



Fee Schedule

Phone: 203,977,4719 • Fax: 203.9

APPLICATION FOR SPECIAL PERMIT

Complete, notorize, and forward thirteen (13) hard copies and (1) electronic copy in PDF format to Clerk of the Zoning Board with a \$1,000.00 Public Hearing Fee and the required application filling fee (see Fee Schedule below), payable to the City of Stamford.

NOTE: Cost of required advertisements are payable by the Applicant and performance of required mailing to surrounding property owners is the sole responsibility of the applicant. LAND RECORDS RECORDING FEE: \$60.00 for First page - \$5.00 for each additional page)

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City of Stamfard Zoning Board - Land Use Bureau Government Center - 888 Washington Boulevard - Stamford, CT 06904-2152 Phone: 203.977.4719 - Fax: 203.977.4100

DATED AT STAMFORD, CO	•	00	Decembe	20_23	
NOTE: Application cannot Stamford Planning Board. prior to Public Hearing so t	If applicant wishes to	blic Hearing until 35 day	please notify the Zo	oning Board at least three	
STATE OF CONNECTICUT COUNTY OF FAIRFIELD	es STAMFORD	December	13	20 23	
Personally appeared JOS the truth of the contents there	eph J. Capall of, before me.	bo, II	signer of the fo	regoing application, who ma	ade oath to
			ery Public Commis	sioner of the Superior Cour	-
FOR OFFICE USE ONLY					13-22-10-11-
APPL #:	Receiv	ved in the office of the Zor	ning Board: Date: _		
			Ву:		<u></u>

Revised 09/02/2020

AUTHORIZATION

December 13, 2023

Zoning Board
City of Stamford
888 Washington Boulevard
Stamford, CT 06901
ATTENTION: David Stein, Chairman

AUTHORIZATION

RE: APPLICATION OF BUDR CANNABIS FOR A SPECIAL PERMIT TO OPERATE A MARIJUANA DISPENSARY FACILITY AT 389 WEST MAIN STREET, STAMFORD, CONNECTICUT

Dear Mr. Stein;

This shall confirm that the undersigned, 389 West Main St Realty LLC, is the current owner of the premises commonly known as 389 West Main Street, Stamford, Connecticut (the "Premises"). This shall also serve to confirm that authorization has been granted to BUDR Cannabis, and its duly authorized representative, Joseph J. Capalbo II, Attorney At Law, to file with and apply to the Zoning Board of the City of Stamford for a Special Permit to operate a Marijuana Dispensary Facility at the referenced Premises.

389 West Main St Realty LLC:

Donald BAEZ

BY:

Its

Duly Authorized

PROJECT DESCRIPTION



PROJECT DESCRIPTION

About Budr

Budr Cannabis envisions a healthy and equitable workforce, enabling all people to participate and reach their full potential. Further, we believe in the power of community and recognize the importance of social equity in the Connecticut cannabis marketplace.

Budr is currently 50% minority owned, and will endeavor to hire at least 50% of its employees who are of non-caucasian ethnicities.

Budr has also developed, and will deploy, the Social Equity for enhancing equity within the organization through a number of various outreach efforts, and training programs as well as provide employment and training opportunities for individuals in disproportionately impacted areas.

Building Security Plan

Facilities

All doors, gates, safes, vaults, cabinets, and other secured locations will be always locked.

Perimeter Security:

- 1. The premises will be monitored by security personnel
- Only management level and other authorized staff will have keys or access codes/cards to open the perimeter fence
- 3. A security alarm/vendor, including a motion detection system within the perimeter, will notify management of an attempt to breach the perimeter in an unauthorized manner
- 4. Lighting will illuminate all areas of the perimeter during store hours
- 5. A back-up battery alarm system will be in place in case of a power outage to the system
- A security company will be employed regularly to patrol the location during store hours and if there is any failure of alarm system



Premises Ingress and Egress

- 1. Only management and other authorized staff will have keys or access codes to open the building turn off security alarms, and open vault
- 2. There will only be one entrance door to the premises for visitors
- 3. There will be a mantrap to ensure no one under 21 enters the sales floor of the facility
- 4. Employees are expected to enter thru front left door or entrance door as necessary
- 5. The front door will be always staffed when visitors are expected
- 6. The whole interior and exterior of the building will be covered by security cameras in compliance with state regulations
- 7. Front left door will have swipe access such that only authorized personnel can gain access to the building
- 8. Door on roadside will act as an exit for visitors to the building, controlling traffic within the facility
- 9. When the side left door is used to bring in supplies, inventory, or remove waste, a company security officer will be present while the door is open
- 10. All doors will be alarmed, covered by security cameras, and well lit

Facility Access Procedures

Key Issuance Procedure

- 1. An authorized senior staff member will record the issuance of keys or access cards to a new employee into a security access log
- The security access log will contain the name of the person being issued the item, the employee's company assigned identification number, the employee's position/title, the level of access being assigned, date, signature of employee, and the initial of the issuer
- 3. Extra keys/access cards will be in a locked safe, which only designated senior staff members will have access to
- 4. All employees issued an access card / code, will sign a Card User Agreement acknowledging they understand their related procedures and will not share their card or code with anyone else

Access Codes

1. For locations controlled by means of entering an access code into a keypad, each individual will have a separate access code which is used by a third-party security provider, both to track who



is accessing the controlled area, and to allow a single code to be terminated rather than rekeying the entire system after an employee's last day

2. The issuance of an access code will be recorded into the security access log, including a list or designation which indicated the locations the employee is able to access

Key Return and Termination Procedures

1. Upon learning of an employee's final employment date, an authorized senior staff member will notify all staff involved in maintaining the security system, including the computing security officer

Roles and Responsibilities

Chief Compliance Officer

The CEO will designate a Chief Compliance Officer / Store Manager, who will be responsible for implementing the plan by hiring a qualified Security Manager / Third Party vendor who will be responsible for a subset of tasks. The CCO / Store Manager will ensure that all security procedures are followed related to facility access, emergency response, transportation, training, hiring and supervising security officers, background checks, prohibiting the access of minors on the premises, and other procedures required for state compliance. The CCO / Store Manager is responsible for clearly for who will serve as the senior security officer in an emergency if both the CEO and CCO / Dispensary Manager are not present. The CCO / Store Manager will regularly report to the CEO the status and efficacy of the Security Plan. The CCO / Store Manager will review the plan annually and recommend changes or amendments to the CEO to improve security features or processes. The CCO / Dispensary Manager will be the primary contact for all security equipment vendors.

Security Manager

The Security Manager will be responsible for ensuring that the Security Plan is properly implemented. Responsibilities:

- Act as a liaison to law enforcement if needed
- Schedule all security services



- Ensure all security equipment and systems are operated and maintained according to manuals,
 standard security practices, security plan, and the Department of Consumer Protection guidelines
- Administer the access control program
- Compile reports as needed
- Utilize all security systems to discover security breaches and identify compliance issues
- Ensure the protection of visitors and staff
- Manager all visitor access to the facility

Inventory

All inventory stored on the licensed premises will be secured in the vault. Budr will store all cannabis products in a regulation vault overnight mandated by the Department of Consumer Protection.

Offices, and bathrooms will be separate from all storage areas containing cash or cannabis materials. In the event of a disaster, a licensee or designated employee will be assigned to move cash and cannabis products to a new location for a short period of time to prevent loss, theft, or degradation of cannabis products from the disaster.

The Security Manager is responsible for protecting company products during delivery and once they are stored. The Security Manager will ensure good working order of mechanical systems associated with monitoring products, such as lights and cameras, and will track employee and Manager access to sensitive materials and stored company valuables. Steps for this will include:

- Ensure that no valuable or sensitive materials are visible to employees / customers
- Implement policies to ensure that workers cannot easily remove cash, cannabis material, or other valuables in their clothing or personal containers and will implement clear rules around pockets, containers, and bags to avert employee diversion and theft
- Monitor access of appropriate personnel and visitors to limited access areas
- Maintain a video monitor of cameras showing both the outside and inside of the facility
- Maintain logs of removal of sensitive and valuable products from their production or storage areas
- Ensure that all materials have been logged in properly to the Point-of-Sale system.
- Ensure weekly inventory is conducted and any variances explained



Odor Remediation

Unlike Cannabis Cultivation facilities, Cannabis Retailers receive all products in pre-packaged and sealed containers, mylar bags, etc., which is a natural preventative to odor smell both on the interior and exterior of the facility. Furthermore, product cannot be opened on site when purchased as it is illegal. As an added precaution, we will purchase Air Purifiers and have them stationed in each designated Cannabis Storage or working area.

Property Management Program

	Opening Checkli	st				
Task	M	T	W	Th	F	s
Disarm Alarm Code						
Turn Lights On		•				
Ensure all waste disposed	•			:		
Log into Security System and view all areas		•				
Clean POS Counters		1		:		
Ensure air purifiers turned on						
Walk Premises remove any overnight garbage						
Log into all POS and Ecommerce systems	•	•				
Unlock vault for daily access	•				;	

Closin	g Checklis	t				
Task	M	Т	W	Th	F	s
Run Daily POS Reports						
Lock all safes and vaults				•		
Remove all garbage from facility and bring to dumpster	:					
Walk Parking Lot and Sidewalks and remove litter				:		
Clean POS Counters						
Ensure air purifiers turned off						
Log off all technology systems						
Ensure all access doors fully shot						
Turn off all lights						
Turn on alarm when leaving for the night						



Workplace Safety

Workplace safety plans will adhere to federal Occupational Safety and Health Administration (OSHA) regulations. Staff will receive ongoing training on safety protocols and practices.

Budr is committed to creating a secure environment that fully complies with Connecticut's cannabis regulations. Regular reviews and updates will be conducted to adapt to evolving security concerns and regulatory changes. The plan will serve as a framework to ensure the safety, compliance, and security of the operations.

STATEMENT OF FINDINGS

STATEMENT OF FINDINGS

1. The location and nature of the proposed site including its size and configuration, the proposed size, scale and arrangement of structures, drives and parking areas and the proximity of existing dwellings and other structures.

The applicant, BUDR Cannabis is proposing to operate an adult-use Retailer in the existing building at 389 West Main Street. The facility is proposed to be the sole occupant in the existing building which totals approximately 2448 square feet is size. Any and all improvements will consist only of fit out within the existing structure and added security. The existing building footprint will not be changed. The drives and parking areas will remain as is and are more than adequate to service the proposed use. The premises is zoned C-L, Limited Business District, which permits the proposed use by Special permit. The proposed location fronts on West Main Street which is characterized by significant commercial development.

2. The nature and intensity of the proposed use in relation to its site and the surrounding area. Operations in connection with Special Permit uses shall not be injurious to the neighborhood, shall be in harmony with the general purpose and intent of these Regulations, and shall not be more objectionable to nearby properties by reason of noise, fumes, vibration, artificial lighting or other potential disturbances to the health, safety or peaceful enjoyment of property than the public safety demands.

The property is located in the C-L zone. As its zoning designation would indicate, the immediate area along West Main Street is characterized by a variety of commercial uses including a bank, restaurants, car wash, various service establishments (Triple S Carpet, Franklin Glass and others), a self-storage facility, gas stations, fast food establishments as well as others. The premises is a corner lot with frontage on Liberty Street as well. Direct across the street is a restaurant. The properties immediately adjacent on Liberty Street are also zone C-L. The majority of Liberty Street, west of the subject premises, which runs through to Stillwater Avenue is zoned R-5 and is characterized by multi-family housing and legally non-conforming commercial establishments. The nature of the proposed use, being so highly regulated and secure, and in light of the surrounding neighborhood, poses no risk the to the health, safety, welfare and peaceful enjoyment of the area.

3. The resulting traffic patterns, the adequacy of existing streets to accommodate the traffic associated with the proposed use, the adequacy of proposed off street parking and loading, and the extent to which proposed driveways may cause a safety hazard, or traffic nuisance.

The existing site provides for twenty (20) parking spaces consisting of 18 standard spaces and two handicap spaces. All twenty spaces will service the proposed establishment. The proposed uses maintains the highest required number of parking spaces for a use at four (4) spaces per thousand square feet. The proposed use will occupy approximately 2500 square

feet, the entire building, which translates into the fact that there is more than enough parking to satisfy the parking demands for this project.

The site is located at the signalized intersection of West Main Street and Liberty Street. West Main Street is a state highway also referred to as Route 1. Access to and from the site is on West Main Street. A traffic assessment for the proposed use has been provided by Bubaris Traffic Associates which concludes that no adverse traffic consequences will result from the proposed establishment. A formal detailed traffic study shall be provided and submitted to the staff, land use boards and traffic department.

4. The nature of the surrounding area and the extent to which the proposed use or feature might impair its present and future development.

West Main Street is zoned and characterized almost exclusively by commercial uses. The proposed use, which will occupy an existing building location, is consistent and in character with its surroundings. In fact, the proposed use will be safer, quieter, and less conspicuous than many of the surrounding uses. Any future development within this vicinity will only be redevelopment as it can be argued that the immediate area is already developed to its capacity.

5. The Master Plan of the City of Stamford and all statements of the purpose and intent of these Regulations.

The proposed location is located in Master Plan Category 6, Commercial Neighborhood Business. The Master Plan states that this category is, "intended to provide for and promote pedestrian-scaled "Main Street" environments; encourage a variety of retail...distinct from most intensive downtown development." The district encourages density far below those permitted in the downtown and considers, among other factors for approval, shared parking, mixed use development and Main Street amenities, all of which are prevalent at the proposed location. The proposed use is modest in size and scope and extremely secure resulting in no impact to the surrounding neighborhood and it is consistent with the intention of its Master Plan and Zoning Designations.

ADDITIONAL REQUIREMENTS IN ACCORDANCE WITH SECTION 5 OF THE STAMFORD ZONING REGULATIONS.

 Marijuana Dispensaries must possess a current license from the State of Connecticut Department of Consumer Protection and comply with the Regulations of the State of Connecticut Department of Consumer Protection Concerning the Palliative Use of Marijuana, per the Connecticut General Statutes, Section 21a-408-1 to 21a-408-70, inclusive, as may be amended from time to time. BUDR Cannabis is a Connecticut entity currently fifty (50%) percent minority owned. BUDR has applied for, received and is in possession of a provisional adult-use retailer license from the State of Connecticut. Upon receipt and approval of a Special Permit from the City of Stamford, BUDR Cannabis will operate its facility at the proposed location in compliance with the Rules and Regulations of the State of Connecticut Department of Consumer Protection Concerning the Palliative Use of Marijuana in accordance with the Connecticut General Statutes as amended from time to time.

2. No Medical Marijuana Dispensaries shall be located within a 3000 foot radius of any other Dispensary.

The nearest site of a Dispensary/Retailer in relation to the proposed location is well in excess of the 3000 foot radius as required in the Stamford Zoning regulations. The nearest facility is located at 111 High Ridge Road, a significant distance from this proposed location.

3. Signage for the Dispensary must be in compliance with the seven (7) standards enumerated in the Stamford Zoning Regulations.

Any and all signage proposed for the retailer shall be in full compliance with the Zoning Regulations of the City of Stamford. All signage details shall be included in a signage plan for review.

4. Parking shall be provided according to Section 12 of the Stamford Zoning Regulations, as follows: A Dispensary shall meet the parking standards for a retail store.

The existing building for the proposed location is, according to the City's assessor's records, 2448 square feet. The proposed use requires the most intensive parking requirements at four (4) spaces per thousand. If you round the size of the exiting building up to 3000 square feet, 12 parking spaces would be required. The existing site has twenty (20) parking spaces, 18 conventional and standard sized spaces and two handicap spaces. The existing parking configuration is more than adequate to satisfy the existing parking requirements.

LEGAL DESCRIPTION

LEGAL DESCRIPTION

ALL THAT CERTAIN piece or parcel of land, together with the buildings and improvements thereon, situated in the City of Stamford, County of Fairfield and State of Connecticut, shown and delineated on a certain map entitled, "Map Prepared for County Federal Savings and Loan Association, Stamford, Connecticut", certified "substantially correct" by John R. O'Brien, L.S. No. 7228, Sept. 29, 1977, on file in the Office of the Stamford Town Clerk as Map No. 10059. Said premises are bounded:

NORTHERLY:

123.00 feet by land now or formerly of Dante Latte et al;

EASTERLY:

125.03 feet by land now or formerly of The State National Bank

of Connecticut;

SOUTHERLY:

123.02 feet by West Main Street, so-called; and

WESTERLY:

122.71 feet by Liberty Street, so-called.

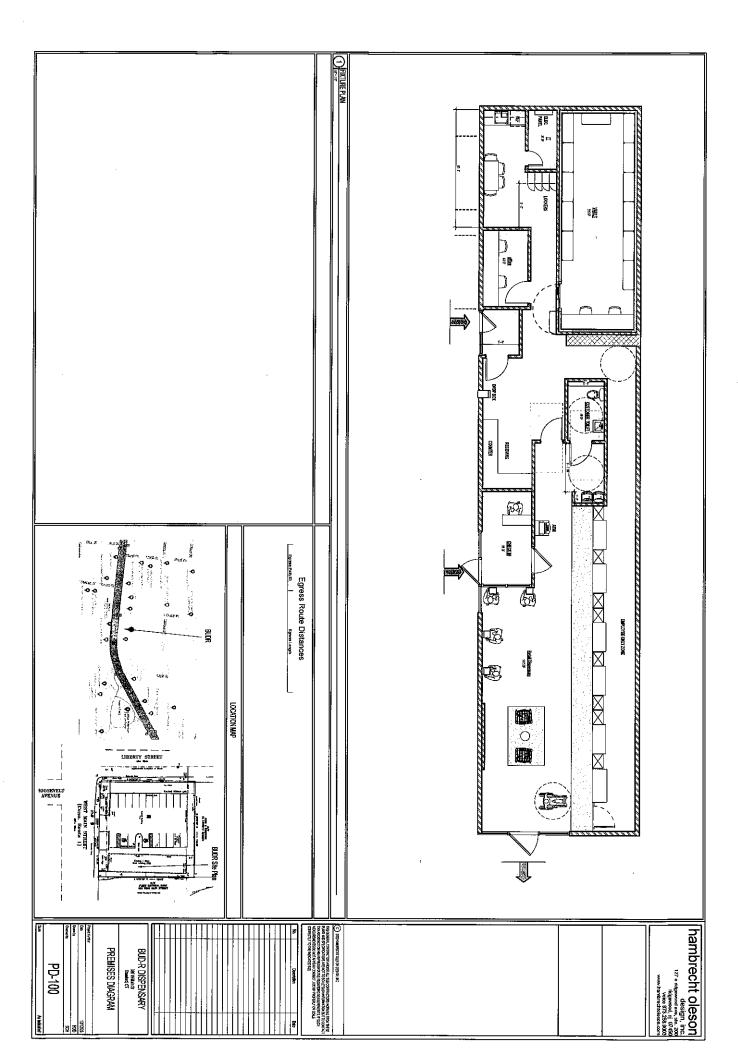
ZONING DATA CHART

ZONING DATA CHART

			-								
		MIN. LOT SIZE		BLDG. HI	EIGHT	BLDG. HEIGHT BLDG. AREA		MIN. YARD DIMENSIONS (ft)	ONS (ft)		
	AREA (sf)	FRONTAGE (ft)	FAR	STORIES	FEET	FAR STORIES FEET CORNER LOT %	STREET LINE	STREET CENTER ONE SIDE BOTH SIDES REAR	ONE SIDE	BOTH SIDES	REAR
										. ;	
C-I REOLIBED	A000	A0	٠		'n	7	<u>.</u>	,) -		
:										,	f
EXISTING	15236	122.71' - Liberty St. 0.162	0.162	⊬	12	15.90%	15' - Liberty St.	35' - Liberty St.	4.3.	13.1'	N/A
	į	123.02' - West Main St.					12.1' - West Main St. 37.1' - West Main St.	37.1' - West Main St.			

^{*} None required but if provided must be a minimum of 4 feet.
All Dimensions are existing.
No changes are being proposed to the footprint of the building.

FLOOR PLAN

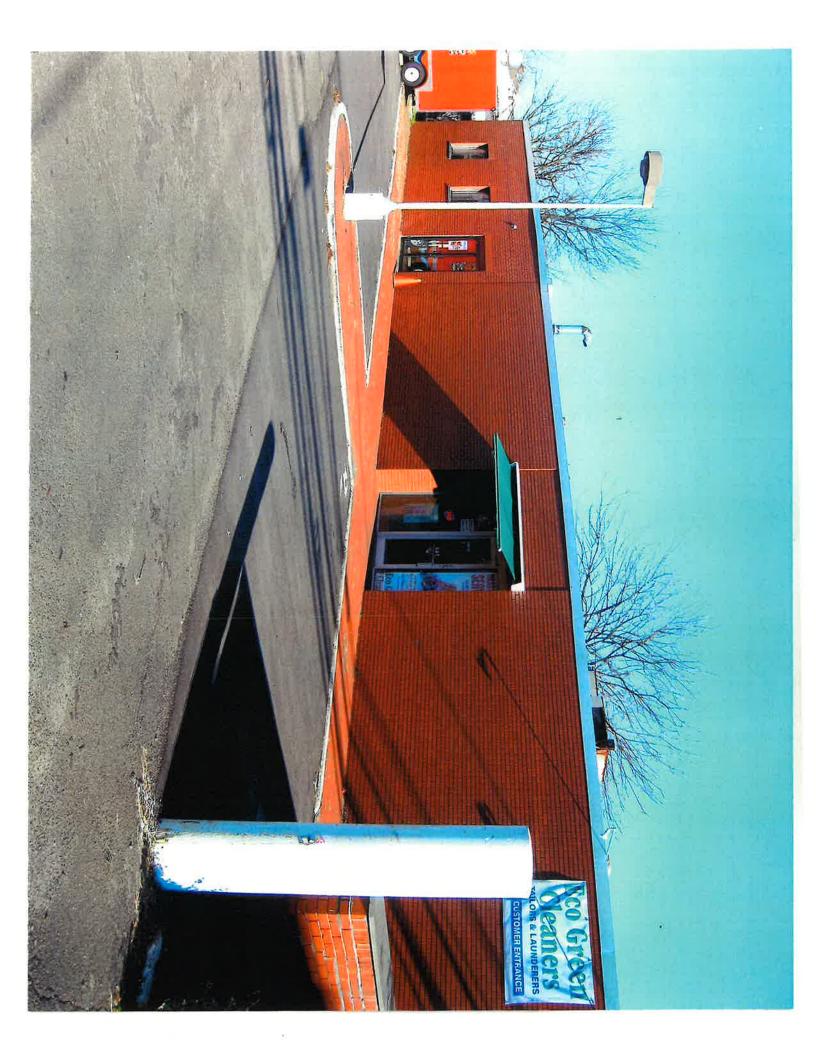


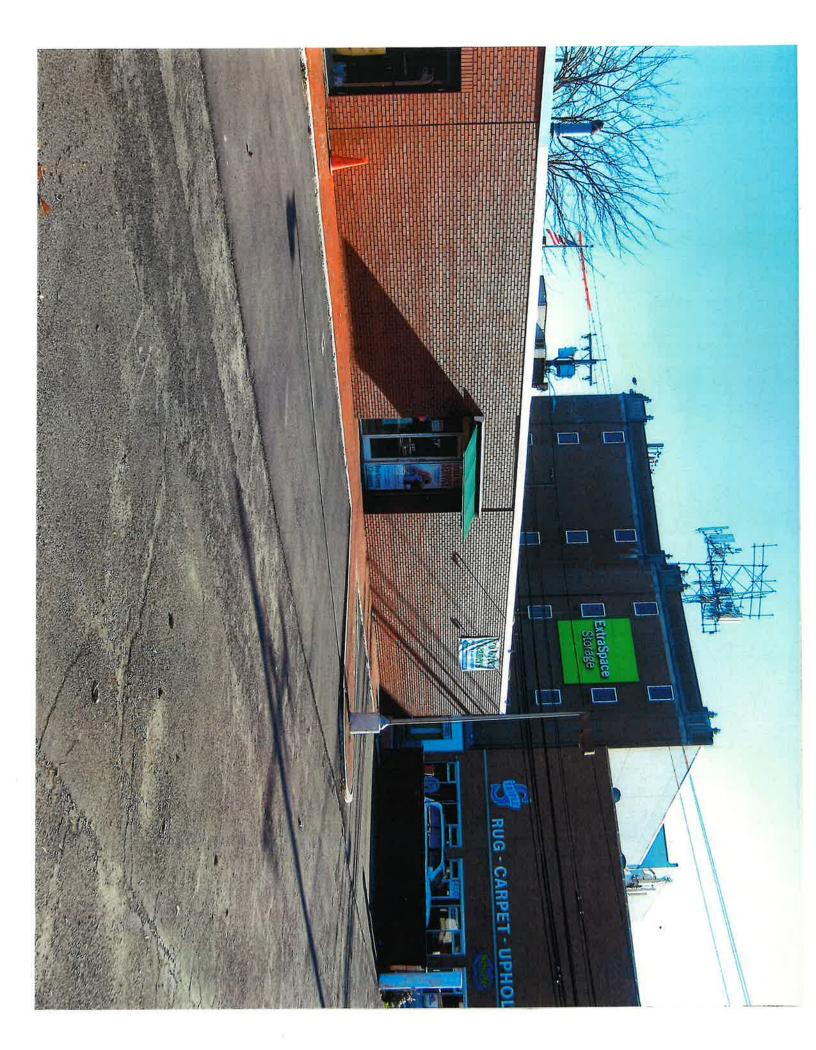
SITE PHOTOGRAPHS











TRAFFIC STUDY



December 12, 2023

Joseph J. Capalbo, III Attorney at Law 1100 Summer Street Stamford, CT 06905

Re: Site Traffic Assessment

Proposed Cannabis Dispensary 389 West Main Street (Route 1) Stamford, Connecticut

Dear Mr. Capalbo:

The site is located at the northeast quadrant of the signalized intersection of West Main Street (Route 1) at Liberty Street and Roosevelt Avenue, with one two-way site drive serving the site intersecting the north side of West Main Street directly east of Liberty Street.

Please refer to Exhibit 1 of the Appendix which locates this site with respect to the surrounding roadway network.

Traffic Conditions

West Main Street is a state highway (Route 1) running east-west in the vicinity of the site, with one eastbound and one westbound lane, and opposing eastbound and westbound left-turn lanes. Both the Liberty Street southbound and Roosevelt Avenue northbound approaches to this intersection are each one lane wide.

Please refer to Exhibit 2 of the Appendix which shows a detail of the adjacent signalized intersection.

A review of Connecticut Department of Transportation (CTDOT) data indicates the following:

- West Main Street carries about 13,000 vehicles per day, with about 6,500 vehicles per day traveling each way.
- West Main Street weekday am peak hour falls between 7 and 8 am carrying about 900 vehicles per hour, with 350 vehicles per hour westbound and 550 vehicles per hour eastbound.
- West Main Street weekday pm peak hour falls between 5 and 6 pm carrying about 1100 vehicles per hour, with 600 vehicles per hour westbound and 500 vehicles per hour eastbound.

 West Main Street is posted at 25 miles per hour with average speeds measured at 21 miles per hour, 85th percentile speeds measured at 26 miles per hour, and the measured 10-mile per hour pace (i.e., the 10-mph range which carries the most traffic) computed at 18 to 27 miles per hour.

Site Plan

Please refer to Exhibit 3 in the Appendix which shows a copy of the proposed site plan.

The footprint of the existing building will essentially remain the same and is located along the east side of the parcel. The site drive located at the southern end of the site will remain in its current location and directly interfaces with West Main Street. The 20 parking spaces to be provided will be located between the building and the west side of the parcel, with the west end of the parcel abutting Liberty Street with no site drive on Liberty Street itself. Nine of the parking spaces, including two handicapped spaces, will be located along the front, west side of the building, and the remaining eleven parking spaces will be located along the west end of the parcel. The aisle serving these spaces will run between these two rows of parking and align directly with the site drive which provides access to/egress from the north side of West Main Street.

Floor Space Layout

Please refer to Exhibit 4 in the Appendix which shows a copy of the proposed floor space layout.

The development plan includes refurbishing the interior space located within the existing building to accommodate the proposed dispensary functions. The interior space will include a retail dispensary area, a check-in area, an employee only zone, rest rooms, a vault, an office, a break room, and employee lockers. In all, the dispensary function will consist of about 2448 square feet of gross floor area, wherein it is the gross floor area of the specific use that is traditionally utilized in calculating the estimated trip generation estimates below.

Trip Generation

In estimating the likely trip generation associated with the proposed cannabis dispensary use, we utilized the trip generation calculations that are made available to the traffic engineering profession by the Institute of Transportation Engineers (ITE) in its data source entitled <u>Trip Generation Manual</u>.

This document provides trip generation equations derived from sources from throughout the country for various land uses based on the size of an independent variable. For most land uses, as with the subject dispensary, that independent variable is the size of the space devoted to the use, typically the gross floor area of the space allotted to the use.

Table A Proposed Cannabis Dispensary 389 West Main Street (Route 1) Stamford, Connecticut

Building Size (SF):

Retail Space:	1458
Vault:	295
Other Ancillary Space:	695
Total:	2448
Hours of Operation:	
Monday thru Saturday:	9:00 am to 8:00 pm
Sunday:	9:00 am to 4:00 pm
Trip Generation (2-way):	
Weekday Average Daily Trips:	620
Weekday AM Peak Hour of Adjacer	nt Street: 26
Weekday AM Peak Hour of Genera	itor: 52
Weekday PM Peak Hour of Adjacer	nt Street: 54
Weekday PM Peak Hour of Genera	tor: 74
Saturday Average Daily Trips:	636
Saturday Peak Hour of Generator:	90
Parking Spaces:	
Standard passenger vehicles:	18
Handicap parking spaces:	2
Total:	20

Bubaris Traffic Associates
December 2023

Please refer to Table A on the previous page which summarizes the trip generation estimates for various time periods based on the trip generation calculations summarized in Exhibit 5 of the Appendix for ITE Land Use Code No. 882.

A review of Table A indicates that the expected trip generation for the proposed cannabis dispensary use would vary between 26 to 90 vehicles per hour over the course of a typical day. As a "worse case", 90 vehicle per hour would convert to a maximum of 1.5 vehicles per minute which can certainly be accommodated by the proposed site drive at its interface with West Main Street with no adverse consequence.

Very truly yours, Bubaris Traffic Associates

T. Bubans

James G. Bubaris, P.E.

President

Conn. Reg. No. 9203

Cc: Carl Tirella, Jr.
BUDR Hartford Holding, LLC
1251 South Main Street
Middletown, CT 06457

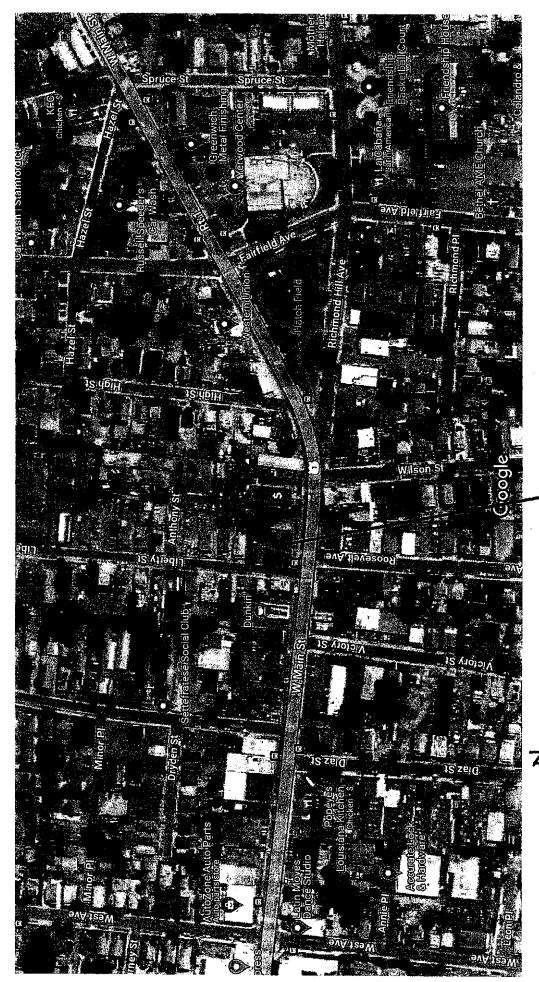
Site Traffic Assessment Proposed Cannabis Dispensary 389 West Main Street (Route 1) Stamford, Connecticut

APPENDIX

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Exhibit 2	Adjacent Signalized intersection to the Site
Exhibit 3	Site Plan
Exhibit 4	Floor Space Layout
Exhibit 5	Trip Generation Calculations

Exhibit 1
Location Map
Proposed Cannabis Dispensary
389 West Main Street (Route 1)
Stamford, Connecticut



200 ft bus, Maxar Technologies, New York GIS, USDA/FPAC/GEO, Map data @2023 Imagery @2023 Airbus, CNES / Air

Sita

Exhibit 2
Adjacent Signalized Intersection to Site
West Main Street (Route 1) at Liberty Street and Roosevelt Avenue
Stamford, Connecticut

imagery ©2023 Airbus, CNES / Airbus, Maxar Technologies, New York GIS, Map data ©2023 50 ft

Z

Site

Exhibit 3
Site Plan
Proposed Cannabis Dispensary
389 West Main Street (Route 1)
Stamford, Connecticut

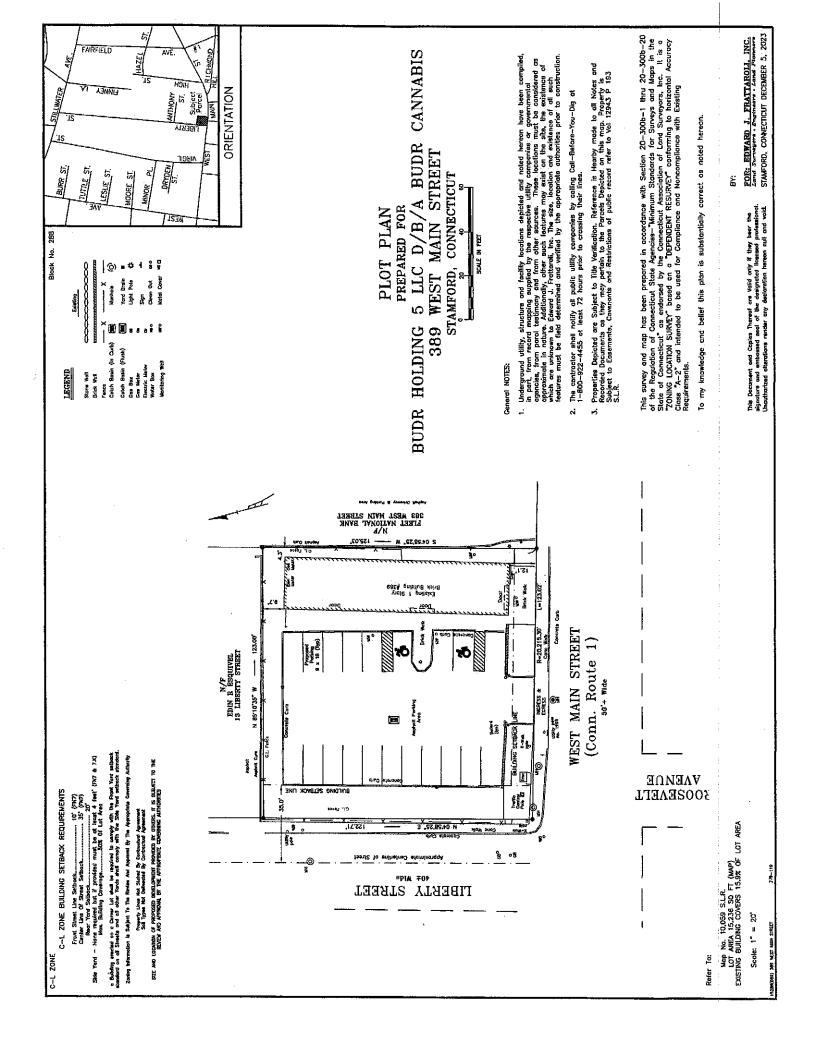


Exhibit 4
Floor Space Layout
Proposed Cannabis Dispensary
389 West Main Street (Route 1)
Stamford, Connecticut

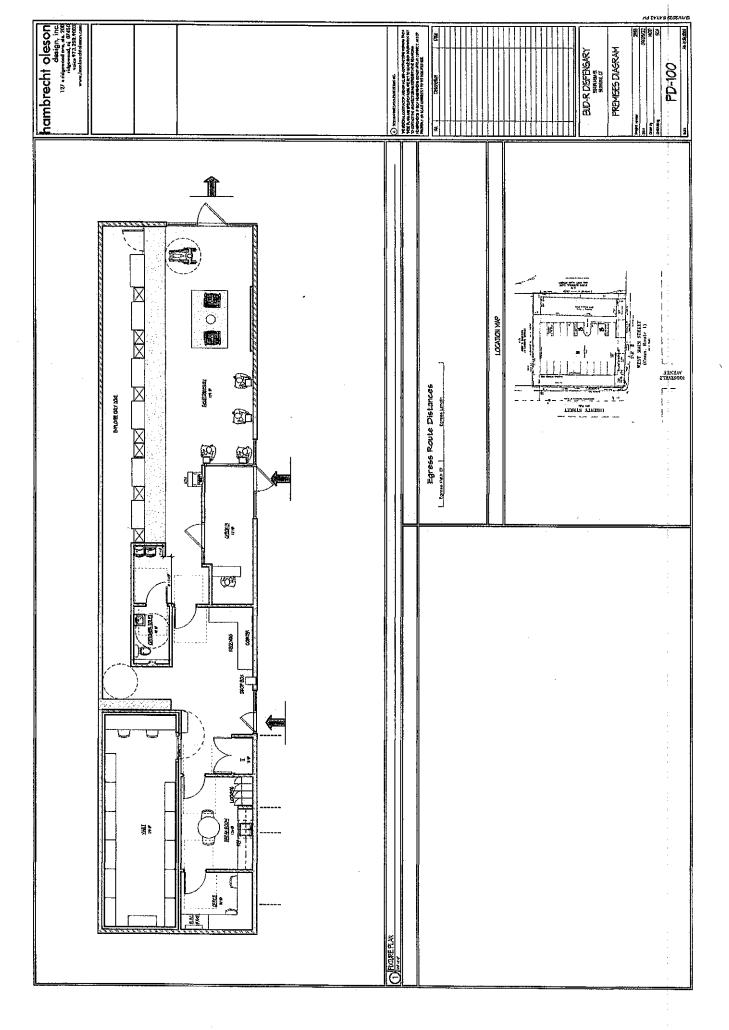


Exhibit 5
Trip Generation Calculations
Proposed Cannabis Dispensary
389 West Main Street (Route 1)
Stamford, Connecticut

Detailed Land Use Data For 2.45 1000 Sq. Ft. GFA of MARIJDISP 1 (882) Marijuana Dispensary

12/8/2023 12/8/2023

Open Date: Analysis Date:

Project: 389 West Main, Stamford, CT

낊 Equation False False False False Use False Ö False False က္ရ % 8 44 22 20 22 20 % 52 56 20 က္ထ S Size N 2 N N 2 2 336.11 26.07 41.69 364.24 27.36 50.44 Sta 7 791.22 128.38 Rate 63.51 852.03 118.92 Max 31.08 98.65 79 74 Rate Ξ 6.33 1.17 5.88 75.34 10.85 2.94 252.7 Avg Rate 20.88 10.44 29.93 259.31 21.83 36.43 Pass-By Trips 0 0 0 O 0 0 Total Trips 619 635 5 26 53 $^{\circ}$ 8 Source: Trip Generation Manual 10th Edition Source: Trip Generation Manual 10th Edition Weekday AM Peak Hour of Adjacent Street Traffic Source: Trip Generation Manual 10th Edition Source: Trip Generation Manual 10th Edition Weekday PM Peak Hour of Adjacent Street Traffic Source: Trip Generation Manual 10th Edition Source: Trip Generation Manual 10th Edition Source: Trip Generation Manual 10th Edition Weekday AM Peak Hour of Generator Weekday PM Peak Hour of Generator Saturday Peak Hour of Generator Weekday Average Daily Trips Saturday Average Daily Trips Day / Period

Source: Institute of Transportation Engineers, Trip Generation Manual 10th Edition TRIP GENERATION 10, TRAFFICWARE, LLC

Trip Generation Summary

Alternative: Alternative 1

Phase: Project:

389 West Main, Stamford, CT

Open Date:

12/8/2023

Analysis Date: '

12/8/2023

Weekday	Average	Daily	Trips
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Saturday Average Daily Trips

ITE Land Use	 Enter	Exit	Total	*	Enter	Exit	Total
882 MARIJDISP 1 2.45 1000 Sq. Ft. GFA	310	309	619		318	317	635
Unadjusted Volume	 310	309	619		318	317	635
Internal Capture Trips	0 -	0	0		0	0	0
Pass-By Trips	0	0	0		0	0	0
Volume Added to Adjacent Streets	310	309	619		318	317	635

Total Weekday Average Daily Trips Internal Capture = 0 Percent Total Saturday Average Daily Trips Internal Capture = 0 Percent

Trip Generation Summary

Alternative: Alternative 1

Phase:

200

Project: 389 West Main, Stamford, CT

Open Date: 12/8/2023

Analysis Date: 12/8/2023

	۸	Weekday AM Peak Hour of Adjacent Street Traffic	M Peak H Street Tra	our of ffic	8	Weekday PM Peak Hour of Adjacent Street Traffic	f Peak Ho Street Traf	rur of fic	×	Weekday AM Peak Hour of Generator	ay AM Peak Ho Generator	ur of	We	Weekday PM Peak Hour of Generator	Peak Ho erator	ur of
ITE Land Use	*	Enter	蓝	Total	*	Enter	Exit	Total	*	Enter	EX	Total	*	Enter	Έ	Total
882 MARIJDISP 1		15	7	56		27	56	53		27	24	51		37	36	73
2.45 1000 Sq. Ft. GFA																
Unadjusted Volume		5	11	56		27	26	53		27	24	55		37	38	23
Internal Capture Trips			0			0	0	0		0	0.	0			0	0
Pass-By Trips		0	0	0		0	0	0		0	0	0		0	0	0
Volume Added to Adjacent Streets		15	=	26		27	56	53		27	24	51		37	36	23

Total Weekday AM Peak Hour of Adjacent Street Traffic Internal Capture = 0 Percent

Total Weekday PM Peak Hour of Adjacent Street Traffic Internal Capture = 0 Percent

Total Weekday AM Peak Hour of Generator Internal Capture = 0 Percent

Total Weekday PM Peak Hour of Generator Internal Capture = 0 Percent

Custom rate used for selected time period.

Trip Generation Summary

Alternative: Alternative 1

Phase: Project:

389 West Main, Stamford, CT

Open Date:

12/8/2023

Analysis Date:

12/8/2023

Saturday Peak Hour of Generator

Sunday Peak Hour of Generator

ITE Land Use	 Enter	Exit	Total	*	Enter	Exit	Total
882 MARIJDISP 1 2.45 1000 Sq. Ft. GFA			89				O
Unadjusted Volume	 0	0	0		0	0	0
Internal Capture Trips	0	0	ó		0	0	. 0
Pass-By Trips	0	0	0		0	0	0
Volume Added to Adjacent Streets	0	0	0		0	0	. 0

Total Saturday Peak Hour of Generator Internal Capture = 0 Percent Total Sunday Peak Hour of Generator Internal Capture = 0 Percent

^{* -} Custom rate used for selected time period.

Properties Depicted are Subject to Title Verification. Reference is Hearby made to all Notes and Recorded Decuments as they may pertain to the Parcels Depicted on this map. Property is Subject to Easements, Covenants and Restrictions of public record refer to Vol 1294.3 P 193. S.L.R. ORIENTATION The contractor shall notify all public utility companies by calling Call-Before-You-Dig at 1-800-922-4455 at least 72 hours prior to crossing their lines. AIBOIT BURR ST. JUTILE ST. DRYDEN ST. LESUE ST. MINOR PL STAMFORD, CONNECTICUT MOORE ST. PREPARED FOR PLOT PLAN Block No. 288 SCALE IN FEET Sign Claran Dut Metal Cover Yard Drain Light Pole BUDR HOLDING Fence Catch Basin (in Curb) Colch Basin (Flush) Manitaring Well General NOTES: Stone Wall Brick Wall N/F Pleet National Bank 383 West Main Street ģa Sec. Cas. LETTER THE Briek Wolk 1=123.02 Concrete Curb Brick Welk WEST MAIN STREET 123.00 (Conn. Route 1) N/F EDIN R ESQUIVEL 13 LIBERTY STRRET N 85'10'35" W MORESS & Utility pole No. 1998 (d.) BULDING SETBACK SIZE AND LOCATION OF PROPOSED DEVELOPMENT & PARKAGE PROMISED BY CITHCHES, IT IS SUBJECT TO THE REVIEW AND APPROVAL BY THE APPROPRIATE CONFIGURAC ALTHORITIES Ampholit Curb o Bailding erected on a Cornor Lot shall be raquired to comply with the Front Yord setback atomdard on all Streets and all other Yords shall comply with the Side Yard setback standard Zoning Information is Subject to the Review And Approval By the Appropriate Governing Authority VAENOE 300SEAELT 3NIT C-L ZONE BUILDING SETBACK REQUIREMENTS 9 Property Lines Not Stated By Contractual Agreement Soil Types Not Definedted By Contractual Agreement 155,71 N 04.28,32 E 40¥ Alqe SLEEEL LIBERTY C-L ZONE

AMTHONY ST, PECH

Subject .

NAM

CANNABIS 5 LLC D/B/A BUDR 389 WEST MAIN STREET

- Underground utility, structure and facility locations depicted and noted hereon have been compiled,
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This survey and map has been prepared in accordance with Section 20–300b–1 thru 20–300b–20 of the Reguldtion of Connecticut. State Agencies-"Minimum Standards for Surveys and Maps in the State of Connecticut" as endorsed by the Connecticut Association of Land Surveyors, Inc. It is a ZONNNG LOCATION SURVEY. based on a "DEPENDENT RESURVEY" conforming to horizontal Accuracy Class: "A-2" and intended to be used for Compliance and Noncompliance with Existing

To my knowledge and belief this plan is substantially garrect as noted her



Map No. 10,059 S.L.R. LOT AREA 15,236 SQ FT (MAP) EXISTING BUILDING COVERS 15.9% OF LOT AREA

Refer To;

Scale: 1" = 20"

STAMFORD, CONNECTICUT DECEMBER 5, 2023 FOR: EDWARD J. FRATTAROLL, INC.

This Document and Copies Thereof are Volid only if they bear the signature and embossed seal of the designated licensed professional. Unauthorized alterations render any declaration harson nutil and void.



January 24, 2024

Carl Tirella, Jr.
BUDR Hartford Holding, LLC
1251 South Main Street
Middletown, CT 06457

Re: Site Traffic Assessment

Proposed Cannabis Dispensary 389 West Main Street (Route 1)

Stamford, Connecticut

The site is located at the northeast quadrant of the signalized intersection of West Main Street (Route 1) at Liberty Street and Roosevelt Avenue, with one two-way site drive serving the site intersecting the north side of West Main Street directly east of Liberty Street.

Please refer to Exhibit 1 of the Appendix which locates this site with respect to the surrounding roadway network.

Please refer to Table A on the next page which summarizes the development parameters for the proposed dispensary.

Traffic Conditions

West Main Street is a state highway (Route 1) running east-west in the vicinity of the site, with one eastbound and one westbound lane, and opposing eastbound and westbound left-turn lanes. Both the Liberty Street southbound and Roosevelt Avenue northbound approaches to this intersection are each one lane wide.

Please refer to Exhibit 2 of the Appendix which shows a detail of the adjacent signalized intersection.

A review of Connecticut Department of Transportation (CTDOT) data indicates the following:

- West Main Street carries about 13,000 vehicles per day, with about 6,500 vehicles per day traveling each way.
- West Main Street weekday am peak hour falls between 7 and 8 am carrying about 900 vehicles per hour, with 350 vehicles per hour westbound and 550 vehicles per hour eastbound.

Table A Development Parameters Proposed Cannabis Dispensary 389 West Main Street (Route 1) Stamford, Connecticut

Building Size (SF):

Retail Space: 1458
Vault: 295
Other Ancillary Space: 695
Total: 2448

Hours of Operation:

Monday thru Saturday: 9:00 am to 8:00 pm Sunday: 9:00 am to 4:00 pm

Trip Generation (2-way):

Weekday AM Peak Hour of Adjacent Street: 22
Weekday PM Peak Hour of Adjacent Street: 46
Saturday Peak Hour of Generator: 70

Parking Spaces:

Standard passenger vehicles: 18
Handicap parking spaces: 2

Total: 20

Bubaris Traffic Associates
January 2024

- West Main Street weekday pm peak hour falls between 5 and 6 pm carrying about 1100 vehicles per hour, with 600 vehicles per hour westbound and 500 vehicles per hour eastbound.
- West Main Street is posted at 25 miles per hour with average speeds measured at 21 miles per hour, 85th percentile speeds measured at 26 miles per hour, and the measured 10-mile per hour pace (i.e., the 10-mph range which carries the most traffic) computed at 18 to 27 miles per hour.

Building Layout

Please refer to Exhibit 3 in the Appendix which shows a copy of the proposed building layout.

The development plan includes refurbishing the existing building located on the site to repurpose the interior space to accommodate the dispensary functions. The interior space will include a reception area, a sales area, a vault area to secure the product, a cash-out function, and auxiliary storage space. In all, the building will consist of about 2448 square feet of gross floor area, wherein it is the gross floor area of the specific use that is typically utilized in calculating the estimated trip generation.

Site Plan

Please refer to Exhibit 4 in the Appendix which shows a copy of the proposed site plan.

The footprint of the existing building will essentially remain the same and is located along the east side of the parcel. The site drive located at the southern end of the site will remain in its current location and directly interfaces with West Main Street. The 20 parking spaces to be provided will be located between the building and the west side of the parcel, with the west end of the parcel abutting Liberty Street with no site drive on Liberty Street itself. Nine of the parking spaces, including two handicapped spaces, will be located along the front, west side of the building, and the remaining eleven parking spaces will be located along the west end of the parcel. The aisle serving these spaces will run between these two rows of parking and align directly with the site drive which provides access to/egress from the north side of West Main Street.

Trip Generation

In estimating the likely trip generation associated with the proposed cannabis dispensary use, we utilized the trip generation calculations that are made available to the traffic engineering profession by the Institute of Transportation Engineers (ITE) in its data source entitled <u>Trip Generation Manual</u>.

This document provides trip generation equations derived from sources from throughout the country for various land uses based on the size of an independent variable. For most land uses, as with the subject dispensary, that independent variable is typically the gross floor area of the space allotted to the use.

For this latest iteration of our evaluation of the subject proposal, we have opted to use the trip generation data from the latest edition of the Trip Generation Manual, version 11, versus the data that was used from the prior edition, version 10, previously presented in our preliminary Site Assessment dated December 11, 2023. The latest edition, which has a larger database for this relatively new use includes trip generation factors that are somewhat lower than the previous edition which had limited data for this use.

Please refer to Table B on the next page which summarizes the trip generation estimates for various time periods based on the latest trip generation factors for ITE Land Use Code No. 882, cannabis dispensaries, as follows:

Weekday AM Peak:
Weekday PM Peak:
Saturday Midday Peak:
8.99 trips per 1,000 square feet building area
18.92 trips per 1,000 square feet building area
28.85 trips per 1,000 square feet building area

A review of Table B indicates that the expected trip generation for the proposed cannabis dispensary use would be about 22 trips per hour during the weekday am commuter peak, about 46 trips per hour during the weekday pm commuter peak, and about 70 trips per hour during the Saturday midday retail peak, all of which are considered relatively low trip generation values, distributed 50/50 east and west of the site.

Existing Traffic Volumes

For the purpose of determining the existing traffic volumes traveling through the study area, manual turning movement counts were conducted during mid-January 2024 during representative peak periods at the adjacent signalized intersection as follows:

Time Interval
8 am to 10 am 4 pm to 6 pm
11 am to 1 pm

Table B
Trip Generation and Distribution
Proposed Cannabis Dispensary
389 West Main Street (Route 1)
Stamford, Connecticut

				Trip Dist	<u>tribution</u>
				To/From	To/From
				West	East
	Tr	ip Generati	<u>on</u>	via Route 1	via Route 1
	<u>Inbound</u>	Outbound	Total	(50%)	(50%)
Analysis Time Period					
Weekday AM Peak Hour of Adjacent Street:	11	11	22	11	11
Weekday PM Peak Hour of Adjacent Street:	23	23	46	23	23
Saturday Midday Peak Hour of Generator:	35	35	70	35	35

Bubaris Traffic Associates January 2024 Please refer to Exhibit 5 of the Appendix which graphically summarizes the existing 2024 peak hour traffic volumes for the weekday am, weekday pm, and Saturday midday peaks for the adjacent signalized intersection of West Main Street at Liberty Street and Roosevelt Avenue.

Background and Combined Traffic Volumes

Please refer to Exhibits 6 and 7 of the Appendix which graphically depict the background 2026 (no-build) and combined 2026 (build) peak hour traffic volumes, respectively, for the adjacent signalized intersection of West Main Street at Liberty Street and Roosevelt Avenue during the weekday am, weekday pm, and Saturday midday peaks.

The background (no-build) peak hour traffic volumes were derived by projecting the existing traffic volumes from Exhibit 5 two years forward assuming a 2 percent per year growth in the pass-by traffic volumes, which is a typical growth rate.

The combined traffic volumes were derived by adding the estimated trip generation volumes from Table B to the background (no-build) traffic volumes from Exhibit 6 to develop the combined (build) traffic volumes shown in Exhibit 7.

Traffic Operations Analyses

Please refer to Exhibit 8 of the Appendix which includes a copy of the traffic control signal plan for the adjacent intersection of West Main Street at Liberty Street and Roosevelt Avenue.

A review of Exhibits 2 and 8 of the Appendix show that the two West Main Street eastbound and westbound approaches, are each two lanes wide with one dedicated left-turn lane and one combination through/right lane. The Roosevelt Avenue northbound and Liberty Street southbound approaches, which are offset from one another, are each one lane wide to accommodate combination left/through/right movements. There are sidewalks on both sides of all four approaches, and there are crosswalks across all four legs. The traffic signal operates to basically move the eastbound and westbound approaches during the same phase, and the two side-street approaches, which are offset, are programmed to move on each of their separate phases. Pedestrians are provided Walk signals at the end of each of the four crosswalks and are moved concurrently with the associated vehicular through movement.

All four stop bars at this intersection are set back to: (1) make space for the location of the crosswalks, and (2) to provide clear paths for turning movements to and from the four approaches. The site drive for no. 389 is located on the north side of West Main Street between Liberty Street and the stop bar for the West Main Street westbound approach. As such, this creates a situation where traffic traveling in and out of the dispensary site is not directly controlled by the traffic signal and free to move in and out as it sees fit, which works to the benefit of the site.

Intersection traffic operational analyses were performed for the adjacent signalized intersection and the subject site drive intersection utilizing the methodology described in the latest edition of Highway Capacity Manual, Special Report 209, Transportation Research Board, 1985, updated to 2010. Application of this methodology was facilitated by use of Synchro Analysis Software, developed by the Trafficware Corporation, Version 10, 2020. Operational analyses are utilized to determine a Level of Service (LOS) for a given intersection operating under either signalized or unsignalized control.

In the case of <u>signalized intersections</u> similar to the intersection adjacent to the site, Level of Service (LOS) is defined in terms of control delay, which is a measure of driver discomfort, frustration, increased fuel consumption, and lost of travel time. The delay experienced by a motorist is comprised of several factors that relate to control, geometric, traffic, and incidents. Total delay is the difference between the travel time experienced and the reference travel time that would result during base conditions in the absence of traffic control, geometric delay, any incidents, and any other vehicles. Specifically, LOS criteria for traffic signals are stated in terms of the average control delay per vehicle, typically for a 15-minute analysis period. Delay is a complex measure and depends on several variables, including the quality of progression, the cycle length, the green ratio, and the volume-to-capacity (v/c) ratio for the lane group. In the case of signalized intersections, the Level of Service for each approach is computed, and an overall Level of Service for the entire intersection is determined. In today's environment, Levels of Service C to D are considered acceptable, and Levels of Service A to B are seldom achieved at signalized intersections.

Please refer to Exhibit 9-A of the Appendix which provides the definitions of levels of service for <u>signalized intersections</u>.

In the case of <u>unsignalized intersections</u> similar to the proposed site drive intersection, Level of Service (LOS) is defined in terms of the average control delay for the approach or movement evaluated. Control delay involves movements at slower speeds and stops on intersection approaches as vehicles move up in the queue or slow down upstream of an intersection. The delay experienced by a motorist is comprised of factors that relate to control, geometrics, traffic, and incidents. Total delay is the difference between the travel time actually experienced and the reference time that would result during base conditions in the absence of incident, control, traffic, or geometric delay. Control delay

includes initial deceleration delay, queue move-up time, stopped delay, and final acceleration delay. At two-way stop-controlled and all-way stop-controlled intersections, control delay is the total elapsed time from a vehicle joining the queue until its departure from the stopped position at the head of the queue. The control delay also includes the time required to decelerate to a stop and to accelerate to the free-flow speed. Level of Service for a one-way or two-way stop-controlled intersection is determined by the computed or measured control delay and is defined for each minor movement. LOS for a one-way or two-way stop-controlled intersection is **not defined** for the intersection as a whole. In today's environment, Levels of Service D to F are common and are often experienced on minor street approaches to major streets carrying relatively high traffic volumes.

Please refer to Exhibit 9-B in the Appendix, which provides details on the definitions of Levels of Service for <u>unsignalized intersections</u>.

The results of the operational analyses, which evaluate 2024 existing, 2026 background (no-build), and 2026 combined (build) peak hour traffic operations for the three peak hour scenarios defined above are summarized in Table C on the next page.

The computer-generated worksheets for these traffic operations analyses are included in the Appendix as follows:

- Exhibit 10 Existing 2024 Peak Hours
- Exhibit 11 Background 2026 (no-build) Peak Hours
- Exhibit 12 Combined 2026 (build) Peak Hours

A review of Table C shows that levels of service for the intersection of West Main Street (Route 1) at Liberty Street and Roosevelt Avenue will continue to operate at excellent overall level of service A for the entire intersection taken as a whole, with the West Main Street approaches operating at excellent level of service A, and the side street approaches operating at very good level of service B.

In addition, a review of Table C shows that the West Main Street at site drive intersection will provide excellent level of service A for the inbound left turn and right turn movements, and very good level of service B to good level of service C for the outbound movements.

Therefore, the proposed development should not have an adverse impact on pass-by traffic travelling through this area.

Table C
Summary of Traffic Operations Analysis
Levels of Service
Proposed Cannibas Dispensary Site
389 West Min Street (Route 1)
Stamford, Connecticut

Intersection	Veekday AM Peak	————————————————————————————————————	Saturday Midday Peak	<backg Weekday AM Peak</backg 	Background (no-build) 2026> Weekday Weekday Saturday AM Peak Midday Pez	2026> Saturday Midday Peak	CONTRACTOR Weekday AM Peak		Saturday Midday Peak
West Main Street at Liberty Street and Roosevelt Avenue									
West Main Street eastbound approach	LOS A	LOS A	LOS A	LOS A	LOS A	LOS A	LOS A	LOS A	LOS A
Delay per vehicle (sec.)	4.1	5.3	4.5	4.2	5.5	4.7	4.2	5.6	4.8
West Main Street westbound approach	LOS A	LOS A	LOS A	LOS A	LOS A	LOS A	LOS A	LOS A	LOS A
Delay per vehicle (sec.)	4.7	4.8	4.7	4.8	5.0	4.8	4.8	5.0	4.9
Roosevelt Avenue northbound approach	LOS B	LOS B	LOS B	LOS B	LOS B	LOS B	LOS B	LOS B	LOS B
Delay per vehicle (sec.)	18.3	16.3	16.8	18.3	16.1	16.7	18.3	16.1	16.7
Liberty Street southbound approach	LOS B	LOS B	LOS B	LOS B	LOS B	LOS B	LOSB	LOS B	LOS B
Delay per vehicle (sec.)	17.6	17.6	17.6	17.7	17.6	17.6	17.7	17.6	17.6
Overall Intersection Delay per vehicle (sec.) Maximum volume/capacity ratio	- LOS A -	- LOS A -	- LOS A -	- LOS A -	- LOS A -	- LOS A -	- LOS A -	- LOS A -	- LOS A -
	6.5	6.2	6.2	6.6	6.4	6.2	6.6	6.4	6.3
	0.47	0.45	0.43	0.48	0.47	0.43	0.48	0.48	0.43
West Main Street at Dispensary Site Drive									
West Main Street eastbound inbound left Delay per vehicle (sec.)							LOS A 8.4	LOS A 8.7	LOS A 8.7
Site Drive southbound outbound approach Delay per vehicle (sec.)							LOS B 14.4	LOS C 20.2	LOS C 22.4

Bubaris Traffic Associates January 2024

Traffic Crash Analysis

A review was made of the most recent available three-year traffic crash experience summary for the immediate site drive intersection of West Main Street at Liberty Street and Roosevelt Avenue, and or the West Main Street at no. 389 site drive intersection, rive as compiled in UConn's Traffic Crash Data Depository from information provided by CTDOT, and state and municipal police departments.

A review of the latest UConn records for the adjacent signalized intersection shows only a total of 7 crashes during this 3-year period, and for the subject site drive intersection shows no traffic crash experience whatsoever for at least this three-year period, with no recurring problems requiring correction, or that may be exacerbated by the proposed development.

Please refer to Table D on the next page for a summary of this traffic crash review.

Sight Line Evaluation

Available sight lines from the proposed site drive location were evaluated utilizing the guidelines set forth by the CTDOT for this purpose.

West Main Street is posted at 25 miles per hour with measured 85th percentile speeds recently measured by CTDOT at 26 miles per hour, for which CTDOT guidelines recommend minimum sight distance of 305 feet.

Please refer to Exhibit 14 of the Appendix which demonstrates that adequate sight lines in each direction to and from the subject site drive are available.

Recommended Improvements

Given the foregoing favorable traffic operations, satisfactory traffic crash experience, and available sight line distances, traffic control and/or geometric improvements are neither deemed necessary nor recommended.

Summary of Traffic Crash Experience Most Recent 3-Year Experience 2019 thru 2021 Table D

Immediate Study Area Proposed Cannabis Dispensary, 389 West Main Street Stamford, Connecticut Source: UConn Traffic Crash Data Depository

TOTAL	9	~		7
Total	0	0	ļ	0
S B			!	0
WB				0
8				0
Total	က	-		4
NB / SB	7			7
EB / WB	~	-		2
Total	0	0		0
WB / SB				0
WB / NB				0
EB/SB				0
EB / NB				0
Total	က	0		ო
SB/SB	~			-
NB / NB				0
WB / WB	~			~
EB / EB	~			~
		ā		Total
Intersection	lest Main Street (Route 1) at Lberty Street	'est Main Street (Route 1) at Roosevelt Avenu		
	SB/SB Total EB/NB EB/SB WB/NB WB/SB Total EB/WB NB/SB Total EB WB NB SB	EB/EB WB/WB NB/NB SB/SB Total EB/NB EB/NB WB/NB WB/NB Total EB WB NB SB Total erty Street 1 1 3 0 1 2 3 0	EB/EB WB/NBB SB/SB Total EB/NB EB/NB EB/NB EB/NB EB/NB EB/NB EB/NB Total EB/NB Total EB/NB NB NB Total Total NB NB	EB/EB WB/NB SB/SB Total EB/NB WB/NB Total EB/WB NB/SB Total EB WB NB SB Total 1 1 1 3 0 1 2 3 0 0 1 0 1 1 1 0

Bubaris Traffic Associates January 2024

Conclusions

It is the professional opinion of Bubaris Traffic Associates that the proposed cannabis dispensary, to be located at the northeast quadrant of the signalized intersection of West Main Street at Liberty Street and Roosevelt Avenue, with access to/egress from the surrounding roadway network via the one existing drive serving the site intersecting the north side of West Main Street east of Liberty Street, should not adversely impact traffic operations on the surrounding roadway network in the year 2025-2026 when occupancy of the subject space and full operation is expected.

The proposed development will generate about 22 vehicular trips per hour during the weekday am peak, about 46 trips per hour during the weekday pm peak, and about 70 trips per hour during the Saturday middy peak, where each patron visiting the site will generate one inbound and one outbound vehicular trip. Therefore, the projected patron numbers would be HALF of these estimated trip generation estimates.

Traffic operational analyses indicate that the proposed development should not have an adverse impact on traffic operations with good to excellent levels of service.

The traffic crash experience for the subject study area is satisfactory, with no recurring problems that need to be corrected, or that may be exacerbated by the proposed development.

Available sight line distances at the proposed site drive location were found to be satisfactory. No. 9203

Very truly yours, Bubaris Traffic Associates

ames G. Bubarin

James G. Bubaris, P.E.

President

Conn. Reg. No. 9203

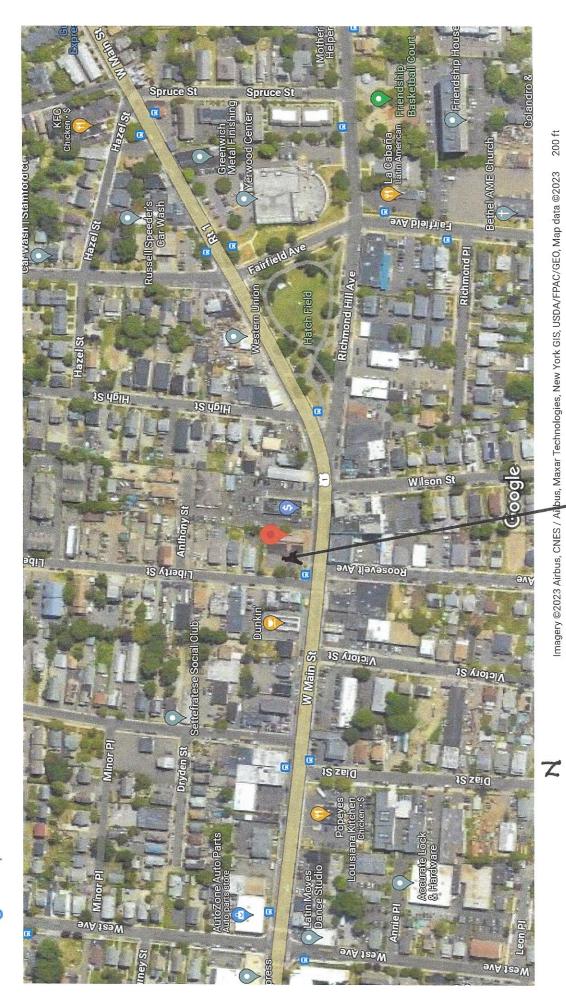
Site Traffic Assessment Proposed Cannabis Dispensary 389 West Main Street (Route 1) Stamford, Connecticut

APPENDIX

Table of Contents

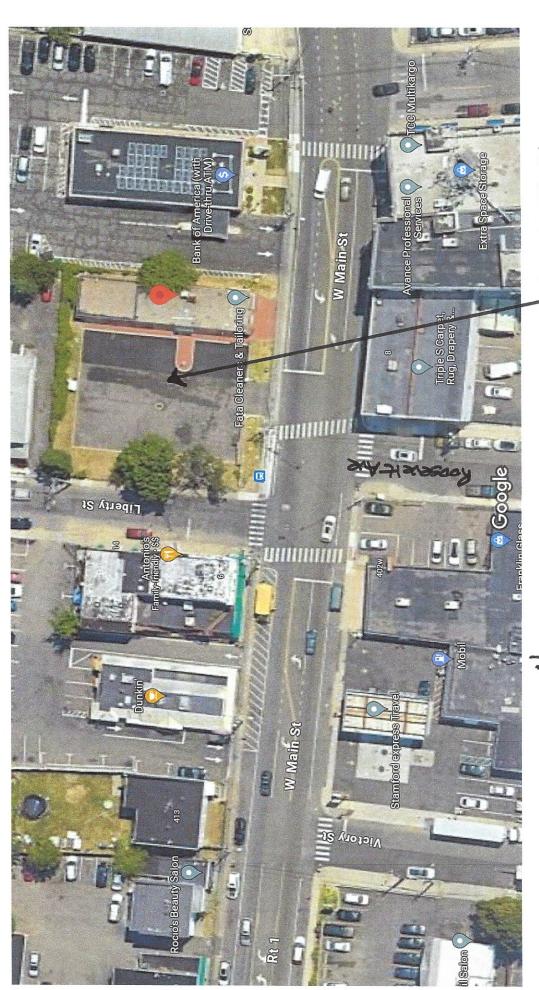
Exhibit 1	Location Map
Exhibit 2	Adjacent Signalized intersection to the Site West Main Street (Route 1) at Roosevelt Avenue and Liberty Street
Exhibit 3	Building Layout
Exhibit 4	Site Plan
Exhibit 5	Existing Peak Hour Traffic Volumes
Exhibit 6	Background (no-build) Peak Hour Traffic Volumes
Exhibit 7	Combined (build) Peak Hour Traffic Volumes
Exhibit 8	Traffic Control Signal Plan West Main Street (Route 1) at Roosevelt Avenue and Liberty Street
Exhibit 9	Definition of Levels of Service
	A. Signalized Intersections B. Unsignalized Intersections
Exhibit 10	Traffic Operations Analyses Existing Peak Hours
Exhibit 11	Traffic Operations Analyses Background (no-build) Peak Hours
Exhibit 12	Traffic Operations Analyses Combined (build) Peak Hours Evaluation of Adjacent Signalized Intersection
Exhibit 13	Traffic Operations Analyses Combined (build) Peak Hours Evaluation of Dispensary Site Drive
Exhibit 14	Sight Line Evaluation Proposed Dispensary Site Drive

Exhibit 1
Location Map
Proposed Cannabis Dispensary
389 West Main Street (Route 1)
Stamford, Connecticut



Site

Exhibit 2
Adjacent Signalized Intersection to Site
West Main Street (Route 1) at Liberty Street and Roosevelt Avenue
Stamford, Connecticut



50 ft Imagery ©2023 Airbus, CNES / Airbus, Maxar Technologies, New York GIS, Map data ©2023

Site

Exhibit 3
Building Layout
Proposed Cannabis Dispensary
389 West Main Street (Route 1)
Stamford, Connecticut

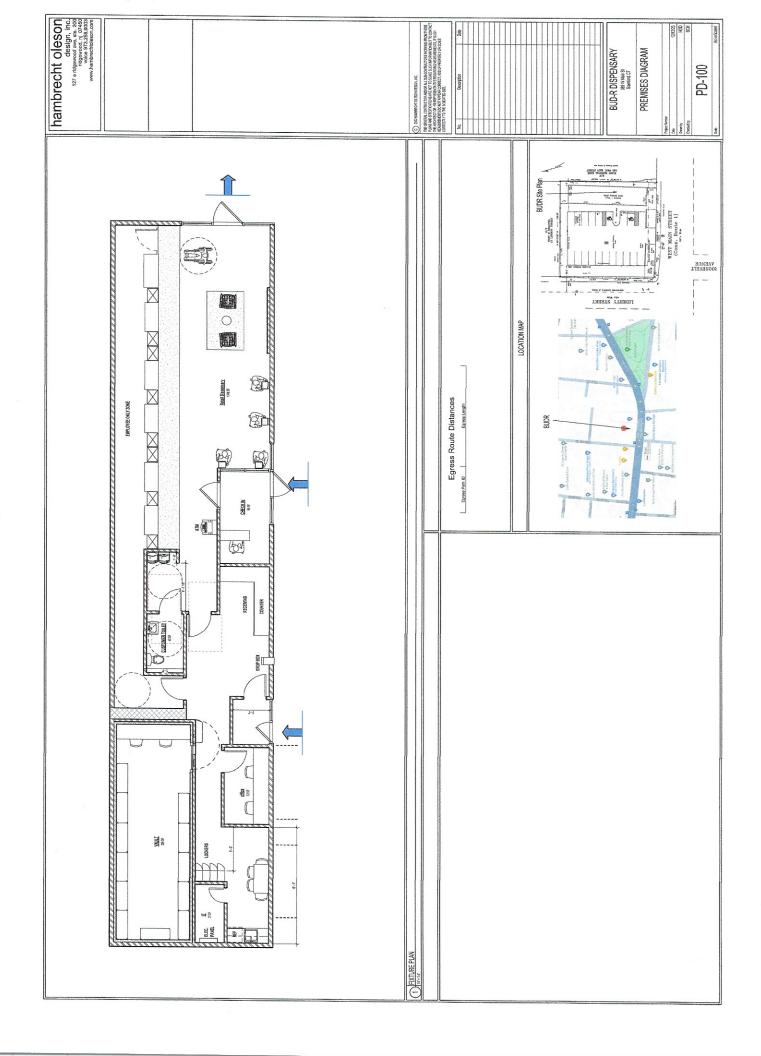


Exhibit 4
Site Plan
Proposed Cannabis Dispensary
389 West Main Street (Route 1)
Stamford, Connecticut

PLOT PLAN m # 1 8 9 Yard Drain Light Pole Sign Clean Out Metal Cover Existing 389 BUDR HOLDING Stone Wall Ezza Brick Wall Ezza Fence Catch Basin (in Curb) Catch Basin (Flush)
Gas Box
Gas Meter
Electric Meter
Water Box Monitoring Well General NOTES: 'n 'n 1 303 MEST MAIN STREET NATIONAL BANK Eber Gos ... ₹0 Brick Walk Existing 1 Story Brick Building #389 L=123.02 Concrete Curb Brick Walk R=20,215.30' Conc Walk WEST MAIN STREET 123.00 (Conn. Route 1) N/F EDIN R ESQUIVEL 13 LIBERTY STREET 10 50'+ Wide N 85'10'35" W INGRESS & **⊚**₹ Utility pole No. 1996 BUILDING SETBACK SIZE AND LOCATION OF PROPOSED DEVELOPMENT & PARKING PROVIDED BY OTHERS. IT IS SUBJECT TO THE REVIEW AND APPROVAL BY THE APPROPRIATE COVERNING AUTHORITIES a Building erected on a Corner Lot shall be required to comply with the Front Yard setback standars standard on all Streets and all other Yards shall comply with the Side Yard setback standars Zoning Information is Subject To The Review And Approval By The Appropriate Coverning Authority Tran C-L ZONE BUILDING SETBACK REQUIREMENTS 35.0' CL. Fence Property Lines Not Staked By Contractual Agreement Soil Types Not Delineated By Contractual Agreement 122.71 N 04.28,32, E g . Utility #10± Wide LIBERTY STREET Side Yard

Subject

DRYDEN

MINOR

LESLIE ST. MOORE ST.

BURR

Block No. 288

C-L ZONE

ORIENTATION MAIN

CANNABIS 5 LLC D/B/A BUDR WEST MAIN STREET STAMFORD, CONNECTICUT PREPARED FOR

- Underground utility, structure and facility locations depicted and noted hereon have been compiled, in part, from record mapping supplied by the respective utility componies or governmental agencies, from parol testimony and from other sources. These locations must be considered as approximate in noture. Additionally, other such features may exist on the site, the existence of which are unknown to Edward J. Frantandi, Inc. The size, location and existence of all such features must be field determined and verified by the appropriate authorities prior to construction.
- The contractor shall notify all public utility companies by calling Call-Before-You-Dig at 1-800-922-4455 at least 72 hours prior to crossing their lines.
- Properties Depicted are Subject to Title Verification, Reference is Hearby made to all Notes and Recorded Documents as they may perfain to the Parcels Depicted on this map. Property is Subject to Easements, Coverants and Restrictions of public record refer to Vol 1294.3 P 193 S.L.R.

This survey and map has been prepared in accordance with Section 20–300b–1 thru 20–300b–20 of the Regulation of Connecticut State Agrantiem in Standards for Surveys and Maps in the State of Connecticut, as endorsed by the Connecticut Association of Land Surveyors, Inc. It is a "ZONING LOCATION SURVEY" based on a "DEPENDENT RESURVEY" conforming to horizontal Accuracy class X-12" and intended to be used for Compliance and Noncompliance with Existing Requirements.

To my knowledge and belief this plan is substantially correct as noted hereo

This Document and Capies Thereof are Valid only if they bear the signature and embossed seal of the designated licensed professional. Unauthorized alterations render any declaration hereon null and vold.

ВҮ:

STAMFORD, CONNECTICUT DECEMBER 5, 2023 FOR: EDWARD J. FRATTAROLI, INC. Land Surveyors .Consultants. Land Planners

LOT AREA

Map No. 10,059 S.L.R. LOT AREA 15,236 SQ FT (MAP) EXISTING BUILDING COVERS 15.9% OF

Refer To:

VAENNE SOOSEAELT

Scale: 1" = 20°

Exhibit 5
Existing Peak Hour Traffic Volumes
West Main Street (Route 1) at Liberty Street and Roosevelt Avenue
Stamford, Connecticut

Exhibit 6 Background (no-build) Peak Hour Traffic Volumes West Main Street (Route 1) at Liberty Street and Roosevelt Avenue Stamford, Connecticut

Exhibit 7
Combined (build) Peak Hour Traffic Volumes
West Main Street (Route 1) at Liberty Street and Roosevelt Avenue
Stamford, Connecticut

Exhibit 8
Traffic Control Signal Plan
West Main Street (Route 1) at Roosevelt Avenue and Liberty Street

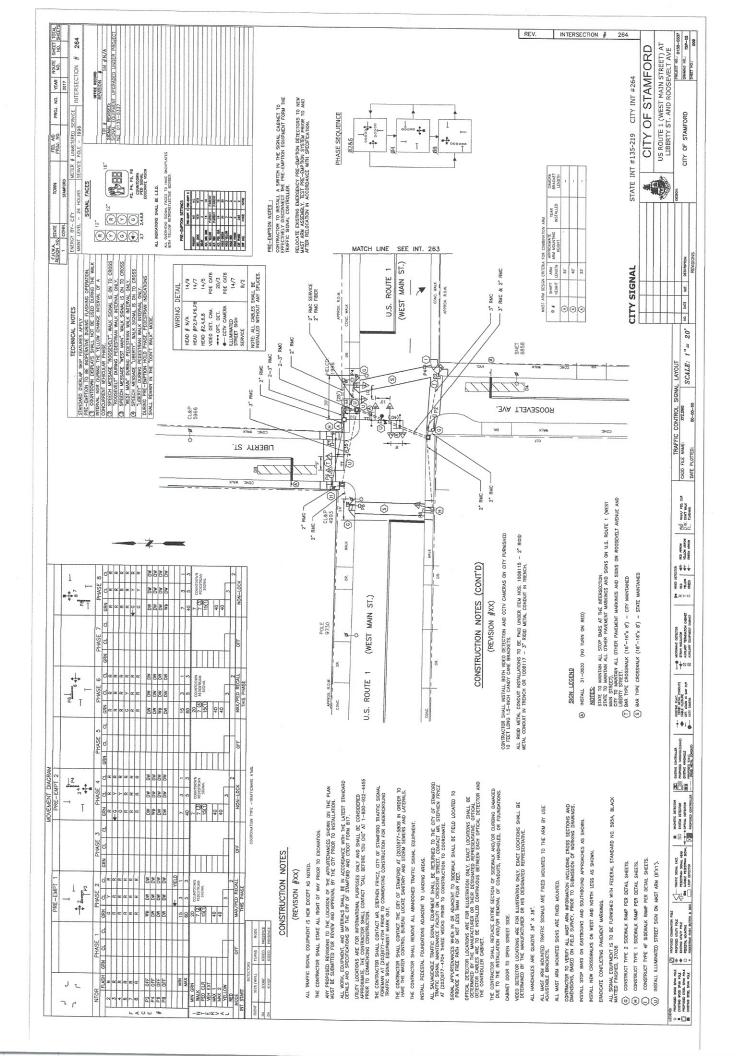


EXHIBIT 9-A LEVEL OF SERVICE CRITERIA SIGNALIZED INTERSECTIONS

SOURCE: <u>HIGHWAY CAPACITY MANUAL (HCM)</u>, 2000 TRANSPORTATION RESEARCH BOARD (1)

Level of Service for **signalized intersections** is defined in terms of control delay, which is a measure of driver discomfort, frustration, increased fuel consumption, and lost travel time. The delay experienced by a motorist is comprised of a number of factors that relate to control, geometric, traffic, and incidents. Total delay is the difference between the travel time actually experienced and the reference travel time that would result during base conditions in the absence of traffic control, geometric delay, any incidents, and any other vehicles. Specifically, LOS criteria for traffic signals are stated in terms of the average control delay per vehicle, typically for a 15-minute analysis period. Delay is a complex measure and depends on a number of variables, including the quality of progression, the cycle length, the green ratio, and the volume-to-capacity (v/c) ratio for the lane group.

In the case of **signalized intersections**, the Level of Service for each approach is computed, and an overall Level of Service for the entire intersection is determined.

Levels of Service (LOS) for signalized intersections are defined as follows:

LEVEL OF SERVICE	CONTROL DELAY PER VEHICLE (SECONDS)	CONDITION
LOS A	<u><</u> 10	LOW DELAY
LOS B	> 10 TO 20	SHORT DELAY
LOS C	> 20 TO 35	AVERAGE DELAY
LOS D	> 35 TO 55	CONGESTION
		NOTICEABLE
LOS E	> 55 TO 80	LIMIT OF
		ACCEPTABLE DELAY
LOS F	> 80	UNACCEPTABLE

In today's environment, Levels of Service C to D are considered acceptable, and Levels of Service A to B are seldomly achieved at signalized intersections.

(1) HCM, Exhibit 16-2.

EXHIBIT 9-B LEVEL OF SERVICE CRITERIA UNSIGNALIZED INTERSECTIONS

SOURCE: <u>HIGHWAY CAPACITY MANUAL (HCM)</u>, 2010 TRANSPORTATION RESEARCH BOARD (1)

Level of Service for **unsignalized intersections** similar to the study intersections is defined in terms of the average control delay for the approach or movement evaluated. Control delay involves movements at slower speeds and stops on intersection approaches as vehicles move up in the queue or slow down upstream of an intersection.

The delay experienced by a motorist is comprised of factors that relate to control, geometrics, traffic, and incidents. Total delay is the difference between the travel time actually experienced and the reference time that would result during base conditions in the absence of incident, control, traffic, or geometric delay. Control delay includes initial deceleration delay, queue move-up time, stopped delay, and final acceleration delay.

At two-way stop-controlled and all-way stop-controlled intersections, control delay is the total elapsed time from a vehicle joining the queue until its departure from the stopped position at the head of the queue. The control delay also includes the time required to decelerate to a stop and to accelerate to the free-flow speed.

Level of Service (LOS) for a two-way stop-controlled intersection is determined by the computed or measured control delay and is defined for each minor movement. LOS is **not defined** for the intersection as a whole.

Level of Service (LOS) for an all-way stop-controlled intersection is determined by the computed or measured control delay and is defined for all movements. A LOS **is then defined** for the intersection as a whole.

Levels of Service (LOS) for unsignalized intersections are defined as follows:

LEVEL OF SERVICE	AVERAGE CONTROL DELAY PER VEHICLE (SECONDS)	CONDITION
LOS A	0 TO 10	LITTLE OR NO DELAY
LOS B	> 10 TO 15	SHORT DELAY
LOS C	> 15 TO 25	AVERAGE DELAY
LOS D	> 25 TO 35	LONG DELAY
LOS E	> 35 TO 50	VERY LONG DELAY
LOS F	> 50	EXTREME DELAY

In today's environment, Levels of Service D to F are common and are often experienced on minor street approaches to major streets carrying relatively high traffic volumes.

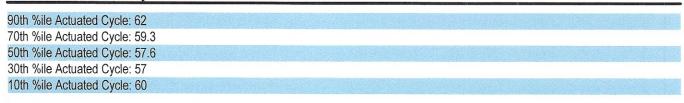
(1) HCM, Exhibits 17-2 and 17-22.

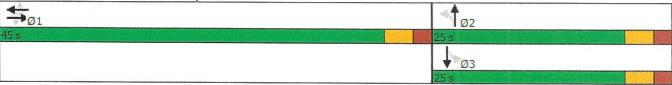
Exhibit 10
Traffic Operations Analyses
Existing Peak Hour Traffic Volumes
West Main Street (Route 1) at Liberty Street and Roosevelt Avenue
Stamford, Connecticut

	۶	→	*	•	+	*	4	1	~	1	+	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	75	f)		79	7>			4			4	
Traffic Volume (vph)	27	274	15	6	399	20	10	3	8	45	7	65
Future Volume (vph)	27	274	15	6	399	20	10	3	8	45	7	65
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%		· <u>-</u>	0%	12	12	0%	12	12	0%	12
Storage Length (ft)	50	0,10	0	60	0 70	0	0	0 70	0	0	0 /0	0
Storage Lanes	1		0	1		0	0		0	0		0
Taper Length (ft)	25			25		0	25		U	25		U
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.992			0.993			0.947			0.925	
Flt Protected	0.950	0.002		0.950	0.000			0.977			0.923	
Satd. Flow (prot)	1770	1848	0	1770	1850	0	0	1723	0	0	1690	0
Flt Permitted	0.490	1040	U	0.570	1000	U	U	0.846	U	U	0.864	0
Satd. Flow (perm)	913	1848	0	1062	1850	0	0	1492	0	0		0
Right Turn on Red	010	10-10	No	1002	1000	No	U	1492	Yes	0	1489	0
Satd. Flow (RTOR)			140			INO		9	165		71	Yes
Link Speed (mph)		35			35			25			71	
Link Distance (ft)		993			1384			635			25	
Travel Time (s)		19.3			27.0						738	
Confl. Peds. (#/hr)		10.0			21.0			17.3			20.1	
Confl. Bikes (#/hr)												
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.00	0.00	0.00
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	0.92	0.92	0.92
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	100% 2%	100%	100%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0		2%	2%
Parking (#/hr)	J				U	U	U	U	U	0	0	0
Mid-Block Traffic (%)		0%			0%			0%			00/	
Adj. Flow (vph)	29	298	16	7	434	22	11	3	9	40	0%	74
Shared Lane Traffic (%)		200	10	1	707	22	11	J	9	49	8	71
Lane Group Flow (vph)	29	314	0	7	456	0	0	23	0	0	400	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	0	0	128	0
Lane Alignment	Left	Left	Right	Left	Left	Right	Left		No	No	No	No
Median Width(ft)	Lon	12	rtigitt	LGIL	12	Rigit	Len	Left	Right	Left	Left	Right
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			0 16			0	
Two way Left Turn Lane		10			10			10			16	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	4.00	4.00	4.00	4.00	4.00
Turning Speed (mph)	1.00	1.00	9	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Number of Detectors	1	2	9	15	2	9	15	0	9	15		9
Detector Template	Left	Thru		-	2 Th:::		1	2		1	2	DOMESTICAL PROPERTY.
Leading Detector (ft)	20			Left	Thru		Left	Thru		Left	Thru	
Trailing Detector (ft)		100		20	100		20	100	Nasaan tarah	20	100	
Turn Type	0 Porm	0		0	0		0	0		_ 0	0	
Protected Phases	Perm	NA 1		Perm	NA		Perm	NA		Perm	NA	
Protected Phases Permitted Phases	A	1		,	1			2			3	
Service and the service and th	1	1		1	1		2	2		3	3	Control of the latest and the latest
Detector Phase Switch Phase	1	1		1	1		2	2		3	3	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Minimum Initial (s)	20.0	20.0		20.0	20.0		7.0	7.0		7.0	7.0	
Minimum Split (s)	25.0	25.0		25.0	25.0		12.0	12.0		12.0	12.0	
Total Split (s)	45.0	45.0		45.0	45.0		25.0	25.0		25.0	25.0	
Total Split (%)	64.3%	64.3%		64.3%	64.3%		35.7%	35.7%		35.7%	35.7%	
Maximum Green (s)	40.0	40.0		40.0	40.0		20.0	20.0		20.0	20.0	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0			0.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0			5.0	77.5		5.0	
Lead/Lag						Alfrida Sala Sala Sala Sala Sala Sala Sala Sa						
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Minimum Gap (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Time Before Reduce (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Time To Reduce (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Recall Mode	Max	Max		Max	Max		None	None		None	None	
Walk Time (s)	mar	Wilde		Max	WIGH		140110	140110		140110	140110	
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	44.0	44.0		44.0	44.0			8.0			8.5	
Actuated g/C Ratio	0.74	0.74		0.74	0.74			0.14			0.14	
v/c Ratio	0.04	0.23		0.01	0.33			0.11			0.47	
Control Delay	3.8	4.1		3.7	4.7			18.3			17.6	
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	
Total Delay	3.8	4.1		3.7	4.7			18.3			17.6	
LOS	A	A		Α	Α.			В			17.0 B	
Approach Delay		4.1		^	4.7			18.3			17.6	
Approach LOS		Α			Α.			10.3 B			17.0 B	
90th %ile Green (s)	40.0	40.0		40.0	40.0		12.0	12.0		12.0	12.0	
90th %ile Term Code	MaxR	MaxR		MaxR	MaxR		Hold	Hold				
70th %ile Green (s)	40.0	40.0		40.0	40.0		0.0	0.0		Gap 9.3	Gap	
70th %ile Term Code	MaxR	MaxR		MaxR	MaxR						9.3	
50th %ile Green (s)	40.0	40.0		40.0	40.0		Skip	Skip		Gap	Gap	
50th %ile Term Code	MaxR	MaxR		MaxR	MaxR		0.0	0.0		7.6	7.6	
30th %ile Green (s)	40.0	40.0		40.0	CONTRACTOR OF STREET		Skip	Skip		Gap	Gap	
30th %ile Term Code	MaxR	MaxR			40.0		0.0	0.0		7.0	7.0	
10th %ile Green (s)	55.0	55.0		MaxR	MaxR		Skip	Skip		Min	Min	
10th %ile Term Code	Dwell	Dwell		55.0	55.0		0.0	0.0		0.0	0.0	
	DWell	Dweii		Dwell	Dwell		Skip	Skip	No. of the second second second	Skip	Skip	
Intersection Summary	041-											
A ATTENDED TO THE PARTY OF THE	Other											
Cycle Length: 70												
Actuated Cycle Length: 59.2												
Natural Cycle: 40												
Control Type: Semi Act-Unc	oord											
Maximum v/c Ratio: 0.47	_											
Intersection Signal Delay: 6.				AND DESCRIPTION OF THE PARTY OF	ntersection							
Intersection Capacity Utiliza	ion 20 00/			10	CU Level	of Service	A e					

Existing Weekday AM Peak

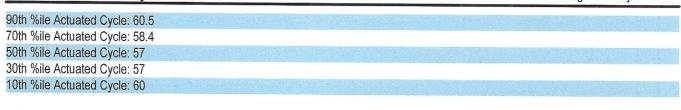


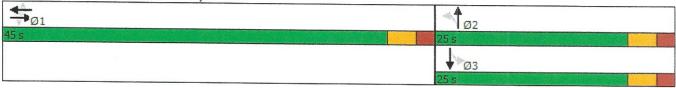


EBI EBI EBI WBI WBR NBL NBL NBR SBL SBI SBR			70 W								Moting 11		- Our
Lane Configurations 1		*	\rightarrow	*	•	-	*	1	†	1	1		1
Lane Configurations	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph)		1	7		79	14			4.				
Future Volume (vph)		57	565	17	14		46	7		15	29		46
Ideal Flow (yphpl) 1900		57	565	17	14	447	46						
Lane Width (ff)	Ideal Flow (vphpl)	1900	1900	1900	1900	1900							
Grade (%) Storage Langh (ft) 50 0 0 60 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		12	12	12	12	12							NAME OF TAXABLE PARTY AND ADDRESS OF TAXABLE PARTY.
Storage Length (ft) 50	Grade (%)		0%			0%						WITH THE PARTY OF	
Storage Lanes		50		0	60		0	0		0	0	070	0
Taper Length (fit)	Storage Lanes	1		0	1								
Lane Uil. Factor		25			25								U
Ped Bike Factor Firt	Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00		1.00	1.00
Fit Protected 0.950										1.00	1.00	1.00	1.00
Filt Profected			0.996			0.986			0.932			0 929	
Satd. Flow (prot) 1770 1855 0 1770 1837 0 0 1715 0 0 1701 0	Flt Protected	0.950			0.950								
Fit Permitted 0.442 0.386 0.924 0.924 0.875 Satd. Flow (perm) 823 1855 0 719 1837 0 0 1604 0 0 1514 0 Right Turn on Red No	Satd. Flow (prot)	1770	1855	0		1837	0	0		0	0		0
Satd, Flow (perm) 823 1855 0 719 1837 0 0 1604 0 0 1514 0 Right Turn on Red No No Tes Yes Yes Satd, Flow (RTOR) 16 50 Link Speed (mph) 35 35 25 25 Link Distance (ft) 993 1384 635 738 Travel Time (s) 19.3 27.0 17.3 20.1 Confl. Peds. (#/hr) 2 27.0 17.3 20.1 Confl. Peds. (#/hr) 2 2 0.92		0.442						•		U	U		U
Right Turn on Red No No Yes Yes Satd, Flow (RTOR)	Satd. Flow (perm)	823	1855	0		1837	0	0		0	0		n
Satd, Flow (RTOR) 35 35 25 25 Link Speed (mph) 35 1384 635 738 Travel Time (s) 19.3 27.0 17.3 20.1 Confl. Bless (#/hr) 50 19.3 27.0 17.3 20.1 Confl. Bless (#/hr) 50 19.2<	Right Turn on Red			No					1001		U	1017	ORDERSON CONTRACTOR AND STREET,
Link Speed (mph)	Satd. Flow (RTOR)								16	100		50	163
Link Distance (ft) 993 1384 635 738 Travel Time (s) 19.3 27.0 17.3 20.1 Confl. Peds (#/hr) Confl. Bikes (#/hr) Peak Hour Factor 100% 100% 100% 100% 100% 100% 100% 100	Link Speed (mph)		35			35							
Travel Time (s) 19.3 27.0 17.3 20.1 Confl. Peds. (#/hr) Confl. Bikes (#/hr) Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92	Link Distance (ft)		993										
Confl. Peds. (#/hr) Confl. Bikes (#/hr) Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92	Travel Time (s)		19.3									THE RESERVED TO SERVED TO	
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Growth Factor 100% 100% 100% 100% 100% 100% 100% 100	Confl. Bikes (#/hr)												
Growth Factor 100% 100% 100% 100% 100% 100% 100% 100	Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.02
Heavy Vehicles (%)	Growth Factor	100%	100%	100%	100%								The Section of the Se
Bus Blockages (#/hr) 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Heavy Vehicles (%)	2%	2%	2%	2%	NAME OF TAXABLE PARTY.							
Parking (#/hr) Mid-Block Traffic (%) 0% 0% 0% 0% Adj. Flow (vph) 62 614 18 15 486 50 8 8 16 32 13 50 Shared Lane Traffic (%) Lane Group Flow (vph) 62 632 0 15 536 0 0 32 0 0 95 0 Enter Blocked Intersection Lane Slow (vph) 62 632 0 15 536 0 0 32 0 0 95 0 Enter Blocked Intersection Lane Slow (vph) Left Left Right Left	Bus Blockages (#/hr)	0	0	0					Manager and Control of the Control	AND THE PERSON NAMED IN COLUMN TWO	Control of the Contro	CHICAGO AND PROPERTY OF THE PR	A STATE OF THE PARTY OF THE PAR
Adj. Flow (vph) 62 614 18 15 486 50 8 8 16 32 13 50 Shared Lane Traffic (%) Lane Group Flow (vph) 62 632 0 15 536 0 0 32 0 0 95 0 Enter Blocked Intersection Lane Alignment Left Left Left Left Left Left Left Left Left Right Left Left												U	U
Adj. Flow (vph) 62 614 18 15 486 50 8 8 16 32 13 50 Shared Lane Traffic (%) Lane Group Flow (vph) 62 632 0 15 536 0 0 32 0 0 95 0 Enter Blocked Intersection No 10 0	Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%) Lane Group Flow (vph) 62 632 0 15 536 0 0 32 0 0 95 0	Adj. Flow (vph)	62	614	18	15		50	8	The same of the sa	16	32		50
Enter Blocked Intersection No No <th< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>10</td><td>UZ.</td><td>10</td><td>30</td></th<>										10	UZ.	10	30
Enter Blocked Intersection No No <th< td=""><td></td><td>62</td><td>632</td><td>0</td><td>15</td><td>536</td><td>0</td><td>0</td><td>32</td><td>0</td><td>0</td><td>95</td><td>0</td></th<>		62	632	0	15	536	0	0	32	0	0	95	0
Lane Alignment Left Left Right Left Right Left Right Link Offset(ft) 0 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 <	Enter Blocked Intersection	No	No	No			THE RESERVE OF THE PARTY OF THE					THE REAL PROPERTY AND ADDRESS OF THE PARTY.	THE PROPERTY AND PROPERTY OF THE PARTY OF TH
Median Width(ft) 12 12 12 0 0 0 1 1 1 1 1 1 1 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Lane Alignment	Left	Left	Right	THE RESERVE THE PARTY OF THE PA	NAME OF TAXABLE PARTY.	NAME OF TAXABLE PARTY.	Color of the Color	Constitution of the last of th	CONTRACTOR OF THE PARTY OF THE	THE RESERVE OF THE PARTY OF THE		CANCEL CONTRACTOR OF THE PARTY
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Crosswalk Width(ft) 16 16 16 16 16 16 16 16 16 16 16 Two way Left Turn Lane 16 10 20 100 1	Link Offset(ft)		0			Allen and the second second			THE REAL PROPERTY AND ADDRESS OF THE PERSON NAMED IN COLUMN TWO			-	
Two way Left Turn Lane Headway Factor 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0			16										
Turning Speed (mph) 15 9 15 2 15 9 <	Two way Left Turn Lane								10			10	
Turning Speed (mph) 15 9 15 2 15 9 <	Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Number of Detectors 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 1 2 1 2 1 1 2 1 1 1 2 1 1 1 1 2 2 3 3 2 2 3	Turning Speed (mph)						CONTRACTOR DESCRIPTION OF THE PARTY OF THE P		1.00	THE RESIDENCE OF THE PARTY OF T	THE RESERVE OF THE PERSON NAMED IN	1.00	
Detector Template Left Thru Left <td>Number of Detectors</td> <td>1</td> <td>2</td> <td></td> <td></td> <td>2</td> <td></td> <td>- Control Control Control Control</td> <td>2</td> <td>U</td> <td>A PROPERTY OF THE PARTY OF THE</td> <td>2</td> <td>9</td>	Number of Detectors	1	2			2		- Control Control Control Control	2	U	A PROPERTY OF THE PARTY OF THE	2	9
Leading Detector (ft) 20 100 20 100 20 100 20 100 Trailing Detector (ft) 0	Detector Template	Left	Thru						The state of the s			THE RESERVE AND ADDRESS OF THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN COLUMN TWO IS	
Trailing Detector (ft) 0	Leading Detector (ft)										WAS THE RESIDENCE OF THE PARTY		
Turn Type Perm NA Perm NA Perm NA Perm NA Protected Phases 1 1 1 2 3 Permitted Phases 1 1 1 2 2 3 Detector Phase 1 1 1 1 2 2 3 3	Trailing Detector (ft)	THE PARTY OF THE P	WORKS AND DESCRIPTION OF THE PARTY OF THE PA		and the second second			ACCORDING TO THE PARTY OF THE P			The second secon	The second second second second	ACTION (CS
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Permitted Phases 1 1 1 1 2 2 3 3 Detector Phase 1 1 1 1 2 2 3 3			MATERIAL PROPERTY AND ADDRESS OF THE PARTY AND		1 OIIII			I CIIII			rerm	THE RESIDENCE OF THE PARTY OF T	
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\mathcal{L}	Detector Phase	The second secon										A STATE OF THE PARTY OF THE PAR	
	Switch Phase								2		3	3	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	20.0	20.0		20.0	20.0		7.0	7.0		7.0	7.0	
Minimum Split (s)	25.0	25.0		25.0	25.0		12.0	12.0		12.0	12.0	
Total Split (s)	45.0	45.0		45.0	45.0		25.0	25.0		25.0	25.0	
Total Split (%)	64.3%	64.3%	SANCON JAMANAN	64.3%	64.3%		35.7%	35.7%		35.7%	35.7%	
Maximum Green (s)	40.0	40.0		40.0	40.0		20.0	20.0		20.0	20.0	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		2.0	0.0		2.0	0.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0			5.0			5.0	
Lead/Lag				0.0	0.0			0.0			0.0	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Minimum Gap (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Time Before Reduce (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Time To Reduce (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Recall Mode	Max	Max		Max	Max		None	None		None		
Walk Time (s)	Wax	WICK		IVIAX	IVIAA		None	None		None	None	SAME
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	43.9	43.9		43.9	43.9			8.0			0.0	
Actuated g/C Ratio	0.75	0.75		0.75	0.75						8.0	
v/c Ratio	0.10	0.45		0.73	0.73			0.14			0.14	
Control Delay	3.8	5.4		3.4	4.8			0.14			0.38	
Queue Delay	0.0	0.0		0.0	0.0			16.3			17.6	
Total Delay	3.8	5.4		3.4	4.8			0.0			0.0	
LOS	Α	Α		3.4 A	4.0 A			16.3			17.6	
Approach Delay		5.3		A	4.8			В			В	
Approach LOS		J.5						16.3			17.6	
90th %ile Green (s)	40.0	40.0		40.0	A		40.5	B		40.5	В	PER SERVICE SE
90th %ile Term Code	MaxR			40.0	40.0		10.5	10.5		10.5	10.5	
70th %ile Green (s)	40.0	MaxR		MaxR	MaxR		Hold	Hold		Gap	Gap	
70th %ile Term Code		40.0		40.0	40.0		8.4	8.4		8.4	8.4	
50th %ile Green (s)	MaxR 40.0	MaxR		MaxR	MaxR		Hold	Hold		Gap	Gap	
50th %ile Term Code		40.0		40.0	40.0		0.0	0.0		7.0	7.0	
30th %ile Green (s)	MaxR	MaxR		MaxR	MaxR		Skip	Skip		Min	Min	TOTAL AND ADDRESS OF THE PARTY
30th %ile Term Code	40.0	40.0		40.0	40.0		0.0	0.0		7.0	7.0	
	MaxR	MaxR		MaxR	MaxR		Skip	Skip		Min	Min	
10th %ile Green (s) 10th %ile Term Code	55.0	55.0		55.0	55.0		0.0	0.0		0.0	0.0	
	Dwell	Dwell		Dwell	Dwell		Skip	Skip		Skip	Skip	
Intersection Summary												
Area Type:	Other											
Cycle Length: 70												
Actuated Cycle Length: 58	3.6											
Natural Cycle: 40												
Control Type: Semi Act-Ur	ncoord											
Maximum v/c Ratio: 0.45												
Intersection Signal Delay:				In	tersection	LOS: A						un Austra (1996)
Intersection Capacity Utiliz	zation 63.4%				CU Level o		В					
Analysis Period (min) 15												

Existing Weekday PM Peak





	۶	-	*	1	-	*	1	†	~	1	+	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	f)		79	1			4			43-	
Traffic Volume (vph)	42	420	16	15	423	33	9	5	12	37	10	55
Future Volume (vph)	42	420	16	15	423	33	9	5	12	37	10	55
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%		12	0%	14	12	0%	12
Storage Length (ft)	50	0.70	0	60	0 70	0	0	0 /0	0	0	0%	
Storage Lanes	1		0	1		0	0		0	0		0
Taper Length (ft)	25		U	25		U	25		U	25		U
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	4.00
Ped Bike Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.995			0.989			0.937			0.007	ASSESSED IN
Flt Protected	0.950	0.000		0.950	0.303						0.927	
Satd. Flow (prot)	1770	1853	0	1770	1842	0	0	0.982	0	0	0.982	0
Flt Permitted	0.466	1000	U	0.480	1042	U	U	1714	0	0	1696	0
Satd. Flow (perm)	868	1853	0	894	1842	0	Λ	0.893	0	0	0.869	6
Right Turn on Red	000	1000	No	034	1042		0	1559	0	0	1501	0
Satd. Flow (RTOR)			INO			No		10	Yes		00	Yes
Link Speed (mph)		35			25			13			60	
Link Distance (ft)		993			35			25			25	ACRES ASSESSMENT OF THE SECOND CONTRACT
Travel Time (s)		19.3			1384			635			738	
Confl. Peds. (#/hr)		13.5			27.0			17.3		na de la composición	20.1	
Confl. Bikes (#/hr)												
Peak Hour Factor	0.92	0.92	0.92	0.92	0.02	0.00	0.00	0.00	0.00	0.00	0.00	
Growth Factor	100%	100%	100%	100%	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	2%	2%	2%	2%	100%	100%	100%	100%	100%	100%	100%	100%
Bus Blockages (#/hr)	0	0	0		2%	2%	2%	2%	2%	2%	2%	2%
Parking (#/hr)	U	U	U	0	0	0	0	0	0	0	0	0
Mid-Block Traffic (%)		0%			00/			00/				
Adj. Flow (vph)	46	457	17	10	0%	20	40	0%	40		0%	On the second
Shared Lane Traffic (%)	40	401	- 17	16	460	36	10	5	13	40	11	60
Lane Group Flow (vph)	46	474	Λ	10	100	^	^					
Enter Blocked Intersection	No	No	0	16	496	0	0	28	0	0	111	0
Lane Alignment	Left	Left	No	No	No	No	No	No	No	No	No	No
Median Width(ft)	Leit	12	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Link Offset(ft)		0			12			0	line and investor		0	STATE OF STA
Crosswalk Width(ft)		16			0			0			0	
Two way Left Turn Lane		10			16			16			16	CEDEL-RECONSTRUCT
Headway Factor	1.00	1.00	4.00	4.00	4.00	4.00						
Turning Speed (mph)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Number of Detectors		0	9	15		9	15		9	15		9
Detector Template	1	2		1	2		1	2		1	2	
Leading Detector (ft)	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Trailing Detector (ft)	20	100		20	100		20	100		20	100	
Turn Type	0 Dorm	0		0	0		0	0		0	0	
Protected Phases	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
A CONTRACTOR OF THE STATE OF TH		1			1			2			3	
Permitted Phases	1	1		1	1		2	2		3	3	
Detector Phase	1	1		1	1		2	2		3	3	
Switch Phase												

	۶	-	*	1	4-	1	1	†	~	1	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	20.0	20.0		20.0	20.0		7.0	7.0		7.0	7.0	
Minimum Split (s)	25.0	25.0		25.0	25.0		12.0	12.0		12.0	12.0	
Total Split (s)	45.0	45.0		45.0	45.0		25.0	25.0		25.0	25.0	
Total Split (%)	64.3%	64.3%		64.3%	64.3%		35.7%	35.7%		35.7%	35.7%	
Maximum Green (s)	40.0	40.0		40.0	40.0		20.0	20.0		20.0	20.0	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		2.0	0.0		2.0	0.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0			5.0			5.0	
Lead/Lag		0,0		0.0	0.0			5.0			0.0	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		2.0	2.0	
Minimum Gap (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Time Before Reduce (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	3.0	
Time To Reduce (s)	0.0	0.0		0.0	0.0		0.0	0.0			0.0	
Recall Mode	Max	Max		Max	Max		None	None		0.0	0.0	
Walk Time (s)	max	WICK		IVIAA	IVIAX		None	None		None	None	
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	44.0	44.0		44.0	44.0			8.2			0.0	
Actuated g/C Ratio	0.75	0.75		0.75	0.75						8.3	
v/c Ratio	0.07	0.73		0.73	0.75			0.14			0.14	
Control Delay	3.8	4.6		3.5	4.8			0.12			0.43	
Queue Delay	0.0	0.0		0.0	0.0			16.8			17.6	
Total Delay	3.8	4.6		3.5	4.8			0.0			0.0	
LOS	A	Α.		A	4.0 A			16.8			17.6	
Approach Delay	, ,	4.5			4.7			B			В	
Approach LOS		4.5 A			4.7 A			16.8			17.6	
90th %ile Green (s)	40.0	40.0		40.0	40.0		11.0	B		44.0	В	
90th %ile Term Code	MaxR	MaxR		MaxR			11.2	11.2		11.2	11.2	
70th %ile Green (s)	40.0	40.0		40.0	MaxR		Hold	Hold		Gap	Gap	
70th %ile Term Code	MaxR	MaxR			40.0		8.9	8.9		8.9	8.9	
50th %ile Green (s)	40.0	40.0		MaxR	MaxR		Hold	Hold		Gap	Gap	
50th %ile Term Code	MaxR	MaxR		40.0	40.0		0.0	0.0		7.3	7.3	
30th %ile Green (s)	40.0	40.0		MaxR	MaxR		Skip	Skip		Gap	Gap	Militaria de Militaria de Caracia
30th %ile Term Code	MaxR	MaxR		40.0	40.0		0.0	0.0		7.0	7.0	
10th %ile Green (s)	55.0			MaxR	MaxR		Skip	Skip		Min	Min	
10th %ile Term Code	A STATE OF THE PARTY OF THE PAR	55.0 Dwell		55.0	55.0		0.0	0.0		0.0	0.0	
	Dwell	Dwell		Dwell	Dwell		Skip	Skip		Skip	Skip	
Intersection Summary												
Area Type:	Other											
Cycle Length: 70												
Actuated Cycle Length: 58	3.9											
Natural Cycle: 40												
Control Type: Semi Act-Ur	ncoord											
Maximum v/c Ratio: 0.43												
Intersection Signal Delay:				In	tersection	LOS: A						THE PERSON NAMED IN COLUMN
Intersection Capacity Utiliz	ration 51.2%			IC	U Level of	Service	Α					
Analysis Period (min) 15									The second secon			

90th %ile Actuated Cycle: 61.2	
70th %ile Actuated Cycle: 58.9	
50th %ile Actuated Cycle: 57.3	
30th %ile Actuated Cycle: 57	
10th %ile Actuated Cycle: 60	

3: Roosevelt/Liberty & Route 1 Splits and Phases: ↑ ø2

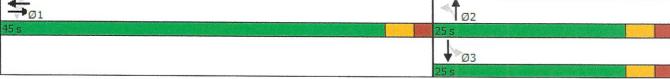
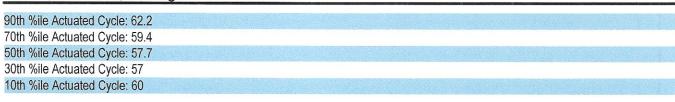
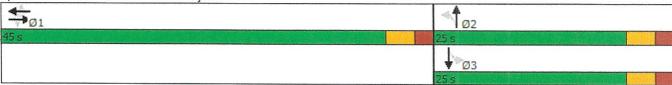


Exhibit 11
Traffic Operations Analyses
Background (no-build) Peak Hour Traffic Volumes
West Main Street (Route 1) at Liberty Street and Roosevelt Avenue
Stamford, Connecticut

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	Ť»		PT.	ĵ.			4			44	
Traffic Volume (vph)	28	285	16	17	415	21	10	3	8	47	7	68
Future Volume (vph)	28	285	16	17	415	21	10	3	8	47	7	68
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%		' <u>-</u>	0%	12
Storage Length (ft)	50		0	60		0	0	• /0	0	0	070	0
Storage Lanes	1		0	1		0	0		0	0		0
Taper Length (ft)	25			25			25		•	25		O
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						.,	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.992			0.993			0.947			0.925	
Flt Protected	0.950			0.950				0.977			0.981	
Satd. Flow (prot)	1770	1848	0	1770	1850	0	0	1723	0	0	1690	0
FIt Permitted	0.479			0.563		•	•	0.838	0	U	0.864	U
Satd. Flow (perm)	892	1848	0	1049	1850	0	0	1478	0	0	1489	0
Right Turn on Red			No			No	•	1110	Yes	U	1400	Yes
Satd. Flow (RTOR)								9	100		74	163
Link Speed (mph)		35			35			25			25	
Link Distance (ft)		993			1384			635			738	
Travel Time (s)		19.3			27.0			17.3			20.1	
Confl. Peds. (#/hr)								17.0			20.1	
Confl. Bikes (#/hr)												
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)										U	U	U
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	30	310	17	18	451	23	11	3	9	51	8	74
Shared Lane Traffic (%)								U	J	01	O	14
Lane Group Flow (vph)	30	327	0	18	474	0	0	23	0	0	133	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	CANADA SANCTONIA
Median Width(ft)		12	9		12	rugiic	Lon	0	ragiit	LCIL	Len 0	Right
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)	HARMA TO A VALUE OF THE STREET	16			16			16			16	
Two way Left Turn Lane								10			10	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15	1.00	9	1.00	1.00	9	1.00	1.00	1.00
Number of Detectors	1	2		1	2	3	1	2	9	- Commence of the Commence of	2	9
Detector Template	Left	Thru		Left	Thru		Left	Thru		1	2 Th	
Leading Detector (ft)	20	100		20	100		20	100		Left	Thru	
Trailing Detector (ft)	0	0		0	0		0	0		20	100	CHARLES
Turn Type	Perm	NA		Perm	NA		AND RESIDENCE OF THE PARTY OF THE PARTY.			0	0	
Protected Phases	. 01111	1		1 CIIII	1		Perm	NA		Perm	NA	
Permitted Phases	1	1		1	1		2	2		_	3	
Detector Phase	1	1		1	1		2	2		3	3	
Switch Phase	•	1		1	ı		Z	Z		3	3	

	→	-	*	•	-	*	1	†	1	1	+	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	20.0	20.0		20.0	20.0		7.0	7.0		7.0	7.0	
Minimum Split (s)	25.0	25.0		25.0	25.0		12.0	12.0		12.0	12.0	
Total Split (s)	45.0	45.0		45.0	45.0		25.0	25.0		25.0	25.0	
Total Split (%)	64.3%	64.3%		64.3%	64.3%		35.7%	35.7%		35.7%	35.7%	
Maximum Green (s)	40.0	40.0		40.0	40.0		20.0	20.0		20.0	20.0	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		2.0	0.0		2.0	0.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0			5.0			5.0	
Lead/Lag								0.0			0.0	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	294-1257/902-03473-0	3.0	3.0		3.0	3.0	S DOMESTICS	3.0	3.0	
Minimum Gap (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Time Before Reduce (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Time To Reduce (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Recall Mode	Max	Max		Max	Max		None	None		None	None	
Walk Time (s)					Mux		140110	TVOITE		INOILE	INOUG	
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	44.0	44.0		44.0	44.0			8.0			8.6	
Actuated g/C Ratio	0.74	0.74		0.74	0.74			0.13			0.15	
v/c Ratio	0.05	0.24		0.02	0.35			0.13			0.13	
Control Delay	3.8	4.2		3.7	4.8			18.3			17.7	
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	
Total Delay	3.8	4.2		3.7	4.8			18.3			17.7	
LOS	Α	Α		A	А			В			В	
Approach Delay		4.2			4.8			18.3			17.7	
Approach LOS		Α			А			В			В	
90th %ile Green (s)	40.0	40.0		40.0	40.0		12.2	12.2		12.2	12.2	
90th %ile Term Code	MaxR	MaxR		MaxR	MaxR		Hold	Hold		Gap	Gap	
70th %ile Green (s)	40.0	40.0		40.0	40.0		0.0	0.0		9.4	9.4	
70th %ile Term Code	MaxR	MaxR		MaxR	MaxR		Skip	Skip		Gap	Gap	
50th %ile Green (s)	40.0	40.0		40.0	40.0		0.0	0.0		7.7	7.7	
50th %ile Term Code	MaxR	MaxR		MaxR	MaxR		Skip	Skip		Gap	NATIONAL PROPERTY OF THE PROPE	
30th %ile Green (s)	40.0	40.0		40.0	40.0		0.0	0.0		7.0	Gap 7.0	
30th %ile Term Code	MaxR	MaxR		MaxR	MaxR		Skip	Skip		Min	Min	
10th %ile Green (s)	55.0	55.0		55.0	55.0		0.0	0.0		0.0	0.0	
10th %ile Term Code	Dwell	Dwell		Dwell	Dwell		Skip	Skip		Skip	Skip	
Intersection Summary								,		p	O.up	
Area Type:	Other											
Cycle Length: 70												
Actuated Cycle Length: 59.3												
Natural Cycle: 40												
Control Type: Semi Act-Unco	oord			Marie Algert Lake Cons								
Maximum v/c Ratio: 0.48												
Intersection Signal Delay: 6.6	3			ln	tersection	LOS: A						
MANUFACTURE AND ASSESSMENT OF THE PARTY OF T				Charles and the second	CU Level o		٨					
Intersection Capacity Utilizat	1011 40.076				U Level o	Service	A					





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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*1	72		7	p			4			4	
Traffic Volume (vph)	59	588	18	15	465	48	7	7	16	30	12	48
Future Volume (vph)	59	588	18	15	465	48	7	7	16	30	12	48
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	50		0	60		0	0		0	0		0
Storage Lanes	1		0	1		0	0		0	0		0
Taper Length (ft)	25			25		SERVICE CONTRACTOR	25		and the second	25		ATTRACTOR STOP
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.995			0.986			0.930			0.928	
Flt Protected	0.950			0.950				0.988			0.983	
Satd. Flow (prot)	1770	1853	0	1770	1837	0	0	1712	0	0	1699	0
Flt Permitted	0.429			0.370				0.927	•	<u> </u>	0.875	The second second
Satd. Flow (perm)	799	1853	0	689	1837	0	0	1606	0	0	1513	0
Right Turn on Red			No		.007	No	· ·	1000	Yes	· ·	1010	Yes
Satd. Flow (RTOR)								17	100		52	100
Link Speed (mph)		35			35			25			25	
Link Distance (ft)		993			1384			635			738	
Travel Time (s)		19.3			27.0			17.3			20.1	
Confl. Peds. (#/hr)											20.1	
Confl. Bikes (#/hr)												
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	64	639	20	16	505	52	8	8	17	33	13	52
Shared Lane Traffic (%)						_	_	•	••			ÜL
Lane Group Flow (vph)	64	659	0	16	557	0	0	33	0	0	98	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12	9		12	1 (19)	20.1	0	rugitt	Lon	0	rugite
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	,,,,,	9	15	1.00	9	15	1.00	9	15	1.00	9
Number of Detectors	1	2		1	2	J	1	2	0	1	2	0
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases	1 01111	1		ı GIIII	1		ı Cilli	2		r em	3	
Permitted Phases	1	1		1	1		2	2		3	3	
Detector Phase	1	1		1	1		2	2		3	3	
Switch Phase	1	ı		ı	1			Z		J	3	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	20.0	20.0		20.0	20.0		7.0	7.0		7.0	7.0	
Minimum Split (s)	25.0	25.0		25.0	25.0	i, esit esit esit e	12.0	12.0		12.0	12.0	
Total Split (s)	45.0	45.0		45.0	45.0		25.0	25.0		25.0	25.0	
Total Split (%)	64.3%	64.3%		64.3%	64.3%		35.7%	35.7%		35.7%	35.7%	
Maximum Green (s)	40.0	40.0		40.0	40.0		20.0	20.0		20.0	20.0	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		_,0	0.0		2.0	0.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0			5.0			5.0	
Lead/Lag								0.0			0.0	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Minimum Gap (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Time Before Reduce (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Time To Reduce (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Recall Mode	Max	Max		Max	Max		None	None		None	None	
Walk Time (s)	,,,,,,	HIGA		IVIUX	IVIAX		TAOHE	NONE		None	NONE	
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	44.0	44.0		44.0	44.0			8.0			8.0	
Actuated g/C Ratio	0.75	0.75		0.75	0.75			0.14			0.14	
v/c Ratio	0.11	0.47		0.03	0.40			0.14			0.14	
Control Delay	3.9	5.6		3.5	5.0			16.1			17.6	
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	
Total Delay	3.9	5.6		3.5	5.0			16.1			17.6	
LOS	A	A		Α	Α			В			17.0 B	
Approach Delay		5.5			5.0			16.1			17.6	
Approach LOS		A			Α			В			17.0 B	
90th %ile Green (s)	40.0	40.0		40.0	40.0		10.7	10.7		10.7	10.7	
90th %ile Term Code	MaxR	MaxR		MaxR	MaxR		Hold	Hold		Gap	Gap	
70th %ile Green (s)	40.0	40.0		40.0	40.0		8.5	8.5		8.5	8.5	
70th %ile Term Code	MaxR	MaxR		MaxR	MaxR		Hold	Hold		Gap		
50th %ile Green (s)	40.0	40.0		40.0	40.0		0.0	0.0		7.1	Gap 7.1	
50th %ile Term Code	MaxR	MaxR		MaxR	MaxR		Skip	Skip		CONTRACTOR DESIGNATION OF	AND DESCRIPTION OF THE PARTY OF	
30th %ile Green (s)	40.0	40.0		40.0	40.0		0.0	0.0		Gap 7.0	Gap	
30th %ile Term Code	MaxR	MaxR		MaxR	MaxR		Skip	Skip			7.0	
10th %ile Green (s)	55.0	55.0		55.0	55.0		0.0	0.0		Min 0.0	Min	SWEETERS F
10th %ile Term Code	Dwell	Dwell		Dwell	Dwell		Skip	Skip			0.0	
	B WOII	DWCII		DWGII	DWGII		Skih	Skip		Skip	Skip	
Intersection Summary Area Type:	Other											
Cycle Length: 70	Outer											
Actuated Cycle Length: 58.	7											
Natural Cycle: 40	<i>1</i>											
Control Type: Semi Act-Uni	ooord											
Maximum v/c Ratio: 0.47	COOIU											
Intersection Signal Delay: 6	2.4					1.00.4						
					tersection							T-2-7-12-12-12-12-12-12-12-12-12-12-12-12-12-
Intersection Capacity Utiliza	%d.co 110115			10	CU Level o	f Service	C					
Analysis Period (min) 15												

90th %ile Actuated Cycle: 60.7
70th %ile Actuated Cycle: 58.5
50th %ile Actuated Cycle: 57.1
30th %ile Actuated Cycle: 57
10th %ile Actuated Cycle: 60

Splits and Phases: 3: Roosevelt/Liberty & Route 1

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	ĵ»		7	ĵ»			44			4	
Traffic Volume (vph)	44	437	17	16	440	34	9	5	12	38	10	57
Future Volume (vph)	44	437	17	16	440	34	9	5	12	38	10	57
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)	· <u>-</u>	0%	. <u>-</u>		0%	· <u>-</u>	· <u>-</u>	0%			0%	
Storage Length (ft)	50	0,0	0	60		0	0		0	0		0
Storage Lanes	1		0	1		0	0		0	0		0
Taper Length (ft)	25		•	25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.995			0.989			0.937			0.927	
Fit Protected	0.950	0.000		0.950	0.000			0.982			0.982	
Satd. Flow (prot)	1770	1853	0	1770	1842	0	0	1714	0	0	1696	0
Flt Permitted	0.454	1000	U	0.468	1012	V	U	0.891	U	U	0.870	J
Satd. Flow (perm)	846	1853	0	872	1842	0	0	1555	0	0	1502	0
Right Turn on Red	010	1000	No	012	1012	No	•	1000	Yes		1002	Yes
Satd. Flow (RTOR)			110			110		13	103		62	103
Link Speed (mph)		35			35			25			25	
Link Distance (ft)		993			1384			635			738	
Travel Time (s)		19.3			27.0			17.3			20.1	
Confl. Peds. (#/hr)		10.0			21.0			11.0			2.0.1	
Confl. Bikes (#/hr)												
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)	J	Ü							U	Ü	0	Ü
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	48	475	18	17	478	37	10	5	13	41	11	62
Shared Lane Traffic (%)	-10	410	10	1.1	710	01	10	0	10	71	11	02
Lane Group Flow (vph)	48	493	0	17	515	0	0	28	0	0	114	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)	Lon	12	ragin	LUIT	12	right	Leit	0	rigit	LGIL	0	Night
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		10			10			10			10	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Number of Detectors	13	2	9	1	2	9	13	2	9	10	2	9
Detector Template	Left	Thru		Left	Thru		Left					
Leading Detector (ft)	20	100		20	100			Thru 100		Left	Thru	
Trailing Detector (ft)	PRODUCTION OF THE PROPERTY OF	NAME AND ADDRESS OF THE PARTY O					20	THE RESERVE THE PARTY AND ADDRESS OF THE PARTY		20	100	
Turn Type	0 Dorm	0		Dorm.	0		O Dorm	0		0	0	
PRODUCTION OF THE PRODUCTION O	Perm	NA 1		Perm	NA 1		Perm	NA		Perm	NA	
Protected Phases	A	1		4	1		^	2		0	3	
Permitted Phases	1	1		T	1		2	2		3	3	
Detector Phase Switch Phase	1	1		1	1		2	2		3	3	

	۶	→	*	1	4-	4	1	†	-	1	+	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	20.0	20.0		20.0	20.0		7.0	7.0		7.0	7.0	ODIT
Minimum Split (s)	25.0	25.0		25.0	25.0		12.0	12.0		12.0	12.0	
Total Split (s)	45.0	45.0		45.0	45.0		25.0	25.0		25.0	25.0	
Total Split (%)	64.3%	64.3%		64.3%	64.3%		35.7%	35.7%		35.7%	35.7%	
Maximum Green (s)	40.0	40.0		40.0	40.0		20.0	20.0		20.0	20.0	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0				
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		3.0	3.0	CET-PRINCIPLE
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		2.0	0.0		2.0	2.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0			5.0			0.0	
Lead/Lag	0.0	0.0		3.0	3.0			5.0			5.0	
Lead-Lag Optimize?				ASSESSED AND ADDRESSED AND ADD								
Vehicle Extension (s)	3.0	3.0		3.0	3.0		2.0	2.0		0.0	0.0	
Minimum Gap (s)	3.0	3.0					3.0	3.0		3.0	3.0	
Time Before Reduce (s)	0.0	0.0		3.0	3.0		3.0	3.0		3.0	3.0	
Time To Reduce (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Recall Mode	where contrating and a feet of the	energy and productive and designation		0.0	0.0		0.0	0.0		0.0	0.0	
Walk Time (s)	Max	Max		Max	Max		None	None		None	None	
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
The second secon	44.0	44.0		44.0	44.0							
Act Effct Green (s)	44.0	44.0		44.0	44.0			8.2			8.3	Augustonianen
Actuated g/C Ratio	0.75	0.75		0.75	0.75			0.14			0.14	
v/c Ratio	0.08	0.36		0.03	0.37			0.12			0.43	
Control Delay	3.8	4.7		3.6	4.9			16.7			17.6	
Queue Delay	0.0	0.0		0.0	0.0		HATCHIS COLONIA COM	0.0			0.0	
Total Delay	3.8	4.7		3.6	4.9			16.7			17.6	
LOS	Α	Α		Α	Α			В			В	
Approach Delay		4.7			4.8			16.7			17.6	
Approach LOS		Α		CANCEL S Manufacture	Α			В			В	
90th %ile Green (s)	40.0	40.0		40.0	40.0		11.3	11.3		11.3	11.3	
90th %ile Term Code	MaxR	MaxR		MaxR	MaxR		Hold	Hold		Gap	Gap	
70th %ile Green (s)	40.0	40.0		40.0	40.0		8.9	8.9		8.9	8.9	
70th %ile Term Code	MaxR	MaxR		MaxR	MaxR		Hold	Hold		Gap	Gap	rustum et la july rusting.
50th %ile Green (s)	40.0	40.0		40.0	40.0		0.0	0.0		7.3	7.3	
50th %ile Term Code	MaxR	MaxR		MaxR	MaxR		Skip	Skip		Gap	Gap	
30th %ile Green (s)	40.0	40.0		40.0	40.0		0.0	0.0		7.0	7.0	
30th %ile Term Code	MaxR	MaxR		MaxR	MaxR		Skip	Skip		Min	Min	NAME OF THE OWNER, WHICH PARTY AND P
10th %ile Green (s)	55.0	55.0		55.0	55.0		0.0	0.0		0.0	0.0	
10th %ile Term Code	Dwell	Dwell		Dwell	Dwell		Skip	Skip		Skip	Skip	
Intersection Summary												
Area Type:	Other					40 less 4 700 min de la constante de la consta						-
Cycle Length: 70												
Actuated Cycle Length: 58.	9											
Natural Cycle: 40												
Control Type: Semi Act-Und	coord											
Maximum v/c Ratio: 0.43												
Intersection Signal Delay: 6	5.2			In	tersection	LOS: A		A constant of the second				
Intersection Capacity Utiliza				CONTRACTOR DE LA CONTRACTOR DE	CU Level o		Α					
Analysis Period (min) 15					3 23 70 10	. 00/1/00						

90th %ile Actuated Cycle: 61.3 70th %ile Actuated Cycle: 58.9 50th %ile Actuated Cycle: 57.3 30th %ile Actuated Cycle: 57 10th %ile Actuated Cycle: 60 Background Saturday Midday Peak

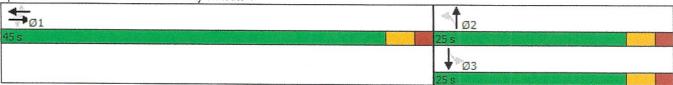


Exhibit 12
Traffic Operations Analyses
Combined (build) Peak Hour Traffic Volumes
West Main Street (Route 1) at Liberty Street and Roosevelt Avenue
Stamford, Connecticut

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	B		T	f)			4			43	
Traffic Volume (vph)	28	290	16	17	420	21	10	3	8	47	7	68
Future Volume (vph)	28	290	16	17	420	21	10	3	8	47	7	68
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	50		0	60		0	0		0	0	0,70	0
Storage Lanes	1		0	1		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor									1.00		1.00	1.00
Frt		0.992			0.993			0.947			0.925	
Flt Protected	0.950		A THE PARTY OF LAND DO	0.950				0.977			0.981	
Satd. Flow (prot)	1770	1848	0	1770	1850	0	0	1723	0	0	1690	0
Flt Permitted	0.475			0.560		_	· ·	0.838		•	0.864	U
Satd. Flow (perm)	885	1848	0	1043	1850	0	0	1478	0	0	1489	0
Right Turn on Red			No			No		1110	Yes	_	1400	Yes
Satd. Flow (RTOR)								9	100		74	103
Link Speed (mph)		35			35			25			25	
Link Distance (ft)		993			1384			635			738	
Travel Time (s)		19.3			27.0			17.3			20.1	
Confl. Peds. (#/hr)								17.0			20.1	
Confl. Bikes (#/hr)												
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												Ü
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	30	315	17	18	457	23	11	3	9	51	8	74
Shared Lane Traffic (%)	COLUMN WATER STREET, COLUMN			Control Control					Ü	01	U	/ 7
Lane Group Flow (vph)	30	332	0	18	480	0	0	23	0	0	133	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12	0		12			0	rugin	Loit	0	ragin
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)	and the past of course	16		and the Andreas	16			16			16	
Two way Left Turn Lane											10	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15	1.00	9	15	1.00	9	1.00	1.00	9
Number of Detectors	1	2		1	2		1	2	J	1	2	J
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0		
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	0	
Protected Phases	. 51111	1		1 01111	1		1 01111	2		rellli	NA	
Permitted Phases	1	1		1	1		2	2		2	3	
Detector Phase	1	1		1	1		2	2		3	3	
Switch Phase	1	1		- 1	1			2		3	3	
torr ridge												

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	20.0	20.0		20.0	20.0		7.0	7.0		7.0	7.0	
Minimum Split (s)	25.0	25.0		25.0	25.0		12.0	12.0		12.0	12.0	
Total Split (s)	45.0	45.0		45.0	45.0		25.0	25.0		25.0	25.0	
Total Split (%)	64.3%	64.3%		64.3%	64.3%		35.7%	35.7%		35.7%	35.7%	
Maximum Green (s)	40.0	40.0		40.0	40.0		20.0	20.0		20.0	20.0	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0			0.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0			5.0			5.0	
Lead/Lag								0.0			0.0	
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	personal control	3.0	3.0		3.0	3.0		3.0	3.0	
Minimum Gap (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Time Before Reduce (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Time To Reduce (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Recall Mode	Max	Max		Max	Max		None	None		None	None	
Walk Time (s)	1100	WICE/C		WIGA	WIGA		140110	INOIIG		NONE	INOILE	
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	44.0	44.0		44.0	44.0			8.0			8.6	
Actuated g/C Ratio	0.74	0.74		0.74	0.74			0.13			0.15	
v/c Ratio	0.05	0.24		0.02	0.35			0.13			0.13	
Control Delay	3.9	4.2		3.7	4.9			18.3			17.7	
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	
Total Delay	3.9	4.2		3.7	4.9			18.3			17.7	
LOS	A	Α		Α	Α.			В			В	
Approach Delay		4.2			4.8			18.3			17.7	
Approach LOS		A			Α.			В			В	
90th %ile Green (s)	40.0	40.0		40.0	40.0		12.2	12.2		12.2	12.2	
90th %ile Term Code	MaxR	MaxR		MaxR	MaxR		Hold	Hold		Gap	Species States (States States States	
70th %ile Green (s)	40.0	40.0		40.0	40.0		0.0	0.0		9.4	Gap 9.4	
70th %ile Term Code	MaxR	MaxR	EDEDGE SANS	MaxR	MaxR		Skip	Skip			SECURIOR SE	
50th %ile Green (s)	40.0	40.0		40.0	40.0		0.0	0.0		Gap 7.7	Gap	
50th %ile Term Code	MaxR	MaxR		MaxR	MaxR		Skip	Skip		The second secon	7.7 Con	
30th %ile Green (s)	40.0	40.0		40.0	40.0		0.0	0.0		Gap	Gap	9800000
30th %ile Term Code	MaxR	MaxR		MaxR	MaxR					7.0	7.0	
10th %ile Green (s)	55.0	55.0		55.0	55.0		Skip	Skip		Min	Min	
10th %ile Term Code	Dwell	Dwell		Dwell	Dwell		0.0 Skip	0.0 Skip		0.0 Skip	0.0 Skip	
Intersection Summary					J.,		Onip	ONIP		Окір	Окір	
	Other											
Cycle Length: 70												
Actuated Cycle Length: 59.3	3											
Natural Cycle: 40												
Control Type: Semi Act-Unc	coord											
Maximum v/c Ratio: 0.48	Jooru									DELINER STREET		
Intersection Signal Delay: 6.	6			le	itersection	100.4						
Intersection Capacity Utiliza					NAME OF TAXABLE PARTY.		٨					
Analysis Period (min) 15	111011 4U. 1 /0			10	CU Level o	o service	: A					

90th %ile Actuated Cycle: 62.2	
70th %ile Actuated Cycle: 59.4	
50th %ile Actuated Cycle: 57.7	
30th %ile Actuated Cycle: 57	
10th %ile Actuated Cycle: 60	
10th %ile Actuated Cycle: 60	



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	^		Ť	ħ			4			4	
Traffic Volume (vph)	59	600	18	15	477	48	7	7	16	30	12	48
Future Volume (vph)	59	600	18	15	477	48	7	7	16	30	12	48
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	50		0	60		0	0		0	0		0
Storage Lanes	1		0	1		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.996			0.986			0.930			0.928	
Flt Protected	0.950			0.950				0.988			0.983	
Satd. Flow (prot)	1770	1855	0	1770	1837	0	0	1712	0	0	1699	0
Flt Permitted	0.421			0.362				0.927			0.875	
Satd. Flow (perm)	784	1855	0	674	1837	0	0	1606	0	0	1513	0
Right Turn on Red			No			No			Yes		10.10	Yes
Satd. Flow (RTOR)								17			52	. 00
Link Speed (mph)		35		the state of the s	35			25			25	
Link Distance (ft)		993			1384			635			738	
Travel Time (s)		19.3			27.0			17.3			20.1	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												Party Broken
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	64	652	20	16	518	52	8	8	17	33	13	52
Shared Lane Traffic (%)												
Lane Group Flow (vph)	64	672	0	16	570	0	0	33	0	0	98	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)	***************************************	12			12	3		0	9		0	i ugin
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16	named Artists and Carried States		16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15	1.00	9
Number of Detectors	1	2		1	2		1	2		1	2	· ·
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100		20	100		20	100	ALCO COLLEGE
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		1			1		. 51111	2		1 01111	3	
Permitted Phases	1	1		1	1		2	2		3	3	
Detector Phase	1	1		1	1		2	2		3	3	
Switch Phase								_		J	J	

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EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
20.0	20.0		20.0	20.0		7.0					
45.0	45.0										
											4.2-20/02/50/5
	and the second s										
			SAME THE PARTY OF			2.0			2.0		
	it should not be a second		0.0	0.0			0.0			0.0	
3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
										The state of the s	
									The Party of the P	Committee of the party of the committee	
WICK	WIGA		IVIUX	IVIAA		NONE	INOILE		NOTIC	None	
Sate in Edition											
44 0	44 0		44.0	44.0			9.0			0.0	
											Merchanis
40.0			40.0			10.7			10.7		
				PERSONAL PROPERTY.					The State of		
	The same of the sa										
	THE REPORT OF THE PERSON NAMED IN									CANADA SERVICIO DE LA COMPANIO	
CONTRACTOR OF THE PARTY OF	Name and Address of the Owner, where						PARAMETER STATES OF THE STATES		A STATE OF THE PARTY OF THE PAR		
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DMCII	DWell		Dwell	Dwell		Экір	БКІР		Skip	Skip	
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illei									NT AND DESCRIPTION OF THE REAL PROPERTY.		
ora											SANKER OF THE PARTY OF THE PART
של.כט ווט			10	JU Level o	t Service	e C					
	EBL 20.0 25.0 45.0 64.3% 40.0 3.0 2.0 0.0 5.0 3.0 0.0 0.0 Max 44.0 0.75 0.11 3.9 0.0 3.9 A 40.0 MaxR 40.0 MaxR 40.0 MaxR 40.0 MaxR 55.0 Dwell Other	EBL EBT 20.0 20.0 25.0 25.0 45.0 45.0 64.3% 64.3% 40.0 40.0 3.0 3.0 2.0 2.0 0.0 0.0 5.0 5.0 3.0 3.0 3.0 3.0 0.0 0.0 0.0 0.0 Max Max 44.0 44.0 0.75 0.75 0.11 0.48 3.9 5.7 0.0 0.0 3.9 5.7 A A 5.6 A 40.0 40.0 MaxR MaxR 55.0 55.0 Dwell Dwell	EBL EBT EBR 20.0 20.0 25.0 25.0 45.0 45.0 64.3% 64.3% 40.0 40.0 3.0 3.0 2.0 2.0 0.0 0.0 5.0 5.0 3.0 3.0 3.0 3.0 0.0 0.0 0.0 0.0 Max Max 44.0 44.0 0.75 0.75 0.11 0.48 3.9 5.7 0.0 0.0 3.9 5.7 A A 5.6 A 40.0 40.0 MaxR MaxR 55.0 55.0 Dwell Dwell	EBL EBT EBR WBL 20.0 20.0 20.0 20.0 25.0 25.0 25.0 45.0 45.0 45.0 45.0 45.0 64.3% 40.0 40.0 40.0 3.0 3.0 2.0 2.0 2.0 2.0 0.0 0.0 0.0 0.0 5.0 5.0 5.0 5.0 3.0 0.0 0.0 0.0 0.0 Max Max Max Max 44.0 44.0 44.0 44.0 0.75 0.75 0.75 0.11 0.48 0.03 3.9 5.7 3.5 0.0 0.0 0.0 0.0 3.9 5.7 3.5 A A A A A 5.6 A 40.0 40.0 40.0 MaxR MaxR MaxR 40.0 40.0 40.0 M	EBL EBT EBR WBL WBT 20.0 20.0 20.0 20.0 25.0 25.0 25.0 45.0 45.0 45.0 45.0 45.0 64.3% 64.	EBL EBT EBR WBL WBT WBR 20.0 20.0 20.0 20.0 20.0 25.0 25.0 25.0 25.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 4	EBL EBT EBR WBL WBT WBR NBL 20.0 20.0 20.0 20.0 20.0 7.0 25.0 25.0 25.0 25.0 25.0 12.0 45.0 45.0 45.0 45.0 45.0 25.0 64.3% 64.3% 64.3% 64.3% 35.7% 40.0 40.0 40.0 40.0 20.0 3.0 3.0 3.0 3.0 3.0 3.0 2.0 2.0 2.0 2.0 2.0 2.0 0.0 0.0 0.0 0.0 0.0 5.0 5.0 5.0 5.0 5.0 3.0 3	EBL EBT EBR WBL WBT WBR NBL NBT	BBL BBT BBR WBL WBT WBR NBT NBR	EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL	EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT 20.0 20.0 20.0 20.0 7.0 7.0 7.0 7.0 7.0 25.0 25.0 25.0 25.0 12.0 12.0 12.0 12.0 45.0 45.0 45.0 45.0 25.0 25.0 25.0 25.0 25.0 64.3% 64.3% 64.3% 64.3% 35.7% 35.7% 35.7% 35.7% 40.0 40.0 40.0 40.0 20.0 20.0 20.0 20.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0

90th %ile Actuated Cycle: 60.7	
70th %ile Actuated Cycle: 58.5	
50th %ile Actuated Cycle: 57.1	
30th %ile Actuated Cycle: 57	
10th %ile Actuated Cycle: 60	

Splits and Phases: 3: Roosevelt/Liberty & Route 1



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	B		7	F)			4			4	
Traffic Volume (vph)	44	455	17	16	458	34	9	5	12	38	10	57
Future Volume (vph)	44	455	17	16	458	34	9	5	12	38	10	57
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%	· -		0%	12
Storage Length (ft)	50		0	60		0	0	0,0	0	0	070	0
Storage Lanes	1		0	1		0	0		0	0		0
Taper Length (ft)	25			25			25		J	25		O.
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.995			0.990			0.937			0.927	
Flt Protected	0.950	0,000		0.950	0.000			0.982			0.982	MARKS
Satd. Flow (prot)	1770	1853	0	1770	1844	0	0	1714	0	0	1696	Λ
Flt Permitted	0.442	1000	U	0.455	דדטו	U	U	0.891	U	U	0.870	0
Satd. Flow (perm)	823	1853	0	848	1844	0	0	1555	Λ	Λ		0
Right Turn on Red	020	1000	No	040	1044	No	U	1000	0 Yes	0	1502	0
Satd. Flow (RTOR)			INO			INO		13	res		00	Yes
Link Speed (mph)		35			35						62	
Link Distance (ft)		993			1384			25			25	
Travel Time (s)		19.3						635			738	
Confl. Peds. (#/hr)		19.3			27.0			17.3			20.1	Ordersal views worse
Confl. Bikes (#/hr)												
Peak Hour Factor	0.92	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Growth Factor	100%	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)		100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)		00/			00/							
Mid-Block Traffic (%)	40	0%	40		0%			0%			0%	TANK TANK TANK
Adj. Flow (vph)	48	495	18	17	498	37	10	5	13	41	11	62
Shared Lane Traffic (%) Lane Group Flow (vph)	40	E40	^									PARAMATERIA IN TANA
Enter Blocked Intersection	48	513	0	17	535	0	0	28	0	0	114	0
	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0	Name and Address of the Owner, when the Owner,		0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16		William Start Countries	16			16			16	
Two way Left Turn Lane	4.00											
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Turn Type	Perm	NA		Perm	NA		Perm	NA	100000000000000000000000000000000000000	Perm	NA	
Protected Phases		1			1			2			3	
Permitted Phases	1	1		1	1		2	2		3	3	Acades provide the Colonial Colonia Colonial Colonial Col
Detector Phase	1	1		1	1		2	2		3	3	
Switch Phase											and the same of th	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	20.0	20.0		20.0	20.0		7.0	7.0		7.0	7.0	
Minimum Split (s)	25.0	25.0		25.0	25.0		12.0	12.0		12.0	12.0	
Total Split (s)	45.0	45.0		45.0	45.0		25.0	25.0		25.0	25.0	
Total Split (%)	64.3%	64.3%		64.3%	64.3%		35.7%	35.7%	63/414-3780-06-38-8	35.7%	35.7%	
Maximum Green (s)	40.0	40.0		40.0	40.0		20.0	20.0		20.0	20.0	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0			0.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0			5.0			5.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Minimum Gap (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Time Before Reduce (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Time To Reduce (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Recall Mode	Max	Max		Max	Max		None	None		None	None	
Walk Time (s)				Max	max		140110	140110		INOTIC	TAOHC	
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	44.0	44.0		44.0	44.0			8.2			8.3	
Actuated g/C Ratio	0.75	0.75		0.75	0.75			0.14			0.14	
v/c Ratio	0.08	0.37		0.03	0.39			0.14			0.43	
Control Delay	3.9	4.8		3.6	5.0			16.7			17.6	
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	
Total Delay	3.9	4.8		3.6	5.0			16.7			17.6	
LOS	А	А		A	A			В			В	
Approach Delay		4.8			4.9			16.7			17.6	
Approach LOS		Α			A			В			В	
90th %ile Green (s)	40.0	40.0		40.0	40.0		11.3	11.3		11.3	11.3	
90th %ile Term Code	MaxR	MaxR		MaxR	MaxR		Hold	Hold		Gap	Gap	
70th %ile Green (s)	40.0	40.0		40.0	40.0		8.9	8.9		8.9	8.9	
70th %ile Term Code	MaxR	MaxR		MaxR	MaxR		Hold	Hold		Gap	Gap	
50th %ile Green (s)	40.0	40.0		40.0	40.0		0.0	0.0		7.3	7.3	
50th %ile Term Code	MaxR	MaxR		MaxR	MaxR		Skip	Skip		Gap	Gap	
30th %ile Green (s)	40.0	40.0		40.0	40.0		0.0	0.0		7.0	7.0	
30th %ile Term Code	MaxR	MaxR		MaxR	MaxR		Skip	Skip		Min	Min	
10th %ile Green (s)	55.0	55.0		55.0	55.0		0.0	0.0		0.0	0.0	
10th %ile Term Code	Dwell	Dwell		Dwell	Dwell		Skip	Skip		Skip	Skip	
Intersection Summary				-	2 11011		Onip	Onip		Okip	OKIP	
Area Type:	Other											
Cycle Length: 70	Otrioi											
Actuated Cycle Length: 58	3 9											
Natural Cycle: 40	7.0											
Control Type: Semi Act-Ui	ncoord											
Maximum v/c Ratio: 0.43	iooord											
Intersection Signal Delay:	63			l.	torocati-	100.4						
Intersection Capacity Utiliz				CHARLEST A CONTRACTOR OF THE	ntersection		٨					
Analysis Period (min) 15	zalion 33.1%			10	CU Level of	Service	9 A					
maiyələ Fellou (IIIIII) 13												

90th %ile Actuated Cycle: 61.3 70th %ile Actuated Cycle: 58.9 50th %ile Actuated Cycle: 57.3 30th %ile Actuated Cycle: 57	
50th %ile Actuated Cycle: 57.3	
	ESCAPE SECTION S
10th %ile Actuated Cycle: 60	

Splits and Phases: 3: Roosevelt/Liberty & Route 1

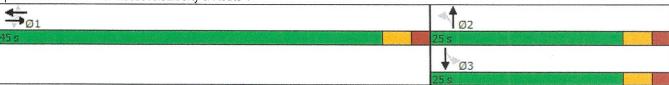
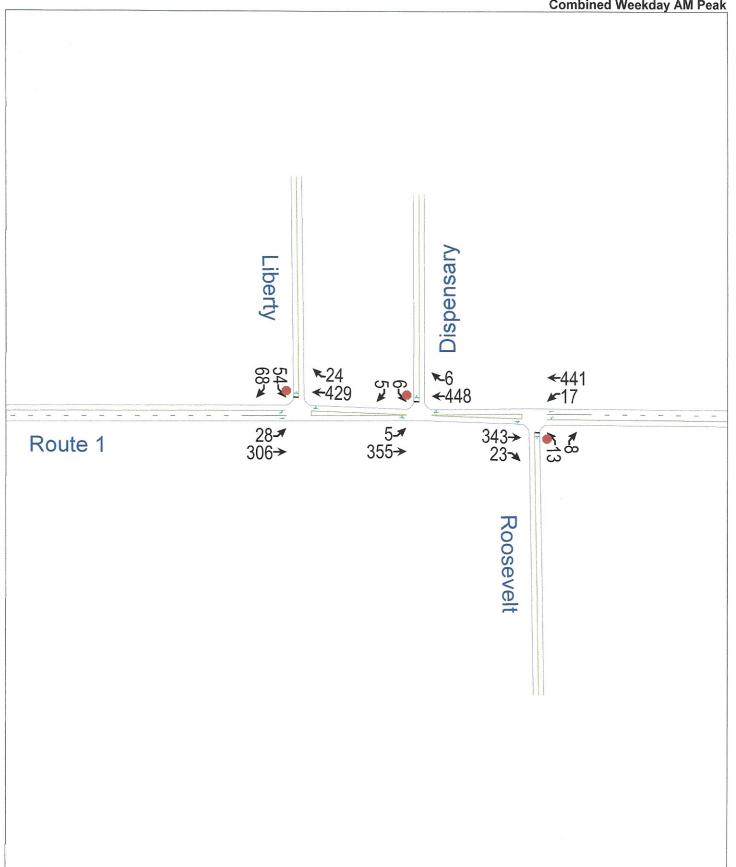


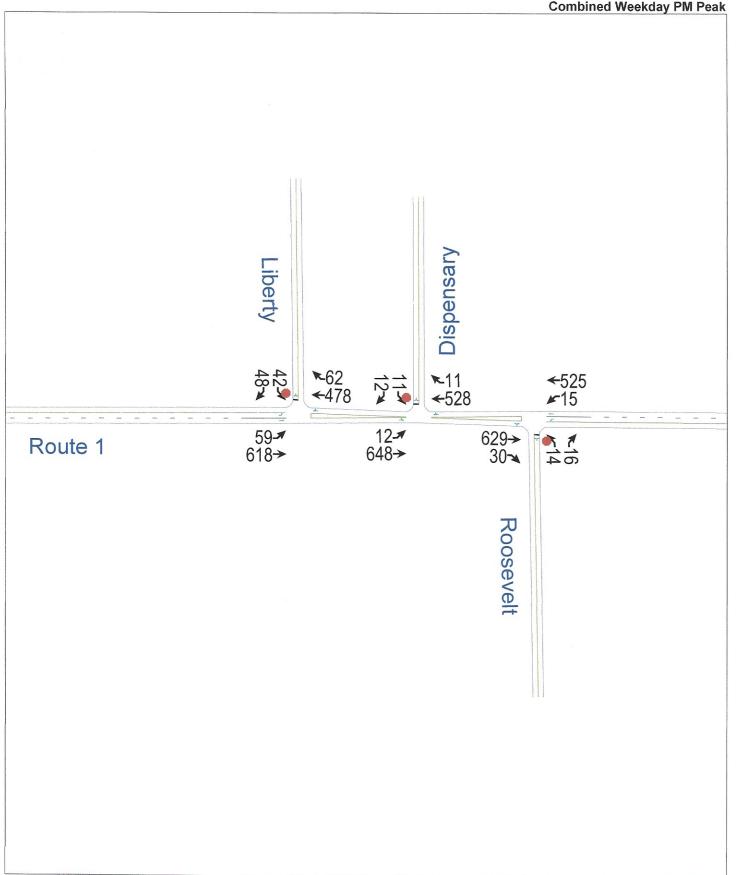
Exhibit 13
Traffic Operations Analyses
Combined (build) Peak Hour Traffic Volumes
West Main Street (Route 1) at Dispensary Site Drive
Stamford, Connecticut



Intersection						
Int Delay, s/veh	2.6					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	'n	†	7.		W	
Traffic Vol, veh/h	28	306	429	24	54	68
Future Vol, veh/h	28	306	429	24	54	68
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	Clop	CONTRACTOR DESCRIPTION OF
Storage Length	0	-	_	-	0	-
Veh in Median Storage,		0	0	_	0	_
Grade, %	_	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	30	333	466	26	59	74
IVIVIIICI IOVV	30	333	400	20	59	14
	/lajor1	D	Major2		Minor2	
Conflicting Flow All	492	0	-	0	872	479
Stage 1	-	-	-	-	479	_
Stage 2	-	-	-	-	393	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	_	5.42	_
Critical Hdwy Stg 2	-	-	-	-	5.42	_
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1071	<u>-</u>	-	_	321	587
Stage 1	-	-	-	-	623	-
Stage 2	-	-	-	_	682	_
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1071	_	_	_	312	587
Mov Cap-2 Maneuver	-	_	-	_	312	-
Stage 1	-	_	_	_	606	-
Stage 2	-	-	-	-	682	_
					002	
Δ						
Approach	EB		WB		SB	
HCM Control Delay, s	0.7		0		17.4	
HCM LOS					С	
Minor Lane/Major Mvm		EBL	EBT	WBT	WBR S	SRI n1
Capacity (veh/h)		1071	_ LD1	-		
HCM Lane V/C Ratio		0.028			_	422 0.314
HCM Control Delay (s)		8.5	-	-		
HCM Lane LOS		6.5 A	-	-	-	17.4
HCM 95th %tile Q(veh)		0.1	-	-	-	C 1.3
TOWN OUR TOURS Q(VCII)		0.1	•	-	-	1.3

Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EDT	MPT	MDD	CDI	CDD
	ERL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	F	4	140	0	Y	-
Traffic Vol, veh/h	5	355	448	6	6	5
Future Vol, veh/h	5	355	448	6	6	5
Conflicting Peds, #/hr	0	_ 0	_ 0	_ 0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	_	0	_
Veh in Median Storage	,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	5	386	487	7	7	5
P4 1 0.0		_				
	Major1		Major2		Minor2	
Conflicting Flow All	494	0	_	0	887	491
Stage 1	-	-	-	-	491	-
Stage 2	-	-	-	-	396	
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	_	5.42	_
Follow-up Hdwy	2.218	-	-	_	3.518	3.318
Pot Cap-1 Maneuver	1070	-	-	-	315	578
Stage 1	_	-	-	_	615	_
Stage 2	-	-	_	_	680	_
Platoon blocked, %		-	_		300	
Mov Cap-1 Maneuver	1070	-	_	_	313	578
Mov Cap-2 Maneuver	-	_	_	-	313	-
Stage 1	_			_	611	
Stage 2	<u>-</u>	_	-	_	680	-
Olaye Z			<u>-</u>	-	000	
Approach	EB		WB		SB	
HCM Control Delay, s	0.1		0		14.4	
HCM LOS					В	
NA						
Minor Lane/Major Mvm)T	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)		1070	-	-	-	395
HCM Lane V/C Ratio		0.005	-	-	-	0.03
HCM Control Delay (s)		8.4	0	-	-	14.4
HCM Lane LOS		Α	Α	-	-	В
HCM 95th %tile Q(veh))	0	-	-	-	0.1

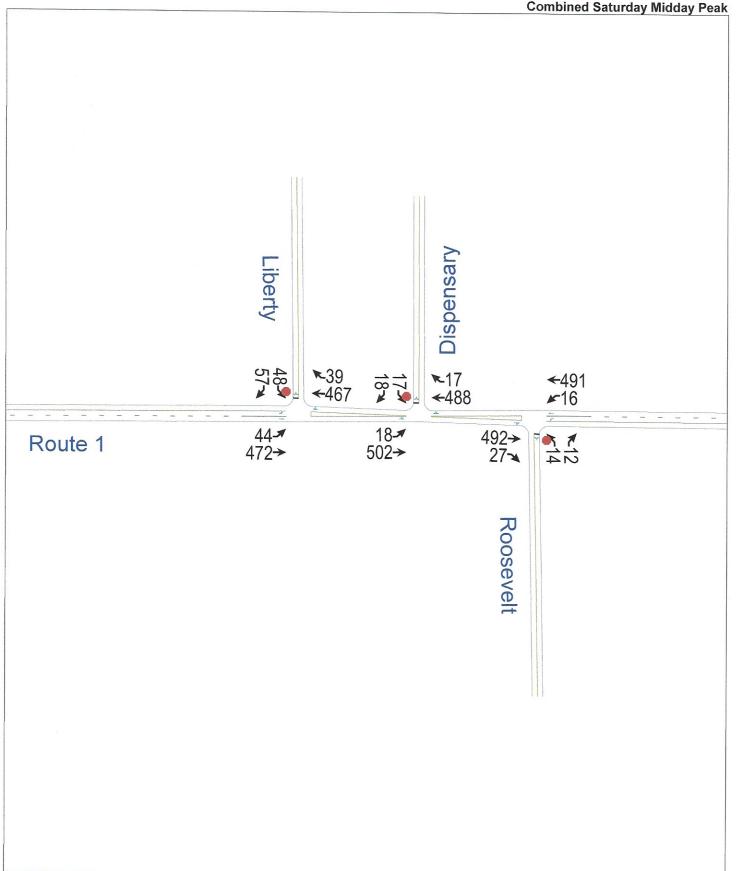
Intersection						
	0.5					
Movement E	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1	LDIV	VVDL	†	MOL	NOIL
	343	23	17	T 441	13	8
	343	23	17	441	13	8
Conflicting Peds, #/hr	0	0	0	0	0	0
ALONDOS DE REPRESENTA DE LA CONTRACTOR D	ree	Free	Free	Free	Stop	Stop
RT Channelized		None	riee -		Stop -	None
		None -		CONTRACTOR COLDS		
Storage Length	-		0	-	0	-
Veh in Median Storage, #		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	373	25	18	479	14	9
Major/Minor Ma	ajor1	N	Major2	-	Minor1	
Conflicting Flow All	0	0	398		901	386
		U	Harate San Control	0		
Stage 1	-	-	-	-	386	-
Stage 2	-	-	1.40		515	- 0.00
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	_
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	
Pot Cap-1 Maneuver	-	-	1161	-	309	662
Stage 1	-	-	-	-	687	-
Stage 2	-	-	-	-	600	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-		1161	-	304	662
Mov Cap-2 Maneuver	-	-	-	-	304	-
Stage 1	-	_	-	-	687	-
Stage 2	-	-	-	-	590	_
J						
A	ED		VAID		ND	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.3		15	
HCM LOS					С	
Minor Lane/Major Mvmt		VBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		383			1161	
HCM Lane V/C Ratio		0.06	-		0.016	-
HCM Control Delay (s) HCM Lane LOS		15	-		8.2	-
		C	-		A	-
HCM 95th %tile Q(veh)		0.2	-	-	0	-



Intersection						
Int Delay, s/veh	2.4					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	7	†	\$	וטייי	M.	ODIT
Traffic Vol, veh/h	59	618	478	62	42	48
Future Vol, veh/h	59	618	478	62	42	48
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-			None
Storage Length	0	-	-	-	0	-
Veh in Median Storage		0	0	_	0	_
Grade, %	_	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	64	672	520	67	46	52
	UT	012	020	UI	70	UZ
	Major1		Major2		Minor2	
Conflicting Flow All	587	0	-	0		554
Stage 1	-	-		-	554	-
Stage 2	-	-	-	-	800	-
Critical Hdwy	4.12	_	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	- T	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	988	-	-	-	165	532
Stage 1	-	-	-	-	575	-
Stage 2	-	-	-	-	442	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	988	-	-	-	154	532
Mov Cap-2 Maneuver	-	-	-	-	154	-
Stage 1	-	-	-	-	538	-
Stage 2	-	-	-	-	442	-
Approach	EB		WB		SB	
HCM Control Delay, s			Andrew Co. Printers and Parkets		and the same of th	
HCM LOS	0.8		0		28.7	
UCINI FOS					D	
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		988		-		248
HCM Lane V/C Ratio		0.065	-	-	_	0.394
HCM Control Delay (s)		8.9	_	-	_	28.7
HCM Lane LOS		Α	_	_	-	D
HCM 95th %tile Q(veh)	0.2	-	-	_	1.8
				Marie Sala		1.0

Intersection						
Int Delay, s/veh	0.5					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	LDL	स	†	NON	SDL	אטט
Traffic Vol, veh/h	12	648	528	11	11	12
Future Vol, veh/h	12	648	528	11	11	12
	0	THE RESERVE THE PARTY OF THE PA				
Conflicting Peds, #/hr		0	0	0	O Cton	0
Sign Control RT Channelized	Free	Free	Free	Free	Stop	Stop
	-	None	-	None		None
Storage Length	-	-	-	-	0	_
Veh in Median Storage		0	0	-	0	-
Grade, %	_	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	13	704	574	12	12	13
Major/Minor	Major1	N	Major2	1	Minor2	
Conflicting Flow All	586	0	viajoiz	0	1310	580
Stage 1	500	-			580	
			-	-		-
Stage 2	1 10	-	-	<u> </u>	730	0.00
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	
Follow-up Hdwy	2.218		-	-	3.518	
Pot Cap-1 Maneuver	989	-	-	-	175	514
Stage 1	-	_	-	_	560	-
Stage 2	-	-	-	-	477	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	989	-	-		171	514
Mov Cap-2 Maneuver	-	-	-	-	171	-
Stage 1	-	-	_	-	548	
Stage 2	-	-	-	-	477	-
Annroach	EB		WB		CD	
Approach					SB	
HCM Control Delay, s	0.2		0		20.2	
HCM LOS					С	
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)		989				262
HCM Lane V/C Ratio		0.013	_	_	_	0.095
HCM Control Delay (s)		8.7	0	_	-	20.2
HCM Lane LOS		Α	A	_	-	C
HCM 95th %tile Q(veh	1	0	A .			0.3
HOW JOHN JOHN GIVEN	1	U	_			0.5

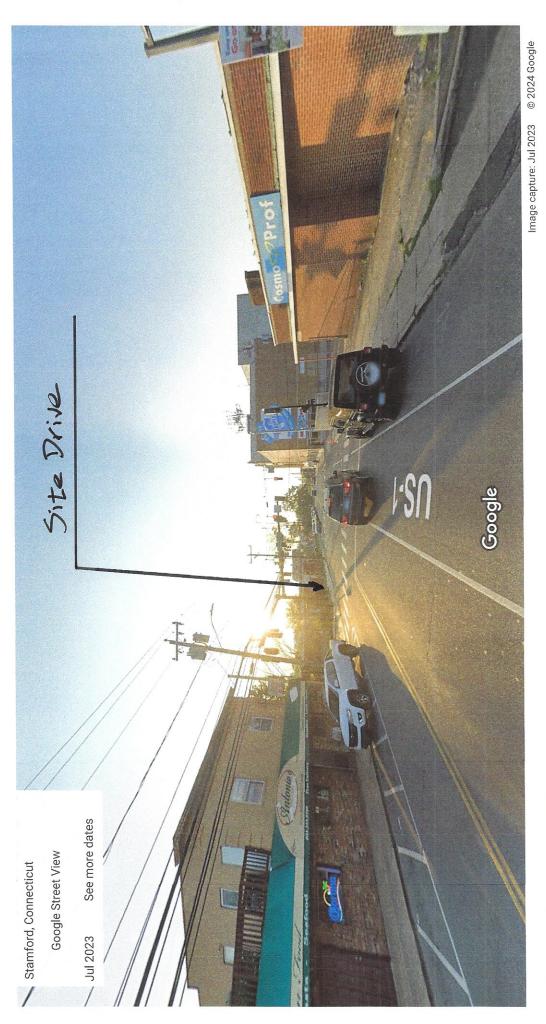
Intersection						
Int Delay, s/veh	0.6					
Movement	EDT	EDD	MDI	MDT	MDI	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	4	00	Ť	†	Y	40
Traffic Vol, veh/h	629	30	15	525	14	16
Future Vol, veh/h	629	30	15	525	14	16
Conflicting Peds, #/hr	0	0	0	0	0	0
0	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	i	None	-	None
Storage Length	-	-	0	-	0	-
Veh in Median Storage,	# 0	- 4	-	0	0	_
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	684	33	16	571	15	17
IVIVIIIL I IOVV	004	JJ	10	3/1	10	11
Major/Minor M	ajor1	1	Major2		Viinor1	
Conflicting Flow All	0	0	717	0	1304	701
Stage 1	_	_	, , ,	-	701	-
Stage 2	_			_	603	-
Critical Hdwy		-	4.12			
	-	16 m		-	6.42	6.22
Critical Hdwy Stg 1	-	-	<u>-</u>	-	5.42	- Sengananan
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	_	-	2.218	-	3.518	
Pot Cap-1 Maneuver	-	-	884	-	177	439
Stage 1	-	-	-	-	492	-
Stage 2	-	-	-	-	546	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	_	_	884	_	174	439
Mov Cap-2 Maneuver	_		- JO 1	_	174	-
Stage 1		-		_	492	
The second secon	-	-				-
Stage 2	-	-		_	536	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.3		21	
HCM LOS					С	
Minor Lane/Major Mvmt	1	VBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		257				
			-	-	884	-
HCM Lane V/C Ratio		0.127	_ 750455500450		0.018	-
HCM Control Delay (s)		21	-	-	9.1	-
HCM Lane LOS		С	-	-	Α	-
HCM 95th %tile Q(veh)		0.4	-	-	0.1	-



Intersection						
Int Delay, s/veh	2.4					
Movement	EBL	EBT	MPT	MPD	CDI	CDD
	EBL 1	<u></u> ←	WBT	WBR	SBL	SBR
Lane Configurations			167	20		E7
Traffic Vol, veh/h	44	472	467	39	48	57
Future Vol, veh/h	44	472 0	467	39	48	57
Conflicting Peds, #/hr	REPORT OF STREET		0	0		0
Sign Control RT Channelized	Free	Free	Free	Free	Stop	Stop
	-	110110	-		THE COLUMN TWO IS NOT THE OWNER.	None
Storage Length	0	-	-	-	0	_
Veh in Median Storage		0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	48	513	508	42	52	62
Major/Minor	Major1	N	Major2		Minor2	
Conflicting Flow All	550	0	-	0	1138	529
Stage 1	-	-		-	529	529
Stage 2	-	- -	- -		609	- -
Critical Hdwy	4.12	_	_		6.42	6.22
Critical Hdwy Stg 1	4.12			-	5.42	0.22
Critical Hdwy Stg 2	-	- 1911 (1911)	-	-	5.42	-
Follow-up Hdwy	2.218			-		
Pot Cap-1 Maneuver	1020	-	-	-	3.518	
The second of the second secon	OWNERS OF STREET, STRE	- T	-	-	223	550
Stage 1	-	<u>-</u>	- 200200000	-	591	_
Stage 2	-	-	-	-	543	-
Platoon blocked, %	4000	-	-	-	0.40	
Mov Cap-1 Maneuver	parameter services and services are services and services and services and services and services and services are services and services	-	-	-	213	550
Mov Cap-2 Maneuver	- processors		-	-	213	-
Stage 1	-	-	-	-	563	-
Stage 2	-	_	-	_	543	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.7		0		22.4	
HCM LOS	0.1		U		C	
TIOW 200					U	
ESSENTIAL CONTRACTOR C	DATE INVESTIGATION					
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)		1020	-		-	319
HCM Lane V/C Ratio		0.047	-	-	-	0.358
HCM Control Delay (s))	8.7	-	-	-	22.4
HCM Lane LOS		Α	-	-	-	С
HCM 95th %tile Q(veh)	0.1	-	-	-	1.6
•		A CONTRACTOR OF THE PARTY OF TH				

Intersection						
Int Delay, s/veh	0.7					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्स	1		W	ODIT
Traffic Vol, veh/h	18	502	488	17	17	18
Future Vol, veh/h	18	502	488	17	17	18
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None		None	-	None
Storage Length	_	-	=	-	0	-
Veh in Median Storage		0	0	_	0	-
Grade, %	-	0	0	-	0	
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	20	546	530	18	18	20
Major/Minor	Major1	Ň	Major2		Minor2	
Conflicting Flow All	548	0	-			539
Stage 1	-	-	_	-	539	-
Stage 2		-	-	_	586	-
Critical Hdwy	4.12	- -		_	6.42	6.22
Critical Hdwy Stg 1	CHARLEST				5.42	0.22
	-		-	-	5.42	-
Critical Hdwy Stg 2	2.218	- 15 E	-	-		
Follow-up Hdwy		-	- (8)33,533	-		
Pot Cap-1 Maneuver	1021	-	-	-	227	542
Stage 1	-	-	-	-	585	-
Stage 2	-	- -	-	-	556	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1021	-	-	-	221	542
Mov Cap-2 Maneuver	-	_	-	-	221	-
Stage 1	-	-	-	-	569	-
Stage 2	-	-	-	-	556	-
Annroach	ED		WB		SB	
Approach	EB		and the second second			
HCM Control Delay, s	0.3		0		17.9	
HCM LOS		Mester execution			С	
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)		1021				318
HCM Lane V/C Ratio		0.019	-	_	_	0.12
HCM Control Delay (s)	8.6	0	NATIONAL PROPERTY AND ADDRESS OF THE PARTY AND	AND DESCRIPTION	4-0
HCM Lane LOS	I	Α	A	Contract Contract Contract	ALMOST BUILDINGS	C
HCM 95th %tile Q(veh	1	0.1			AND DESCRIPTION	0.4
HOW JOHN JOHNE W(VEI)	7	0.1				0.4

Exhibit 14
Sight Line Evaluation
Proposed Dispensary Site Drive



Approaching E

Eastbound

Sing Sylve S

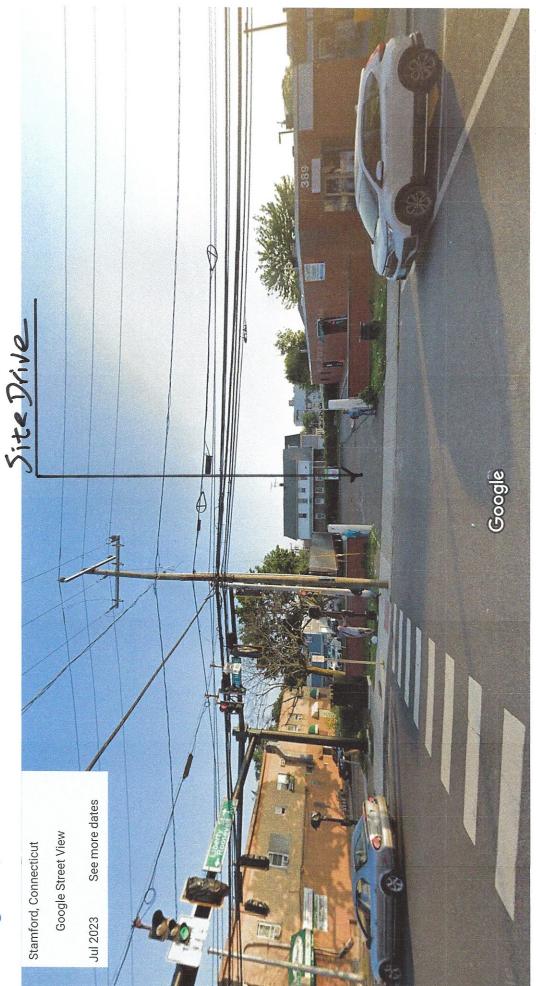
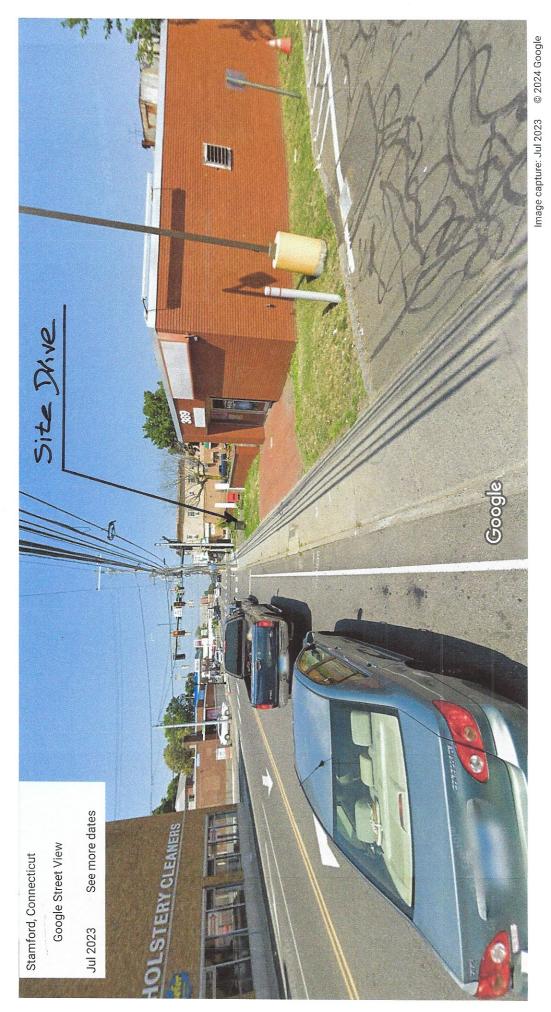


Image capture: Jul 2023 © 2024 Google

View from across the street





Westbound

Approaching

Diaz St Ave Diaz St Ave Diaz St Ave Diaz St Mirgil Virgil Virgil