative.	



# 8.0 Warranty

AbTech Industries, Inc. (AbTech) warrants to buyer that the Ultra-Urban Filter Chester Stainless Steel Filter ("UUF Chester") shall materially conform to the description in AbTech's product documentation as of the sale date and shall be free from defects in material and workmanship for twenty (20) years from the date of purchase as referenced by the sales order number printed on the Chester UUF or by invoice. This warranty is transferable and is conditioned on: (a) the UUF Chester being properly installed and maintained at least semi-annually from the time of installation, (b) no unauthorized repairs or alterations having been made to the UUF Chester, (c) buyer not being in default on any contractual obligation for the UUF Chester, (d) any warranty claim being provided to AbTech in writing within thirty (30) days of buyer's identification of the suspected defect. This warranty specifically excludes coverage of any damage caused by circumstances beyond the control of the party affected, including without limitation acts of God, fire, vandalism, natural disaster, chemical action, abrasive material, misuse, explosion, war, action or demand of governmental authority, injunction, or labor strikes, or improper or unauthorized installation or repairs.

This warranty does not apply to (i) any consumable or wearable parts used in conjunction with the UUF Chester, or other parts which are designed to diminish or wear over time, (eg SmartPaks); or (ii) damage caused by use with a third-party component or product that is not provided by AbTech. The warranties set forth herein are AbTech's sole and exclusive warranties and are in lieu of all other warranties, remedies, and conditions, whether oral, written statutory, express or implied. AbTech disclaims all statutory and implied warranties, including without limitation, warranties of merchantability, fitness for a particular purpose and non-infringement.

If AbTech responds to a claim from buyer under this warranty, and it is later determined that the claim is not, in fact, covered by this warranty, buyer shall pay AbTech its then customary charges for any repair or replacement made by AbTech. IN NO EVENT WILL ABTECH BE LIABLE FOR SPECIAL, INDIRECT, INCIDENTAL, CONSEQUENTIAL OR EXEMPLARY DAMAGES, SUCH AS, BY WAY OF EXAMPLE AND NOT LIMITATION, LOSS OF REVENUES, BUSINESS OPPORTUNITIES OR GOODWILL, ARISING OUT OF OR IN CONNECTION WITH THE INFRASTRUCTURE SLEEVE, HOWSOEVER CAUSED, WHETHER OR NOT BUYER HAS BEEN ADVISED, KNEW OR SHOULD HAVE KNOWN OF THE POSSIBILITY OF SUCH DAMAGES.

# Stormwater Management Report

Prepared for:

Daniel Kolich 1911 Summer St Stamford, CT 06905

March 28, 2022 Revision 1: March 1, 2024

### Prepared by:



Ahneman Kirby, LLC
1171 East Putnam Avenue
Riverside, Connecticut

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# **Project Narrative**

Property of Daniel Kolich 1911 Summer St, Stamford, CT 06903 March 1, 2024

#### A. GEOGRAPHICAL LOCATION AND DESCRIPTION

The subject parcel is located on the East side of Summer St, it is a corner lot located at the intersection of Summer St and 5<sup>th</sup> Street and has a lot area of 0.16 Acres. The topographic nature of the lot is fairly flat sloped from South to North. The property contains an existing dwelling, porch, parking lot, stone and asphalt walkways, with trees.

#### B. PURPOSE AND DESCRIPTION

This application package proposes a new 2397 ft<sup>2</sup> of a 4-story building addition containing 4 new townhouses and new curb cut with parking area on the parcel. The proposed building with the existing building footprint is 3,600 ft<sup>2</sup>. The proposed parking area is 1579 ft<sup>2</sup> including 971 ft<sup>2</sup> of under building driveway (See Appendix A). The regrading of the lot keeps the topography of the site going from South to North at a rate of approximately 1% in the front and rear yards.

Drainage design was performed in accordance with the City of Stamford's Stormwater Drainage Manual, with a net zero increase in the rate of runoff for all events up to the 50 year storm. We proposed collecting runoff from the proposed building and parking area and treating it with Stamford's Stormwater Best Management Practices (BMP).

The area of the site being collected is in the rear yard. The stormwater will be collected by one (1) proposed catch basin in the proposed parking area. The storm drain piping conveys the stormwater from catch basin and the proposed building addition through roof leaders to twelve (12) Cultec 330XL recharger basins located under the proposed parking area. The outlet from the Cultecs will then be routed to a control outlet structure to control the discharge rate. From the controlled outlet the runoff is directed to an existing Stamford Manhole located at the 5<sup>th</sup> Street (See Plans).

#### C. SOIL EVALUATION

The soils within the site below the surface are 100% Type D per the USDA Natural Resource Conservation Service and are depicted on the soils map located in Appendix B of this report as follows:

• Urban land (map unit symbol 307)

Refer to Appendix B for USDA Soils Engineering Properties.

#### D. PRE & POST DEVELOPMENT SITE HYDROLOGY COMPARISON

The proposed development increases the impervious coverage for the watershed but will decrease peak flows to all points of concern. A series of roof leader downspouts and catch basin will pick up the runoff from the newly introduced impervious surfaces. The proposed grades slope towards the same location as the existing grades making for



a straight forward comparison of pre and post development hydrology at the common Points of Interest.

Refer to Table 1 & 2 for a comparison of peak flow rates for the existing and proposed site conditions at point of interest A and point of interest B, respectively. The peak runoff to all points of concern has a zero increase for the 1, 2, 5, 10, 25, and 50 year storms. Based on the above information, the proposed improvements are designed in accordance with the City of Stamford Stormwater Drainage Manual and will not adversely impact adjacent or downstream properties or City-owned drainage facilities.

1911 Summer St, Stamford, CT - P.O.I "A"							
Existing / Proposed Stormwater Runoff Data Comparison Chart							
STORM EVENT	POINT OF INTEREST	Flow/Volume	EXISTING	PROPOSED	Δ	Δ (%)	
1 YEAR	TOTAL FLOW P.O.I. A	q(ft <sup>3</sup> /s)	0.17	0.14	-0.03	-17.65%	
2 YEAR		q(ft <sup>3</sup> /s)	0.21	0.18	-0.03	-14.29%	
5 YEAR		q(ft <sup>3</sup> /s)	0.29	0.25	-0.04	-13.79%	
10 YEAR		q(ft <sup>3</sup> /s)	0.36	0.30	-0.06	-16.67%	
25 YEAR		q(ft <sup>3</sup> /s)	0.48	0.40	-0.08	-16.67%	
50 YEAR		q(ft <sup>3</sup> /s)	0.58	0.49	-0.09	-15.52%	
100 YEAR		q(ft <sup>3</sup> /s)	0.71	0.60	-0.11	-15.49%	

Table 1: Comparison of Existing and Proposed Peak Flow Rates for Point of Interest A

Table 2: Comparison of Existing and Proposed Peak Flow Rates for Point of Interest B

1911 Summer St, Stamford, CT - P.O.I "B"								
Existing / Proposed Stormwater Runoff Data Comparison Chart								
STORM EVENT	POINT OF INTEREST	Flow/Volume	EXISTING	PROPOSED	Δ	Δ (%)		
1 YEAR	TOTAL FLOW P.O.I. B	q(ft <sup>3</sup> /s)	0.16	0.02	-0.14	-87.50%		
2 YEAR		q(ft <sup>3</sup> /s)	0.19	0.03	-0.16	-84.21%		
5 YEAR		q(ft <sup>3</sup> /s)	0.25	0.05	-0.20	-80.00%		
10 YEAR		q(ft <sup>3</sup> /s)	0.29	0.06	-0.23	-79.31%		
25 YEAR		q(ft <sup>3</sup> /s)	0.37	0.16	-0.21	-56.76%		
50 YEAR		q(ft <sup>3</sup> /s)	0.44	0.25	-0.19	-43.18%		
100 YEAR		q(ft <sup>3</sup> /s)	0.53	0.59	0.06	11.32%		

#### E. ALTERNATIVES CONSIDERED

The alternatives considered included drywells collecting runoff from catch basins in the driveway and Cultecs installed under the lawn in the front yard collecting the front portion of the existing building. A level spreader discharge point was also considered.

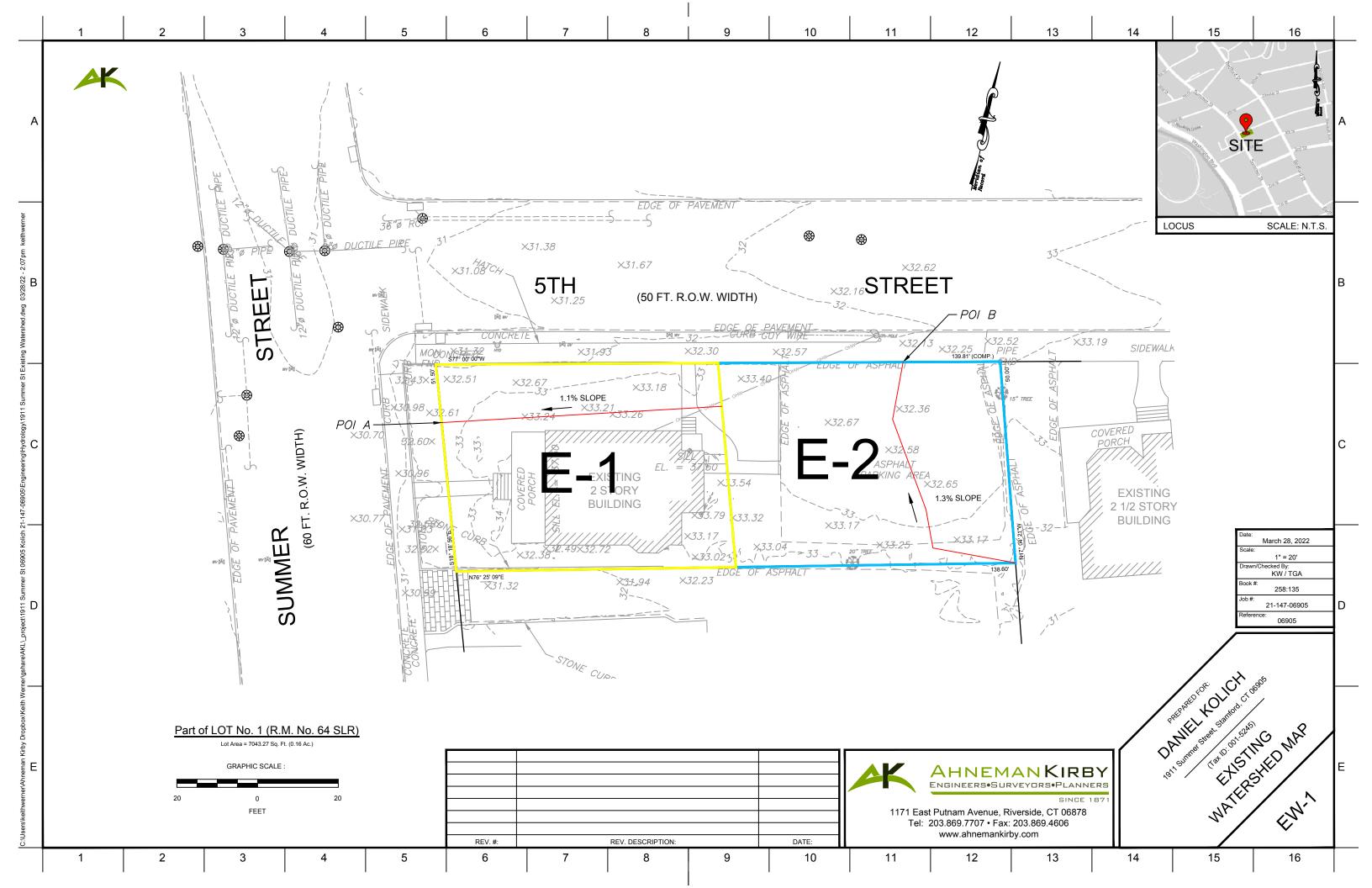
The drywells were discarded due to their limited capacity.

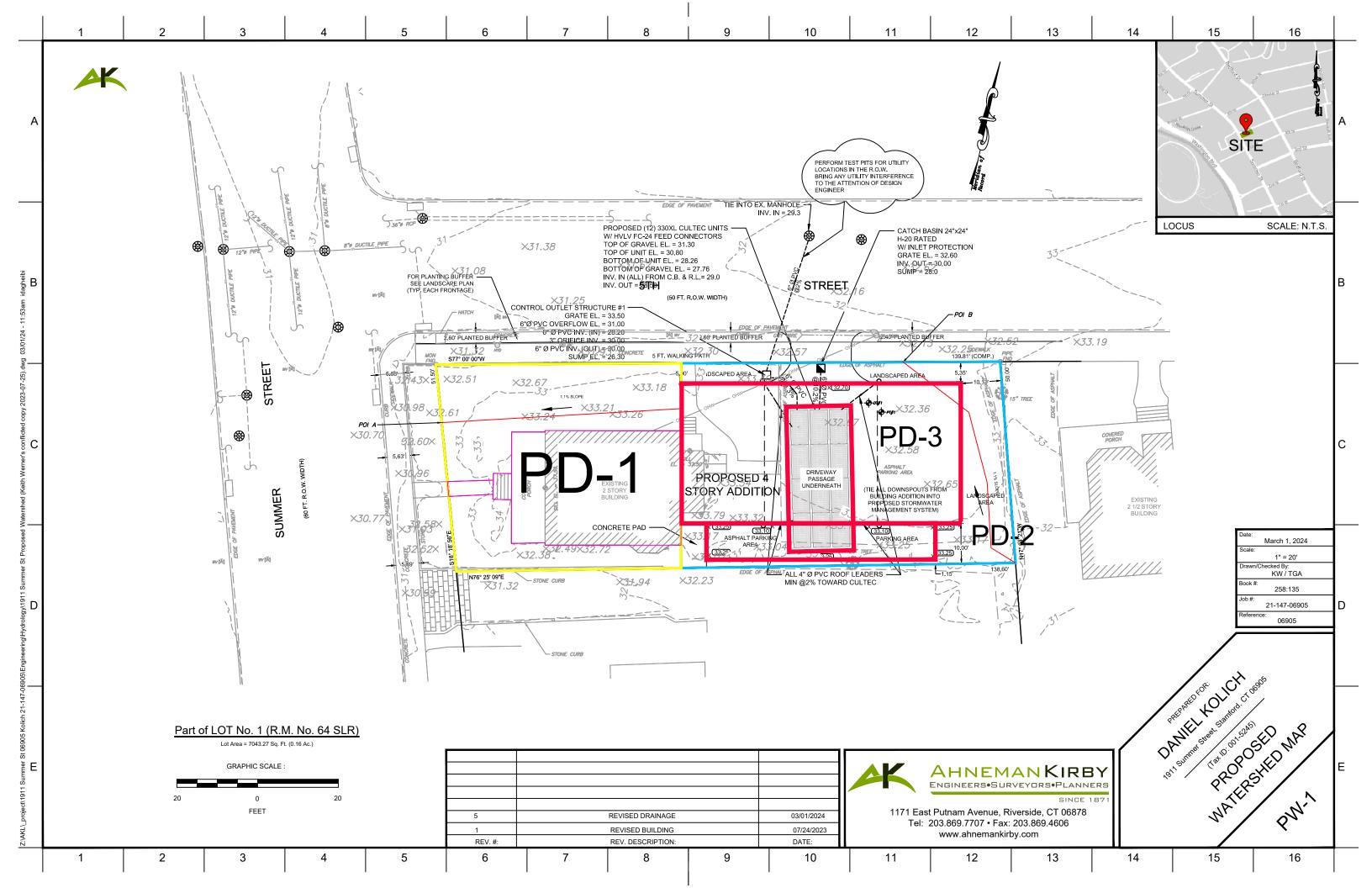
The Cultecs in the front yard were discarded due to limiting the area of disturbance to the rear yard where the other work will be taking place.

The level spreader was discarded due to space limitations on site and it would introduce a concentrated surface discharge point.



# Appendix A Impervious Coverage Pre & Post Development







# **Appendix B USDA Soils Engineering Properties**



#### MAP LEGEND MAP INFORMATION The soil surveys that comprise your AOI were mapped at Area of Interest (AOI) С 1:12.000. Area of Interest (AOI) C/D Soils Warning: Soil Map may not be valid at this scale. D Soil Rating Polygons Enlargement of maps beyond the scale of mapping can cause Not rated or not available Α misunderstanding of the detail of mapping and accuracy of soil **Water Features** line placement. The maps do not show the small areas of A/D Streams and Canals contrasting soils that could have been shown at a more detailed Transportation B/D Rails ---Please rely on the bar scale on each map sheet for map measurements. Interstate Highways C/D Source of Map: Natural Resources Conservation Service **US Routes** Web Soil Survey URL: D Major Roads Coordinate System: Web Mercator (EPSG:3857) Not rated or not available -Local Roads Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts Soil Rating Lines Background distance and area. A projection that preserves area, such as the Aerial Photography Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below. B/D Soil Survey Area: State of Connecticut Survey Area Data: Version 21, Sep 7, 2021 Soil map units are labeled (as space allows) for map scales 1:50.000 or larger. Not rated or not available Date(s) aerial images were photographed: Oct 4, 2020—Oct 31. 2020 **Soil Rating Points** The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background A/D imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident. B/D

# **Hydrologic Soil Group**

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
307	Urban land	D	0.2	100.0%
Totals for Area of Interest			0.2	100.0%

# **Description**

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

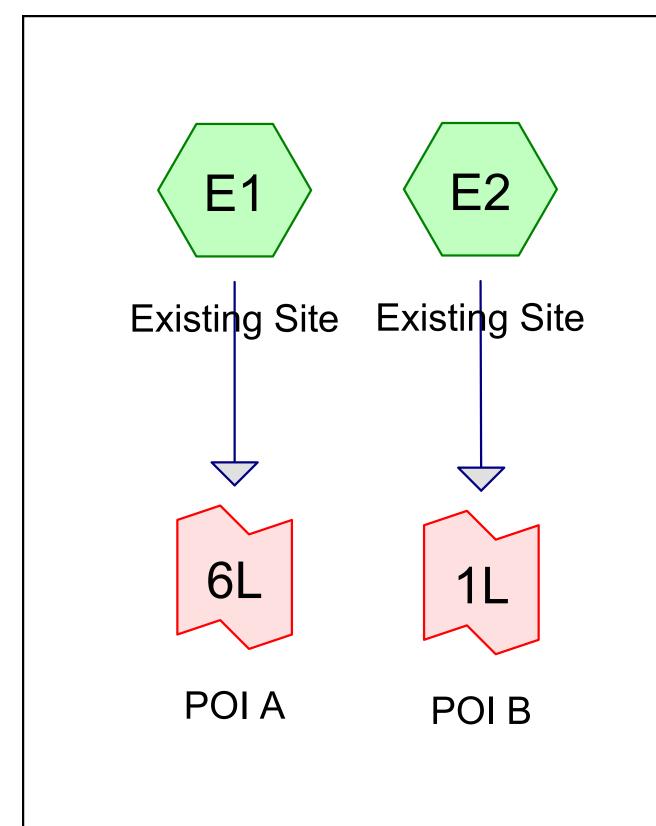
# **Rating Options**

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified



# **Appendix C HydroCAD Pre & Post Development Calculations**











2024-03-01 1911 Summer St\_HydroCAD

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## **Area Listing (selected nodes)**

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
2,042	80	>75% Grass cover, Good, HSG D (E1)
1,273	98	Existing Dwelling (E1)
2,413	98	Existing Partial Driveway (E2)
288	98	Existing Walkways (E1, E2)
6,016	92	TOTAL AREA

2024-03-01 1911 Summer St\_HydroCAD

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## **Ground Covers (selected nodes)**

HS	SG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground
(	sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	Cover
	0	0	0	2,042	0	2,042	>75% Grass
							cover, Good
	0	0	0	0	1,273	1,273	Existing Dwelling
	0	0	0	0	2,413	2,413	Existing Partial
							Driveway
	0	0	0	0	288	288	Existing
							Walkways
	0	0	0	2,042	3,974	6,016	TOTAL AREA

Su Νι

Type III 24-hr 1-Year Rainfall=2.90"

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment E1: Existing Site Runoff Area=3,546 sf 42.41% Impervious Runoff Depth>1.73"

Tc=5.0 min CN=88 Runoff=0.17 cfs 511 cf

Subcatchment E2: Existing Site Runoff Area=2,470 sf 100.00% Impervious Runoff Depth>2.67"

Tc=5.0 min CN=98 Runoff=0.16 cfs 549 cf

Link 1L: POI B Inflow=0.16 cfs 549 cf

Primary=0.16 cfs 549 cf

Link 6L: POI A Inflow=0.17 cfs 511 cf

Primary=0.17 cfs 511 cf

Total Runoff Area = 6,016 sf Runoff Volume = 1,060 cf Average Runoff Depth = 2.11" 33.94% Pervious = 2,042 sf 66.06% Impervious = 3,974 sf

Type III 24-hr 2-Year Rainfall=3.40"

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment E1: Existing Site Runoff Area=3,546 sf 42.41% Impervious Runoff Depth>2.18"

Tc=5.0 min CN=88 Runoff=0.21 cfs 643 cf

Subcatchment E2: Existing Site Runoff Area=2,470 sf 100.00% Impervious Runoff Depth>3.16"

Tc=5.0 min CN=98 Runoff=0.19 cfs 651 cf

Link 1L: POI B Inflow=0.19 cfs 651 cf

Primary=0.19 cfs 651 cf

Link 6L: POI A Inflow=0.21 cfs 643 cf

Primary=0.21 cfs 643 cf

Total Runoff Area = 6,016 sf Runoff Volume = 1,294 cf Average Runoff Depth = 2.58" 33.94% Pervious = 2,042 sf 66.06% Impervious = 3,974 sf

Type III 24-hr 5-Year Rainfall=4.30"

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment E1: Existing Site Runoff Area=3,546 sf 42.41% Impervious Runoff Depth>3.01"

Tc=5.0 min CN=88 Runoff=0.29 cfs 888 cf

Subcatchment E2: Existing Site Runoff Area=2,470 sf 100.00% Impervious Runoff Depth>4.06"

Tc=5.0 min CN=98 Runoff=0.25 cfs 836 cf

Link 1L: POI B Inflow=0.25 cfs 836 cf

Primary=0.25 cfs 836 cf

Link 6L: POI A Inflow=0.29 cfs 888 cf

Primary=0.29 cfs 888 cf

Total Runoff Area = 6,016 sf Runoff Volume = 1,724 cf Average Runoff Depth = 3.44" 33.94% Pervious = 2,042 sf 66.06% Impervious = 3,974 sf

Type III 24-hr 10-Year Rainfall=5.10"

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment E1: Existing Site**Runoff Area=3,546 sf 42.41% Impervious Runoff Depth>3.76"

Tc=5.0 min CN=88 Runoff=0.36 cfs 1,111 cf

Subcatchment E2: Existing Site Runoff Area=2,470 sf 100.00% Impervious Runoff Depth>4.86"

Tc=5.0 min CN=98 Runoff=0.29 cfs 1,000 cf

Link 1L: POI B Inflow=0.29 cfs 1,000 cf

Primary=0.29 cfs 1,000 cf

Link 6L: POI A Inflow=0.36 cfs 1,111 cf

Primary=0.36 cfs 1,111 cf

Total Runoff Area = 6,016 sf Runoff Volume = 2,112 cf Average Runoff Depth = 4.21" 33.94% Pervious = 2,042 sf 66.06% Impervious = 3,974 sf

Type III 24-hr 50-Year Rainfall=7.60"

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment E1: Existing Site Runoff Area=3,546 sf 42.41% Impervious Runoff Depth>6.17"

Tc=5.0 min CN=88 Runoff=0.58 cfs 1,824 cf

Subcatchment E2: Existing Site Runoff Area=2,470 sf 100.00% Impervious Runoff Depth>7.36"

Tc=5.0 min CN=98 Runoff=0.44 cfs 1,514 cf

Link 1L: POI B Inflow=0.44 cfs 1,514 cf

Primary=0.44 cfs 1,514 cf

Link 6L: POI A Inflow=0.58 cfs 1,824 cf

Primary=0.58 cfs 1,824 cf

Total Runoff Area = 6,016 sf Runoff Volume = 3,338 cf Average Runoff Depth = 6.66" 33.94% Pervious = 2,042 sf 66.06% Impervious = 3,974 sf

Type III 24-hr 100-Year Rainfall=9.10"

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment E1: Existing Site Runoff Area=3,546 sf 42.41% Impervious Runoff Depth>7.64"

Tc=5.0 min CN=88 Runoff=0.71 cfs 2,258 cf

Subcatchment E2: Existing Site Runoff Area=2,470 sf 100.00% Impervious Runoff Depth>8.85"

Tc=5.0 min CN=98 Runoff=0.53 cfs 1,822 cf

Link 1L: POI B Inflow=0.53 cfs 1,822 cf

Primary=0.53 cfs 1,822 cf

Link 6L: POI A Inflow=0.71 cfs 2,258 cf

Primary=0.71 cfs 2,258 cf

Total Runoff Area = 6,016 sf Runoff Volume = 4,080 cf Average Runoff Depth = 8.14" 33.94% Pervious = 2,042 sf 66.06% Impervious = 3,974 sf

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## **Ground Covers (selected nodes)**

HSG.	-A HSG	-B HSG-C	HSG-E	Other	Total	Ground
(sq-	ft) (sq-	ft) (sq-ft	) (sq-ft	(sq-ft)	(sq-ft)	Cover
	0	0 0	2,042	2 0	2,042	>75% Grass
						cover, Good
	0	0 (	) (	1,273	1,273	<b>Existing Dwelling</b>
	0	0 (	) (	2,413	2,413	<b>Existing Partial</b>
						Driveway
	0	0 (	) (	288	288	Existing
						Walkways
	0	0 (	2.04	2 3.974	6.016	TOTAL AREA

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Type III 24-hr 25-Year Rainfall=6.40"

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment E1: Existing Site Runoff Area=3,546 sf 42.41% Impervious Runoff Depth>5.01"

Tc=5.0 min CN=88 Runoff=0.48 cfs 1,480 cf

Subcatchment E2: Existing Site Runoff Area=2,470 sf 100.00% Impervious Runoff Depth>6.16"

Tc=5.0 min CN=98 Runoff=0.37 cfs 1,267 cf

Link 1L: POI B Inflow=0.37 cfs 1,267 cf

Primary=0.37 cfs 1,267 cf

Link 6L: POI A Inflow=0.48 cfs 1,480 cf

Primary=0.48 cfs 1,480 cf

Total Runoff Area = 6,016 sf Runoff Volume = 2,747 cf Average Runoff Depth = 5.48" 33.94% Pervious = 2,042 sf 66.06% Impervious = 3,974 sf HydroCAD® 10.00-26 s/n 01998 © 2020 HydroCAD Software Solutions LLC

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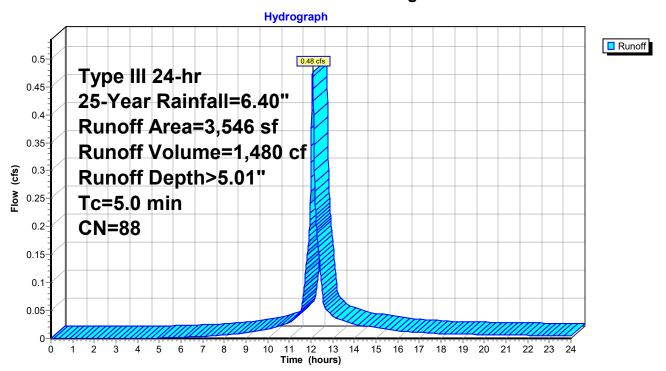
## **Summary for Subcatchment E1: Existing Site**

Runoff = 0.48 cfs @ 12.07 hrs, Volume= 1,480 cf, Depth> 5.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=6.40"

	Ar	ea (sf)	CN	Description				
*		1,273	98	Existing Dw	elling			
*		231	98	Existing Wa	alkways			
		2,042	80	>75% Gras	s cover, Go	Good, HSG D		
		3,546 2,042 1,504	88	Weighted A 57.59% Per 42.41% Imp	vious Area			
(	Tc min)	Length (feet)	Slop (ft/f	,	Capacity (cfs)	· ·		
	5.0					Direct Entry,		

### **Subcatchment E1: Existing Site**



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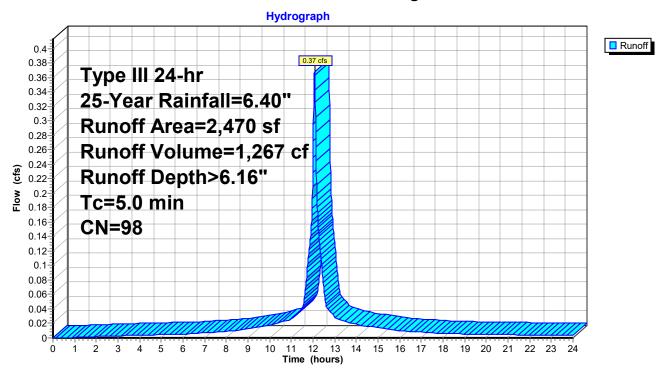
## **Summary for Subcatchment E2: Existing Site**

Runoff = 0.37 cfs @ 12.07 hrs, Volume= 1,267 cf, Depth> 6.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=6.40"

	Α	rea (sf)	CN	Description						
*		2,413	98	Existing Pa	rtial Drivew	/ay				
*		57	98	Existing Wa	Existing Walkways					
		2,470	98	Weighted Average						
		2,470		100.00% In	npervious A	Area				
	Tc (min)	Length (feet)	Slop (ft/f	,	Capacity (cfs)	Description				
	5.0	· , ,	,	, , ,	,	Direct Entry,				

### **Subcatchment E2: Existing Site**



Type III 24-hr 25-Year Rainfall=6.40"

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## **Summary for Link 1L: POI B**

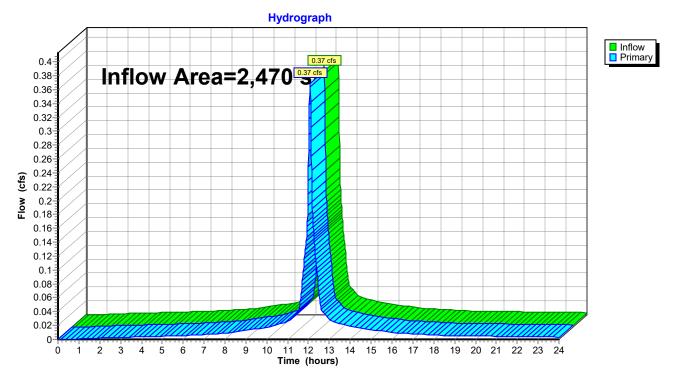
Inflow Area = 2,470 sf,100.00% Impervious, Inflow Depth > 6.16" for 25-Year event

Inflow = 0.37 cfs @ 12.07 hrs, Volume= 1,267 cf

Primary = 0.37 cfs @ 12.07 hrs, Volume= 1,267 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

#### Link 1L: POI B



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## Summary for Link 6L: POI A

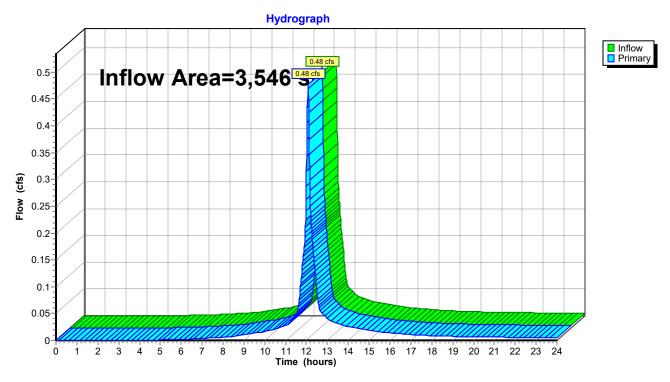
3,546 sf, 42.41% Impervious, Inflow Depth > 5.01" for 25-Year event Inflow Area =

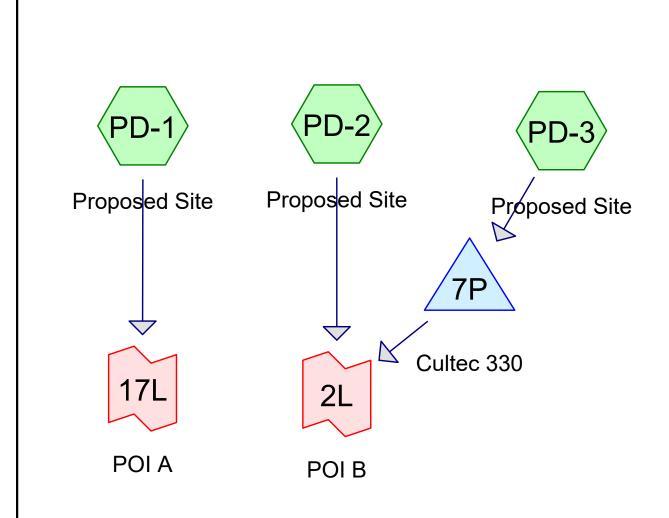
1,480 cf Inflow =

0.48 cfs @ 12.07 hrs, Volume= 0.48 cfs @ 12.07 hrs, Volume= 1,480 cf, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

#### Link 6L: POI A













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## **Area Listing (selected nodes)**

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
2,426	80	>75% Grass cover, Good, HSG D (PD-1, PD-2)
1,184	98	Existing Dwelling (PD-1)
70	98	Existing Walkways (PD-1)
607	98	Proposed Driveway (PD-3)
2,397	98	Proposed Partial Building (PD-3)
67	98	Proposed walkway (PD-3)
6,751	92	TOTAL AREA

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## **Ground Covers (selected nodes)**

HSG-	A HSG-B	HSG-C	HSG-D	Other	Total	Ground
(sq-f	t) (sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	Cover
	0 0	0	2,426	0	2,426	>75% Grass
						cover, Good
	0 0	0	0	1,184	1,184	<b>Existing Dwelling</b>
	0 0	0	0	70	70	Existing
						Walkways
	0 0	0	0	607	607	Proposed
						Driveway
	0 0	0	0	2,397	2,397	Proposed Partial
						Building
	0 0	0	0	67	67	Proposed
						walkway
	0 0	0	2,426	4,325	6,751	TOTAL AREA

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Type III 24-hr 1-Year Rainfall=2.90"

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PD-1: Proposed Site Runoff Area=2,972 sf 42.19% Impervious Runoff Depth>1.73"

Tc=5.0 min CN=88 Runoff=0.14 cfs 428 cf

Subcatchment PD-2: Proposed Site Runoff Area=708 sf 0.00% Impervious Runoff Depth>1.17"

Tc=5.0 min CN=80 Runoff=0.02 cfs 69 cf

Subcatchment PD-3: Proposed Site Runoff Area=3,071 sf 100.00% Impervious Runoff Depth>2.67"

Tc=5.0 min CN=98 Runoff=0.21 cfs 683 cf

Pond 7P: Cultec 330 Peak Elev=29.73' Storage=682 cf Inflow=0.21 cfs 683 cf

Outflow=0.00 cfs 0 cf

Link 2L: POI B Inflow=0.02 cfs 69 cf

Primary=0.02 cfs 69 cf

Link 17L: POI A Inflow=0.14 cfs 428 cf

Primary=0.14 cfs 428 cf

Total Runoff Area = 6,751 sf Runoff Volume = 1,180 cf Average Runoff Depth = 2.10" 35.94% Pervious = 2,426 sf 64.06% Impervious = 4,325 sf

Type III 24-hr 2-Year Rainfall=3.40"

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PD-1: Proposed Site Runoff Area=2,972 sf 42.19% Impervious Runoff Depth>2.18"

Tc=5.0 min CN=88 Runoff=0.18 cfs 539 cf

Subcatchment PD-2: Proposed Site Runoff Area=708 sf 0.00% Impervious Runoff Depth>1.56"

Tc=5.0 min CN=80 Runoff=0.03 cfs 92 cf

Subcatchment PD-3: Proposed Site Runoff Area=3,071 sf 100.00% Impervious Runoff Depth>3.16"

Tc=5.0 min CN=98 Runoff=0.24 cfs 810 cf

Pond 7P: Cultec 330 Peak Elev=30.03' Storage=790 cf Inflow=0.24 cfs 810 cf

Outflow=0.00 cfs 20 cf

Link 2L: POI B Inflow=0.03 cfs 112 cf

Primary=0.03 cfs 112 cf

Link 17L: POI A Inflow=0.18 cfs 539 cf

Primary=0.18 cfs 539 cf

Total Runoff Area = 6,751 sf Runoff Volume = 1,441 cf Average Runoff Depth = 2.56" 35.94% Pervious = 2,426 sf 64.06% Impervious = 4,325 sf

Type III 24-hr 5-Year Rainfall=4.30"

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PD-1: Proposed Site Runoff Area=2,972 sf 42.19% Impervious Runoff Depth>3.01"

Tc=5.0 min CN=88 Runoff=0.25 cfs 745 cf

Subcatchment PD-2: Proposed Site Runoff Area=708 sf 0.00% Impervious Runoff Depth>2.29"

Tc=5.0 min CN=80 Runoff=0.05 cfs 135 cf

Subcatchment PD-3: Proposed Site Runoff Area=3,071 sf 100.00% Impervious Runoff Depth>4.06"

Tc=5.0 min CN=98 Runoff=0.31 cfs 1,039 cf

Pond 7P: Cultec 330 Peak Elev=30.08' Storage=808 cf Inflow=0.31 cfs 1,039 cf

Outflow=0.01 cfs 249 cf

Link 2L: POI B Inflow=0.05 cfs 384 cf

Primary=0.05 cfs 384 cf

Link 17L: POI A Inflow=0.25 cfs 745 cf

Primary=0.25 cfs 745 cf

Total Runoff Area = 6,751 sf Runoff Volume = 1,919 cf Average Runoff Depth = 3.41" 35.94% Pervious = 2,426 sf 64.06% Impervious = 4,325 sf

Type III 24-hr 10-Year Rainfall=5.10"

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PD-1: Proposed Site Runoff Area=2,972 sf 42.19% Impervious Runoff Depth>3.76"

Tc=5.0 min CN=88 Runoff=0.30 cfs 931 cf

Subcatchment PD-2: Proposed Site Runoff Area=708 sf 0.00% Impervious Runoff Depth>2.98"

Tc=5.0 min CN=80 Runoff=0.06 cfs 176 cf

Subcatchment PD-3: Proposed Site Runoff Area=3,071 sf 100.00% Impervious Runoff Depth>4.86"

Tc=5.0 min CN=98 Runoff=0.36 cfs 1,244 cf

Pond 7P: Cultec 330 Peak Elev=30.16' Storage=833 cf Inflow=0.36 cfs 1,244 cf

Outflow=0.04 cfs 452 cf

Link 2L: POI B Inflow=0.06 cfs 628 cf

Primary=0.06 cfs 628 cf

Link 17L: POI A Inflow=0.30 cfs 931 cf

Primary=0.30 cfs 931 cf

Total Runoff Area = 6,751 sf Runoff Volume = 2,351 cf Average Runoff Depth = 4.18" 35.94% Pervious = 2,426 sf 64.06% Impervious = 4,325 sf

Type III 24-hr 50-Year Rainfall=7.60"

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PD-1: Proposed Site Runoff Area=2,972 sf 42.19% Impervious Runoff Depth>6.17"

Tc=5.0 min CN=88 Runoff=0.49 cfs 1,529 cf

Subcatchment PD-2: Proposed Site Runoff Area=708 sf 0.00% Impervious Runoff Depth>5.25"

Tc=5.0 min CN=80 Runoff=0.10 cfs 310 cf

Subcatchment PD-3: Proposed Site Runoff Area=3,071 sf 100.00% Impervious Runoff Depth>7.36"

Tc=5.0 min CN=98 Runoff=0.54 cfs 1,882 cf

Peak Elev=30.84' Storage=1,014 cf Inflow=0.54 cfs 1,882 cf

Outflow=0.20 cfs 1,088 cf

Link 2L: POI B Inflow=0.25 cfs 1,397 cf

Primary=0.25 cfs 1,397 cf

**Link 17L: POI A** Inflow=0.49 cfs 1,529 cf

Primary=0.49 cfs 1,529 cf

Total Runoff Area = 6,751 sf Runoff Volume = 3,721 cf Average Runoff Depth = 6.61" 35.94% Pervious = 2,426 sf 64.06% Impervious = 4,325 sf

Type III 24-hr 100-Year Rainfall=9.10"

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PD-1: Proposed Site Runoff Area=2,972 sf 42.19% Impervious Runoff Depth>7.64"

Tc=5.0 min CN=88 Runoff=0.60 cfs 1,892 cf

Subcatchment PD-2: Proposed Site Runoff Area=708 sf 0.00% Impervious Runoff Depth>6.66"

Tc=5.0 min CN=80 Runoff=0.13 cfs 393 cf

Subcatchment PD-3: Proposed Site Runoff Area=3,071 sf 100.00% Impervious Runoff Depth>8.85"

Tc=5.0 min CN=98 Runoff=0.65 cfs 2,266 cf

Peak Elev=31.13' Storage=1,075 cf Inflow=0.65 cfs 2,266 cf

Outflow=0.49 cfs 1,469 cf

Link 2L: POI B Inflow=0.59 cfs 1,862 cf

Primary=0.59 cfs 1,862 cf

Link 17L: POI A Inflow=0.60 cfs 1,892 cf

Primary=0.60 cfs 1,892 cf

Total Runoff Area = 6,751 sf Runoff Volume = 4,551 cf Average Runoff Depth = 8.09" 35.94% Pervious = 2,426 sf 64.06% Impervious = 4,325 sf

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## **Ground Covers (selected nodes)**

HSC	S-A H	SG-B	HSG-C	HSG-D	Other	Total	Ground
(sq	-ft) (	sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	Cover
	0	0	0	2,426	0	2,426	>75% Grass
							cover, Good
	0	0	0	0	1,184	1,184	Existing Dwelling
	0	0	0	0	70	70	Existing
							Walkways
	0	0	0	0	607	607	Proposed
							Driveway
	0	0	0	0	2,397	2,397	Proposed Partial
							Building
	0	0	0	0	67	67	Proposed
							walkway
	0	0	0	2,426	4,325	6,751	TOTAL AREA

Type III 24-hr 25-Year Rainfall=6.40"

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PD-1: Proposed Site Runoff Area=2,972 sf 42.19% Impervious Runoff Depth>5.01"

Tc=5.0 min CN=88 Runoff=0.40 cfs 1,240 cf

Subcatchment PD-2: Proposed Site Runoff Area=708 sf 0.00% Impervious Runoff Depth>4.14"

Tc=5.0 min CN=80 Runoff=0.08 cfs 244 cf

Subcatchment PD-3: Proposed Site Runoff Area=3,071 sf 100.00% Impervious Runoff Depth>6.16"

Tc=5.0 min CN=98 Runoff=0.46 cfs 1,576 cf

Pond 7P: Cultec 330 Peak Elev=30.43' Storage=918 cf Inflow=0.46 cfs 1,576 cf

Outflow=0.13 cfs 782 cf

Link 2L: POI B Inflow=0.16 cfs 1,027 cf

Primary=0.16 cfs 1,027 cf

Link 17L: POI A Inflow=0.40 cfs 1,240 cf

Primary=0.40 cfs 1,240 cf

Total Runoff Area = 6,751 sf Runoff Volume = 3,060 cf Average Runoff Depth = 5.44" 35.94% Pervious = 2,426 sf 64.06% Impervious = 4,325 sf

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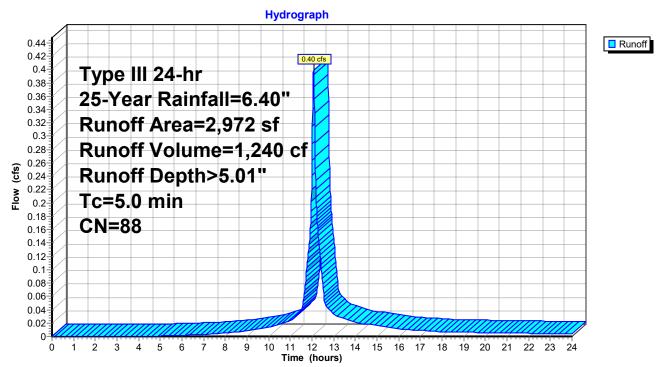
## **Summary for Subcatchment PD-1: Proposed Site**

Runoff = 0.40 cfs @ 12.07 hrs, Volume= 1,240 cf, Depth> 5.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=6.40"

	Α	rea (sf)	CN	Description					
*		1,184	98	Existing Dw	elling				
*		70	98	Existing Wa	alkways				
		1,718	80	>75% Gras	s cover, Go	ood, HSG D			
		2,972 1,718		Weighted Average 57.81% Pervious Area 42.19% Impervious Area					
		1,254		42.19% 1111	bervious Ar	еа			
(	Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description			
	5.0					Direct Entry,			

## **Subcatchment PD-1: Proposed Site**



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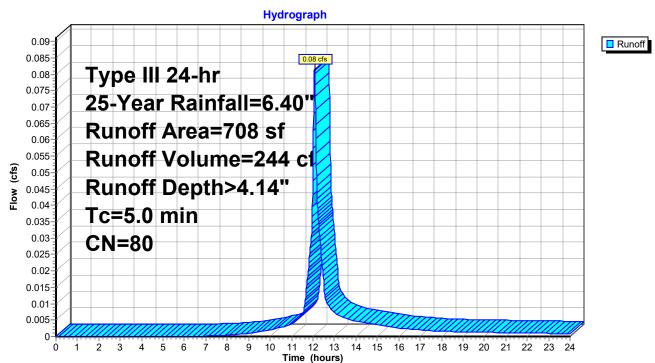
## **Summary for Subcatchment PD-2: Proposed Site**

Runoff = 0.08 cfs @ 12.07 hrs, Volume= 244 cf, Depth> 4.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=6.40"

A	rea (sf)	CN E	escription				
	708	80 >	>75% Grass cover, Good, HSG D				
•	708	1	100.00% Pervious Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
5.0					Direct Entry,		

## **Subcatchment PD-2: Proposed Site**



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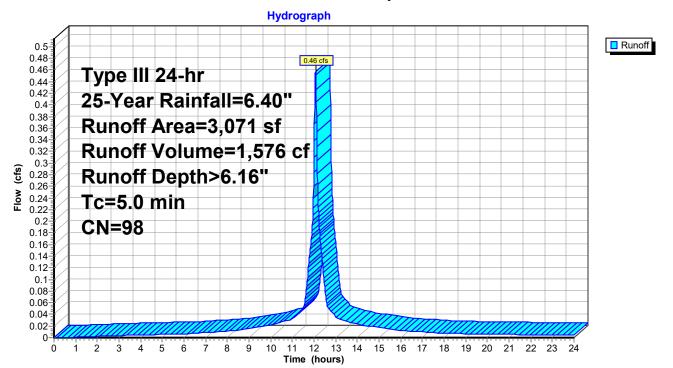
## **Summary for Subcatchment PD-3: Proposed Site**

Runoff = 0.46 cfs @ 12.07 hrs, Volume= 1,576 cf, Depth> 6.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=6.40"

	Α	rea (sf)	CN	Description					
*		2,397	98	Proposed P	artial Build	ling			
*		607	98	Proposed D	riveway				
*		67	98	Proposed w	Proposed walkway				
		3,071 3,071	98	Weighted A 100.00% Im	•	Area			
	Tc (min)	Length (feet)	Slop (ft/f	,	Capacity (cfs)	Description			
	5.0					Direct Entry,			

#### **Subcatchment PD-3: Proposed Site**



Type III 24-hr 25-Year Rainfall=6.40"

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## **Summary for Pond 7P: Cultec 330**

3,071 sf,100.00% Impervious, Inflow Depth > 6.16" for 25-Year event Inflow Area =

0.46 cfs @ 12.07 hrs, Volume= Inflow 1,576 cf

782 cf, Atten= 71%, Lag= 18.9 min Outflow = 0.13 cfs @ 12.39 hrs, Volume=

Primary 0.13 cfs @ 12.39 hrs, Volume= 782 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 30.43' @ 12.39 hrs Surf.Area= 508 sf Storage= 918 cf

Plug-Flow detention time= 279.3 min calculated for 782 cf (50% of inflow)

Center-of-Mass det. time= 147.0 min ( 889.9 - 742.8 )

Volume	Invert	Avail.Storage	Storage Description
#1A	27.76'	460 cf	11.17'W x 45.50'L x 3.54'H Field A
			1,799 cf Overall - 648 cf Embedded = 1,151 cf x 40.0% Voids
#2A	28.26'	648 cf	Cultec R-330XLHD x 12 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 2 rows
· · · · · · · · · · · · · · · · · · ·	•	4 400 5	T ( ) A ( ) ) ) O (

1,109 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	30.00'	<b>6.0" Vert. 6" Outlet</b> C= 0.600
#2	Device 1	30.00'	<b>3.0" Vert. 3" Orifice</b> C= 0.600
#3	Device 1	31.00'	<b>6.0" Horiz. 6" Overflow</b> C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.13 cfs @ 12.39 hrs HW=30.43' (Free Discharge)

-1=6" Outlet (Passes 0.13 cfs of 0.40 cfs potential flow)

**2=3" Orifice** (Orifice Controls 0.13 cfs @ 2.66 fps)

-3=6" Overflow (Controls 0.00 cfs)

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#### Pond 7P: Cultec 330 - Chamber Wizard Field A

#### Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 2 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

6 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 43.50' Row Length +12.0" End Stone x 2 = 45.50' Base Length

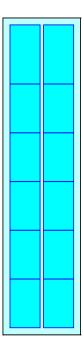
2 Rows x 52.0" Wide + 6.0" Spacing x 1 + 12.0" Side Stone x 2 = 11.17' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

12 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 2 Rows = 648.2 cf Chamber Storage

1,799.5 cf Field - 648.2 cf Chambers = 1,151.2 cf Stone x 40.0% Voids = 460.5 cf Stone Storage

Chamber Storage + Stone Storage = 1,108.7 cf = 0.025 af Overall Storage Efficiency = 61.6% Overall System Size = 45.50' x 11.17' x 3.54'

12 Chambers 66.6 cy Field 42.6 cy Stone



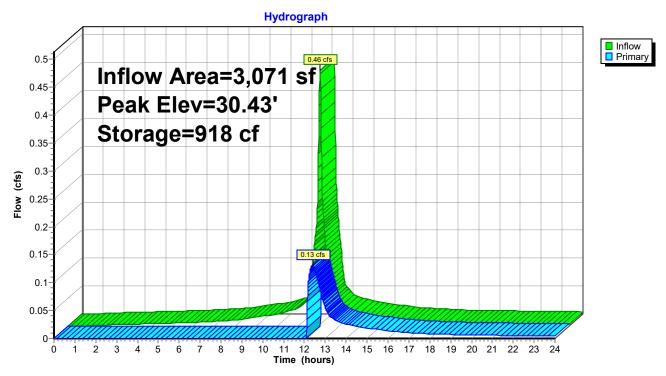


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Pond 7P: Cultec 330



Type III 24-hr 25-Year Rainfall=6.40"

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## **Summary for Link 2L: POI B**

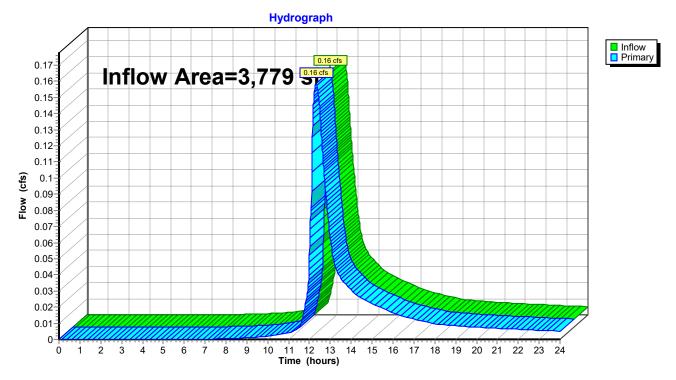
Inflow Area = 3,779 sf, 81.26% Impervious, Inflow Depth > 3.26" for 25-Year event

Inflow = 0.16 cfs @ 12.32 hrs, Volume= 1,027 cf

Primary = 0.16 cfs @ 12.32 hrs, Volume= 1,027 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

#### Link 2L: POI B



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## **Summary for Link 17L: POI A**

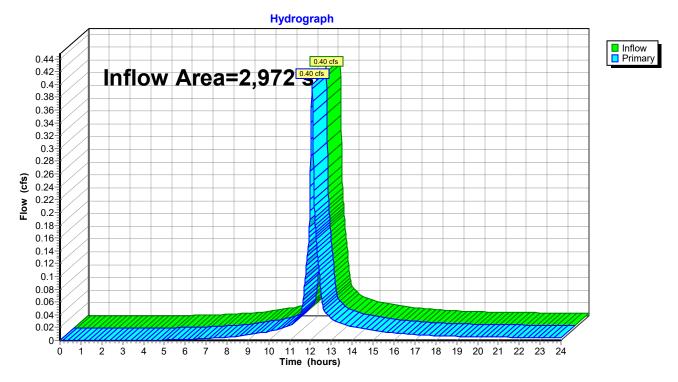
Inflow Area = 2,972 sf, 42.19% Impervious, Inflow Depth > 5.01" for 25-Year event

Inflow = 0.40 cfs @ 12.07 hrs, Volume= 1,240 cf

Primary = 0.40 cfs @ 12.07 hrs, Volume= 1,240 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

#### Link 17L: POI A





# Appendix D DCIA Tracking Worksheet

### **Directly Connected Impervious Area Tracking Worksheet City of Stamford Drainage Manual**



### Note to user: complete all cells of this color only, as indicated by section headings

Double Comment Information (All Dunionts)						
	Part 1: General Information (All Projects)		_			
Project Name			_			
Project Address						
Project Applicant			_			
Title of Plan			_			
Revision Date of Plan Tax Account Number			_			
Tax Account Number			_			
	Part 2: Project Details (All Projects)					
1. What type of develop	ment is this? (choose from dropdown)					
2. What is the total area	of the project site?	f	t <sup>2</sup>			
3. What is the total area	of land disturbance for this project?	f	t <sup>2</sup>			
4. Does project site drain to High Quality Waters, a Direct Waterfront, or						
within 500 ft. of Tidal Wetlands? (Yes/No)						
Does Standard 1 apply based on information above?						
	Part 3: Water Quality Target Total (Only for Standard	1 Projects)	٦			
5. What is the current (g	ore-development) DCIA for the site?	f	t <sup>2</sup>			
	velopment increase <b>DCIA</b> (without consideration of					
proposed stormwater m	·					
7. What is the proposed	-development total impervious area for the site?	f	t <sup>2</sup>			
Water Quality Volume (	WQV)	f	$t^3$			
Standard 1 requirement						
Required treatment/ret	ention volume	fi	t <sup>3</sup>			
•	ention volume for proposed development		t <sup>3</sup>			
Part 4: Proposed DCIA Tracking (Only for Standard 1 Projects)						
Donald also and total			2			
Pre-development total	impervious area		t <sup>2</sup>			
Current DCIA	total transaction and		t <sup>2</sup>			
Proposed-development		t <sup>2</sup>				
Proposed-development		t <sup>2</sup>				
Net change in <b>DCIA</b> from	m <u>current</u> to <u>proposed-development</u>	fi	t <sup>2</sup>			
Part 5: Post-D	Development (As-Built Certified) DCIA Tracking (Only	for Standard 1 Projects)	٦			
	as-built) <b>total impervious area</b>		t <sup>2</sup>			
Post-development (per						
			t <sup>2</sup>			
onange in <b>Den</b> i itol	Net change in <b>DCIA</b> from <u>current</u> to <u>post-development</u> ft <sup>2</sup>					
Certification Statement  I hereby certify that the information contained in this worksheet is true and correct.						

Engineer's Signature

Date

Keith Werner

Engineer's Seal



### **Appendix E Drawdown Calculations**

### 1911 Summer St, Stamford, CT

### **Drawdown Calculations**

Drawdown Cultec System C-1							
		DV	_				
Time <sub>drawdown</sub>	=	(K) (A)	=	6.1	hours	OK < 72 hours	
DV	=	Design Volume	=		cf	= stored volume water below	
				779		outlet pipe, see HydroCAD calculations	
K <sub>Soil Goup C</sub>	=	Infiltration Rate	=	3	inches/hour	Soil Group B (6"/2(safety factor))	
Α	=	Bottom Area	=	508	ft <sup>2</sup>		

# 

## STATISTICS:

EXISTING HISTORIC HOUSE — RESIDENTIAL DWELLING UNIT 5-2 BRM GROSS AREA PRIOR TO DEMOLITION OF NON—HISTORIC REAR ENTRY ADDITION: 3630 S.F.

TOTAL GROSS LIVING AREA WITHOUT DEMOLISHED PORTION: 1930 S.F. BASEMENT AND ATTIC ARE NOT INCLUDED IN AREA ABOVE BASEMENT GROSS AREA IS: 998 S.F. ATTIC GROSS AREA IS 572 S.F.

TOTAL GROSS AREA REMAINING AFTER DEMOLITION OF REAR INCLUDING BASEMENT AND ATTIC AREAS: ADDITION
3500 S.F.

CONSTRICTION: DWELLING UNITS

EACH DWELLING UNIT LIVING AREAS FIRST FLOOR (GARAGE NOT INCLUDED):	1	GROSS 81 S.F.
NOT INCLUDED	···	

LUDED): 81 S.F.  568 S.F.  405 S.F.  1622 S.F.  322 S.F.  1944 S.F.  U.'S  7776 S.F.  3500 S.F.		EXISTING BUILDING GROSS AREA:	TOTAL NEW CONSTRUCTION 4 D.U.'S GROSS AREA INCLUDING GARAGES:	TOTAL GROSS AREA PER UNIT	GARAGE AREA:	TOTAL LIVING AREA PER UNIT	FOURTH FLOOR:	THIRD FLOOR:	SECOND FLOOR:	FIRST FLOOR (GARAGE NOT INCLUDED):	
17 22 14	į	ARFA:	4 D.U.'S RAGES:	Z		JUIT				T INCLUDED):	
67 S.F. 517 S.F. 517 S.F. 366 S.F. 1467 S.F. 285 S.F. 1752 S.F.		3500 S.F.	7776 S.F.	1944 S.F.	322 S.F.	1622 S.F.	405 S.F.	568 S.F.	568 S.F.	81 S.F.	
				1752 S.F.	285 S.F.	1467 S.F.	366 S.F.	517 S.F.	517 S.F.	67 S.F.	

## APPLICABLE CODES:

Adopted and Referenced Publications

- 1 International Existing Building Code
  1 International Plumbing Code
  1 International Mechanical Code
  1 International Energy Conservation Code
  0 NFPA 70, National Electrical Code, of the
  National Fire Protection Association Inc.
  1 International Residential Code

2021

### HANGES FROM PREVIOUS DATED

- HIP ROOF CONFIGURATION OF SIMILAR TO THE ROOF OF THE HOUSE. THE ADDITION IS NOW EXISTING HISTORIC

 $\overset{\textstyle \triangleright}{\cdot}$ 

- 9.00  $\mathcal{C}$
- THE BUILDING HEIGHT IS NOW 37'-10" TO THE MIDPOINT OF THE ROOF, VERSUS 45' TO THE TOP OF THE STAIRS' ROOFS IN THE 2/10/23 DESIGN.

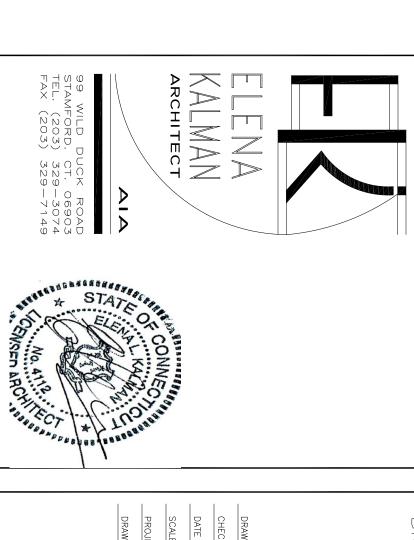
  BUILDING FACADE IS NOW THREE STORY HIGH WITH HI ROOF AND DORMERS AT THE FOURTH FLOOR. THIS IS ONE STORY LESS THAN ON THE PREVIOUS DESIGN.

  ROOF ACCESS IS ELIMINATED.

  BAYS ARE REDUCED.
  COLOR OF THE CENTRAL PROJECTING PORTION OF THE BUILDING IS NOW THE SAME AS THE EXISTING BUILDING. USING TWO COLOR SCHEME VISUALLY BREAKS THE MASS OF THE BUILDING.

  DRIVEWAY UNDER THE BUILDING BETWEEN THE GARAGE DOORS IS NOW 28' WIDE VERSUS 20' WIDE OF THE PREVIOUS DESIGN.
- $\sim$ GARAGE

SIDING NOTE:
SIDING ON THE ADDITION: JAMES HARDIE PLANK,
SMOOTH FINISH COLOR: DEEP OCEAN AND
BOOTHBAY BLUE.
SHINGLES ON THE EXISTING HOUSE PAINTED:
TO MATCH BOOTHBAY BLUE.
TRIM AND SOFFITS ON BOTH HOUSES: WHITE



AND NOTES	COVER, CODE	

1	>	>



